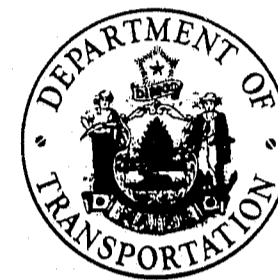


F.S.W.A. REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	IG-195-2(9)	1	37

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

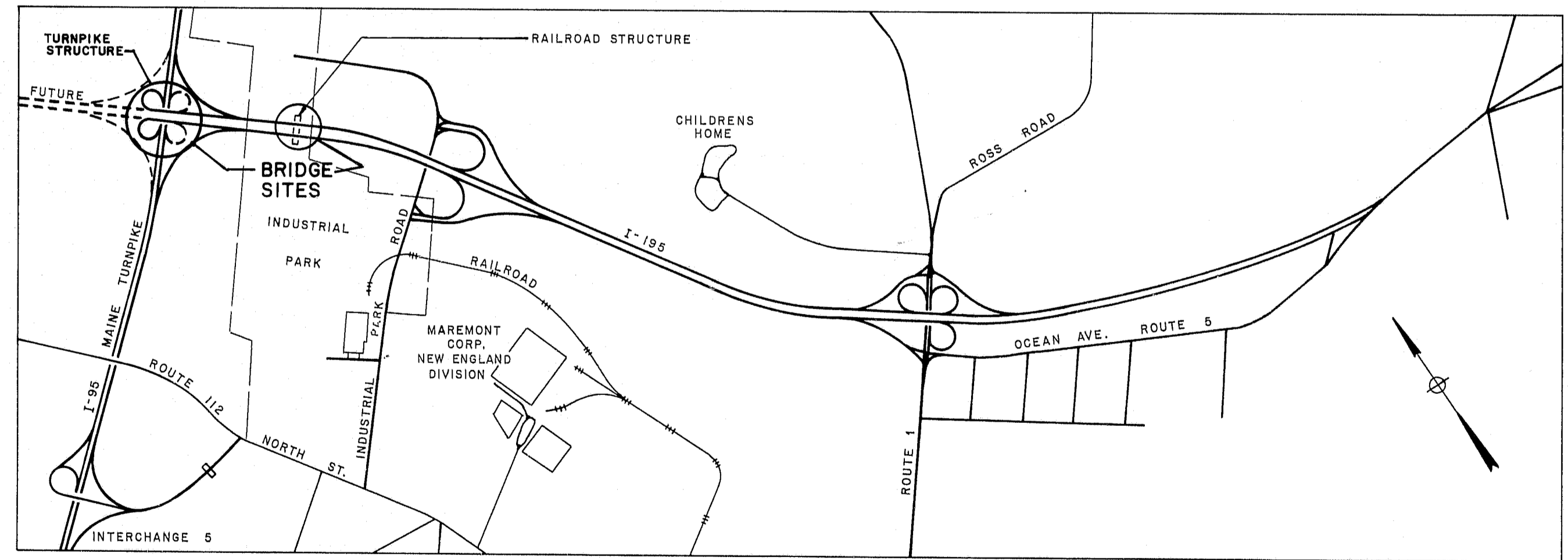
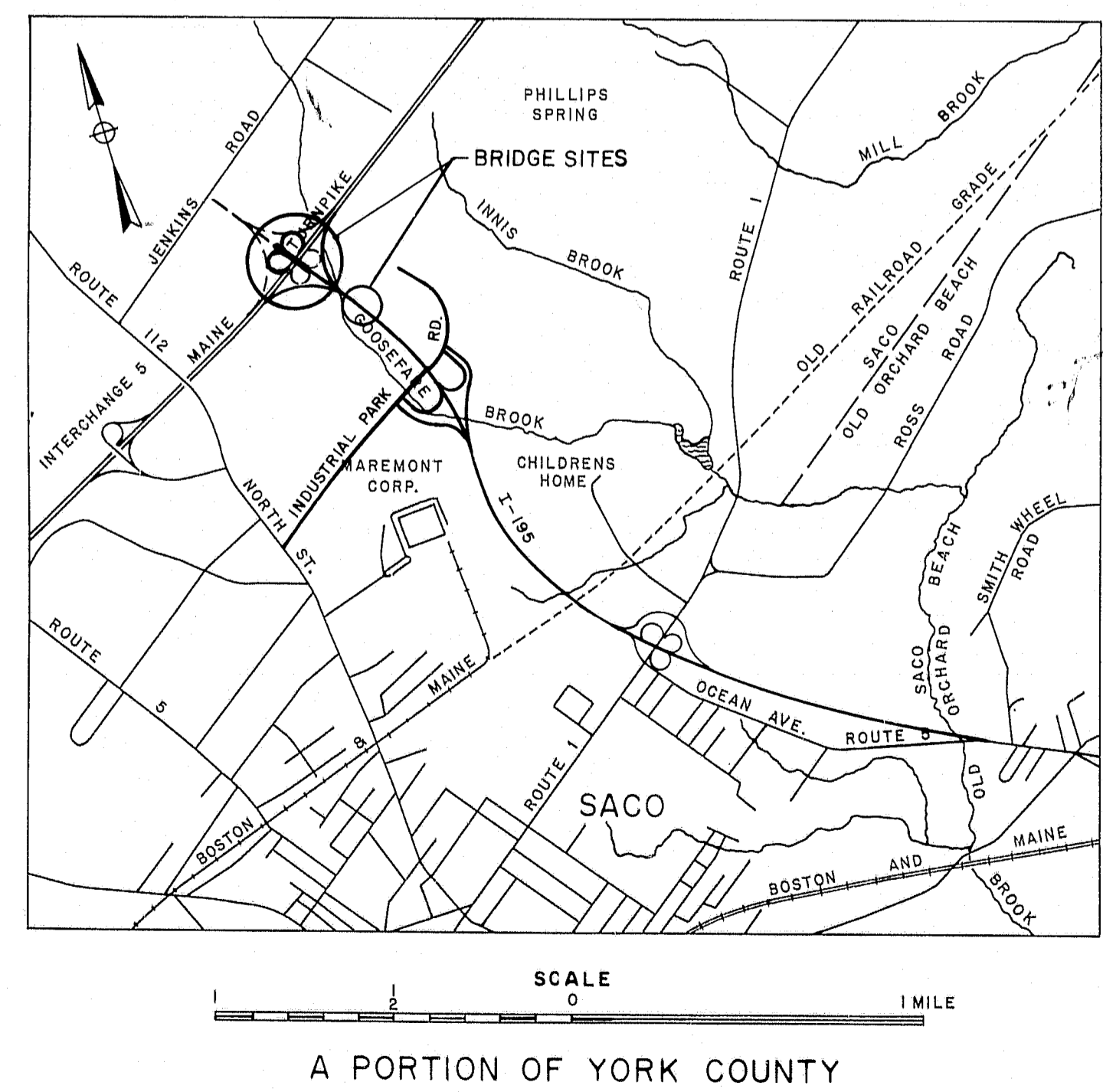


BUREAU OF HIGHWAYS
SACO
YORK COUNTY
MAINE FEDERAL AID INTERSTATE
PROJECT NO. I-IG-195-2(9)35
TOTAL LENGTH 0.0464 MILES

ROUTE 195
OVER
MAINE TURNPIKE *Completed 1982*
AND
ROUTE 195
OVER
INDUSTRIAL PARK RAILROAD SPUR

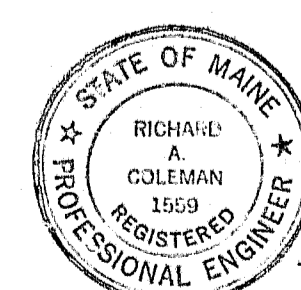
CONVENTIONAL SIGNS	
COUNTY LINES	TRAVELLED WAY - PROPOSED
TOWN LINES	UNDERGROUND UTILITIES - EXISTING
PROPERTY LINES	UNDERGROUND UTILITIES - PROPOSED
R/W LINES - EXISTING	RAILROAD - SINGLE TRACK
R/W LINES - NEW - ACCESS CONTROL	RAILROAD - DOUBLE TRACK
R/W LINES - NEW - NO ACCESS CONTROL	UTILITY POLE - EXISTING
CULVERT - EXISTING	UTILITY POLE - JOINT OCCUPANCY
CULVERT - PROPOSED	PROPOSED UTILITY POLE - TEMPORARY
CURBING - EXISTING	PROPOSED UTILITY POLE - PERMANENT
CURBING - PROPOSED	TREES
TRAVELLED WAY - EXISTING	WOODS

INDEX OF SHEETS (TURNPIKE STRUCTURE)	
1.	TITLE SHEET
2.	GENERAL PLAN
3.	ESTIMATED QUANTITIES
4.	PROFILE
5.	FOUNDATION SURVEY
6.	FOOTING PLAN
7.	ABUTMENTS
8.	PIER
9.	SLOPE PROTECTION & APPROACH SLABS
10.	FRAMING PLAN & DETAILS
11.	STRUCTURAL STEEL DETAILS
12.	SUPERSTRUCTURE DETAILS
13.	END POST DETAIL
14.	JOINT DETAILS
15-16	REINFORCING STEEL SCHEDULE
17-28	RAILROAD STRUCTURE
	STANDARD DETAILS
BD 101-74	BEARING PEDESTAL
BD 104-77	ARMORED JOINT, DRAIN, SHEAR CONNECTORS AND MISCELLANEOUS STRUCTURAL DETAILS
BD 113-78	DIAPHRAGMS AND CROSSFRAMES
BD 114-77	ALUMINUM BRIDGE RAILING, 2-BAR
BD 120-79	END POSTS
(2)	FIELD OFFICE
	MAINTENANCE OF TRAFFIC (3 SHEETS)



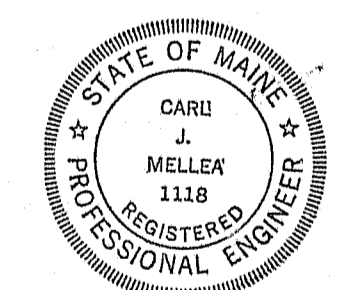
TRAFFIC DATA	
A.D.T.	19
A.D.T.	19
D.H.V.	
T. (%)	
D. (%)	
V.	
P.S.D. (%)	
18 KIPS	

NOTE:
ALL WORK CONTEMPLATED UNDER THIS CONTRACT TO BE GOVERNED BY AND IN CONFORMITY WITH THE STANDARD SPECIFICATIONS (REVISION OF JUNE 1968) AND SUPPLEMENTS THERETO, EXCEPT AS MODIFIED ON THE PLANS AND IN THE SPECIAL PROVISIONS.



APPROVED: STATE OF MAINE
DEPARTMENT OF TRANSPORTATION
Richard A. Coleman COMMISSIONER
Richard A. Coleman BUREAU DIRECTOR AND CHIEF ENGINEER
DATE 11-17-81

R92-192

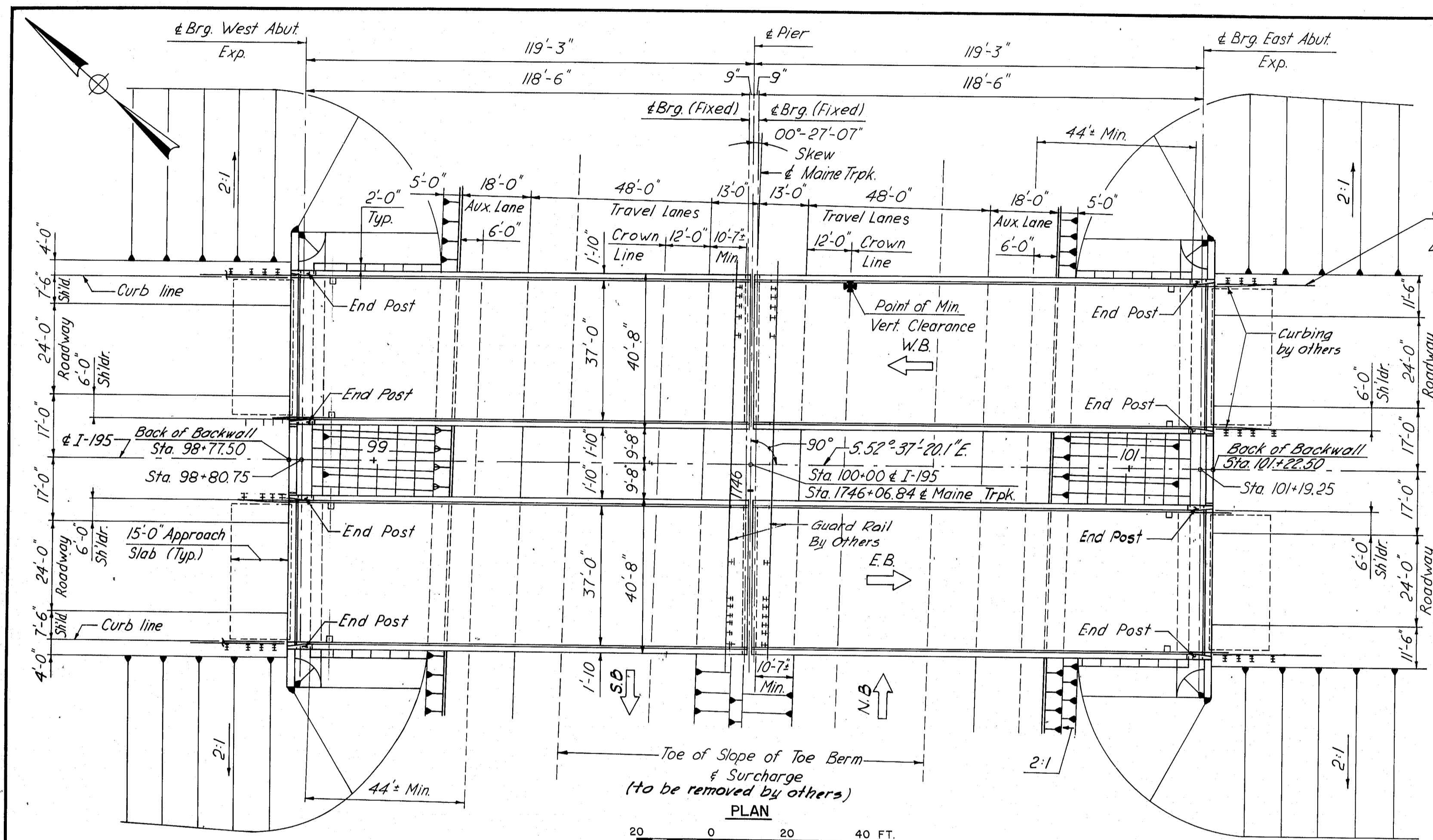


Carl J. Mellea
CARL J. MELLEA

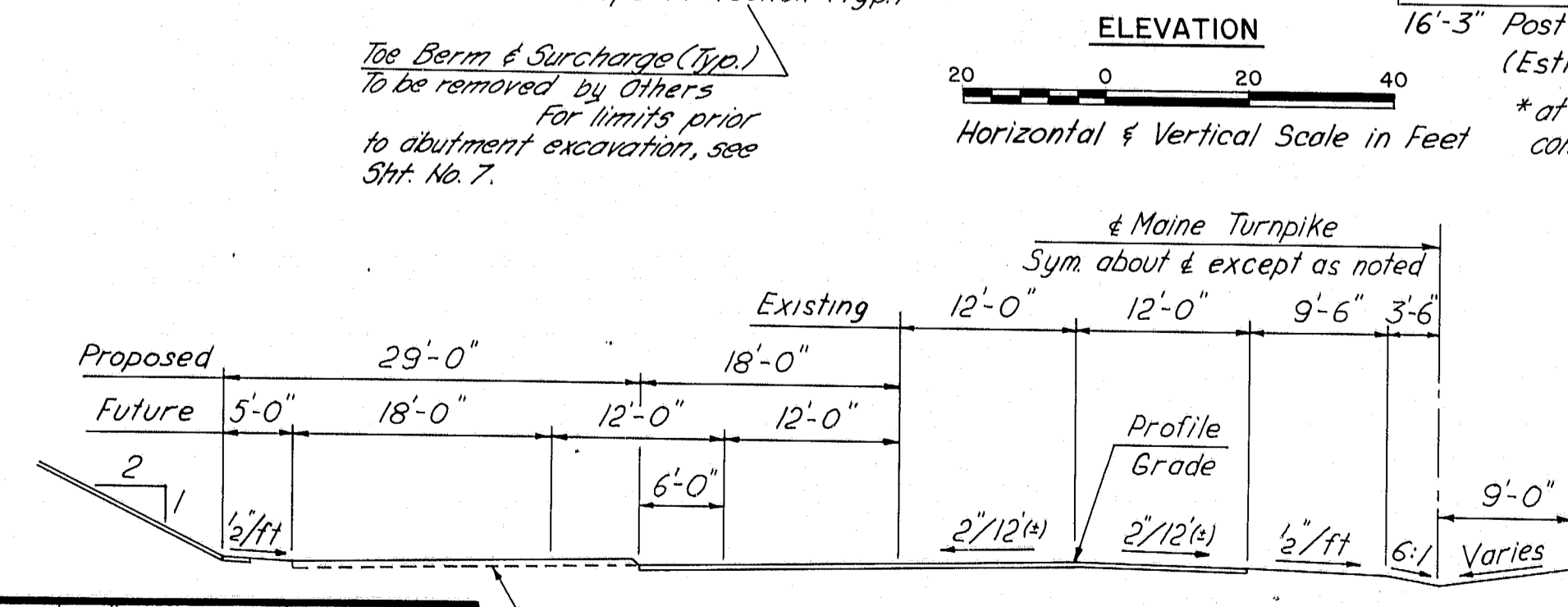
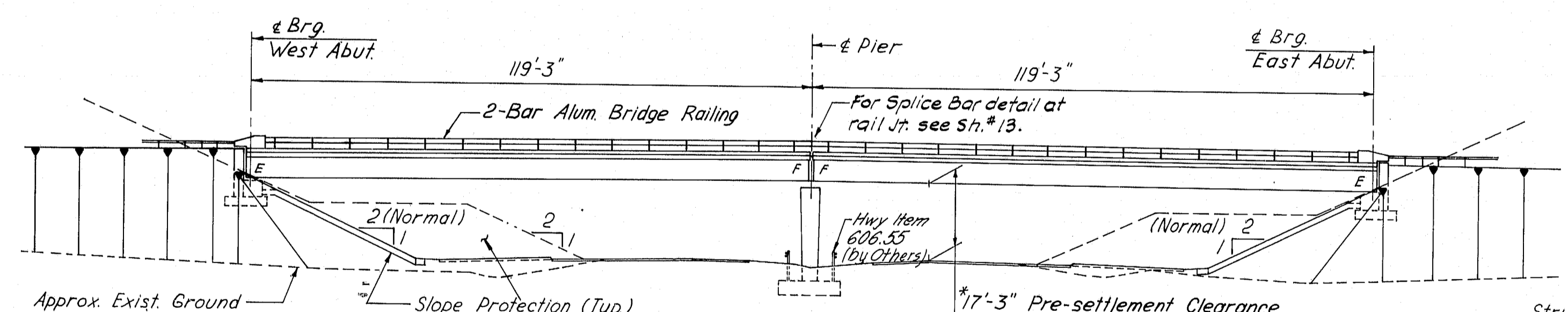
HNTB
HOWARD NEEDLES TAMMEN & BERENDSON
CONSULTING ENGINEERS BOSTON

APPROVED: UNITED STATES
DEPARTMENT OF TRANSPORTATION
FEDERAL HIGHWAY ADMINISTRATION
REGION I
DIVISION ADMINISTRATOR DATE

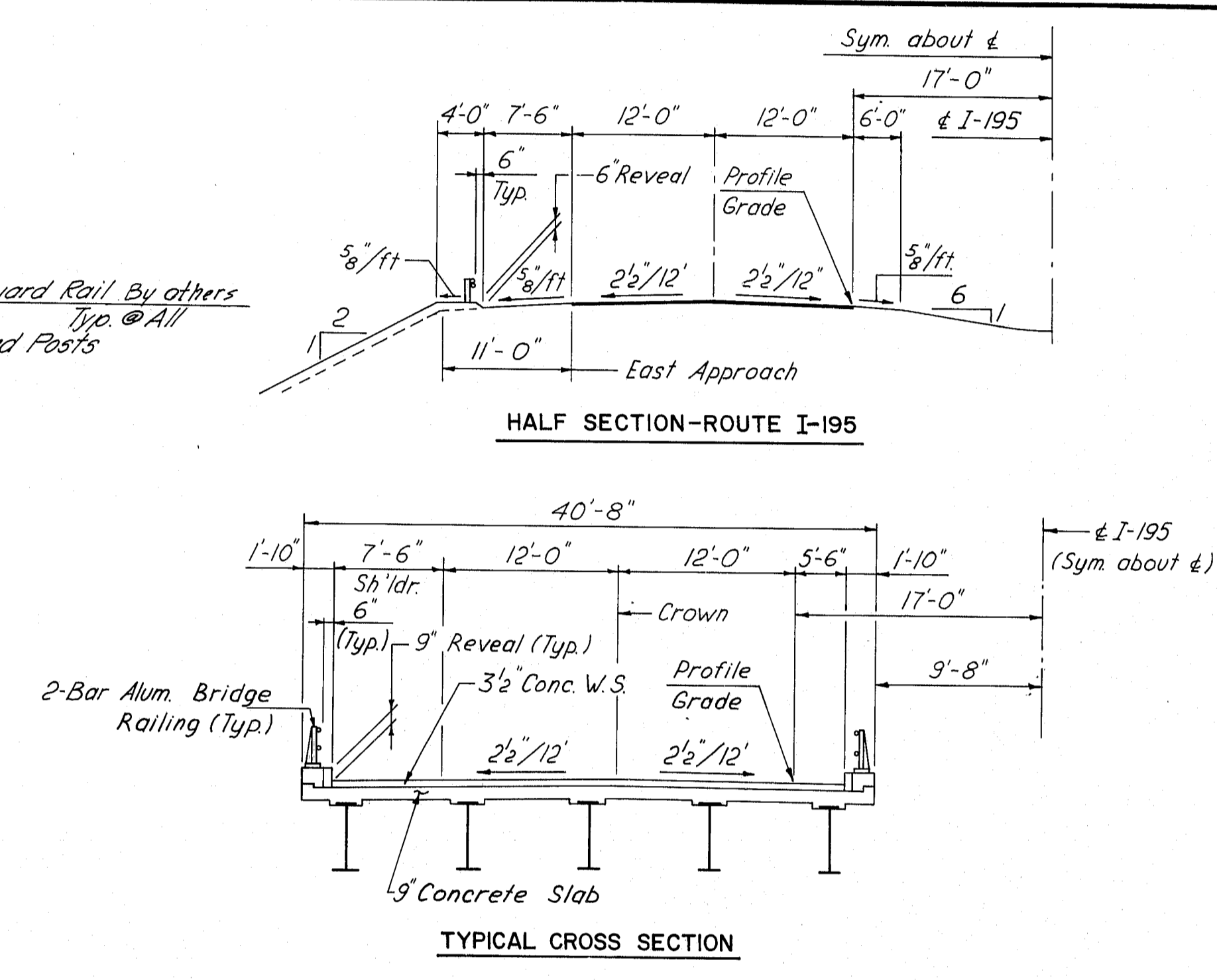
FORMA. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	IG-195-2(9)	2	37



NOTE:
Ultimate construction shown for Maine Turnpike



NO.	REVISION	BY	DATE
		MADE	V.G. 4-81
		TRACED	
		CHECKED	N.F.O. 4-81
		IN CHARGE OF	J.A.E.



DESIGN
AASHTO Specifications for Highway Bridges 1977 and Interim Specifications 1978 & 1979

CONTRACT
State of Maine, State Highway Commission, Standard Specifications Highway and Bridges, Revision of June 1968.

DESIGN LOADING
LIVE LOAD
HS25 as modified for Interstate

MATERIALS

CONCRETE
Wearing Surface - Class AA
Slope Protection - Class Y
All Others - Class A

REINFORCING STEEL
ASTM A615 Grade 60

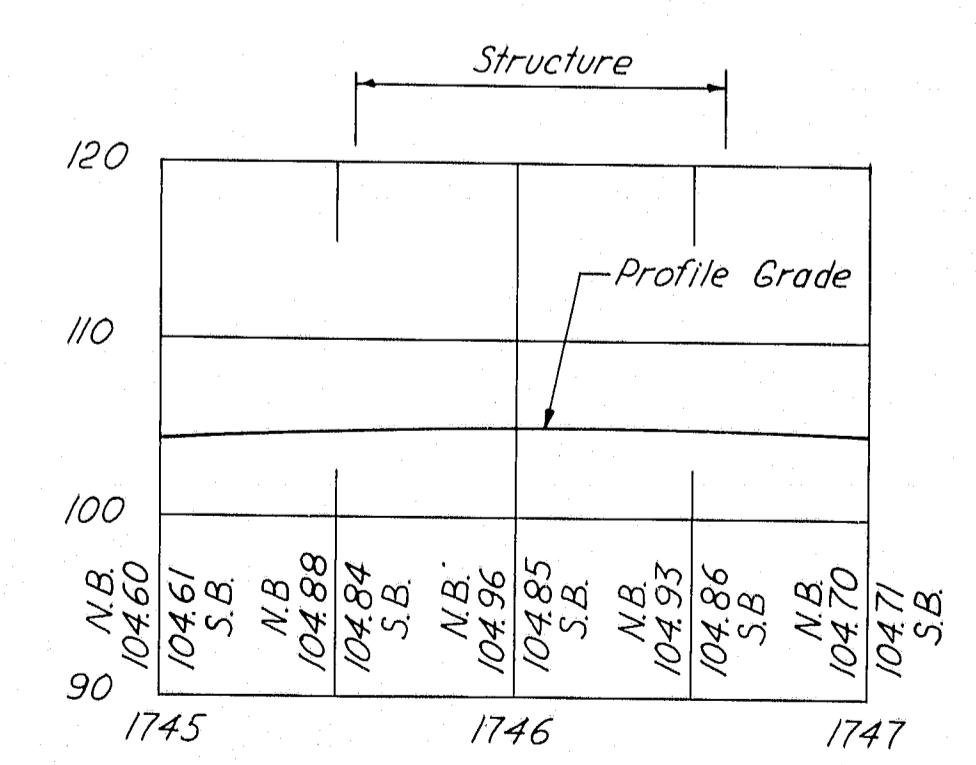
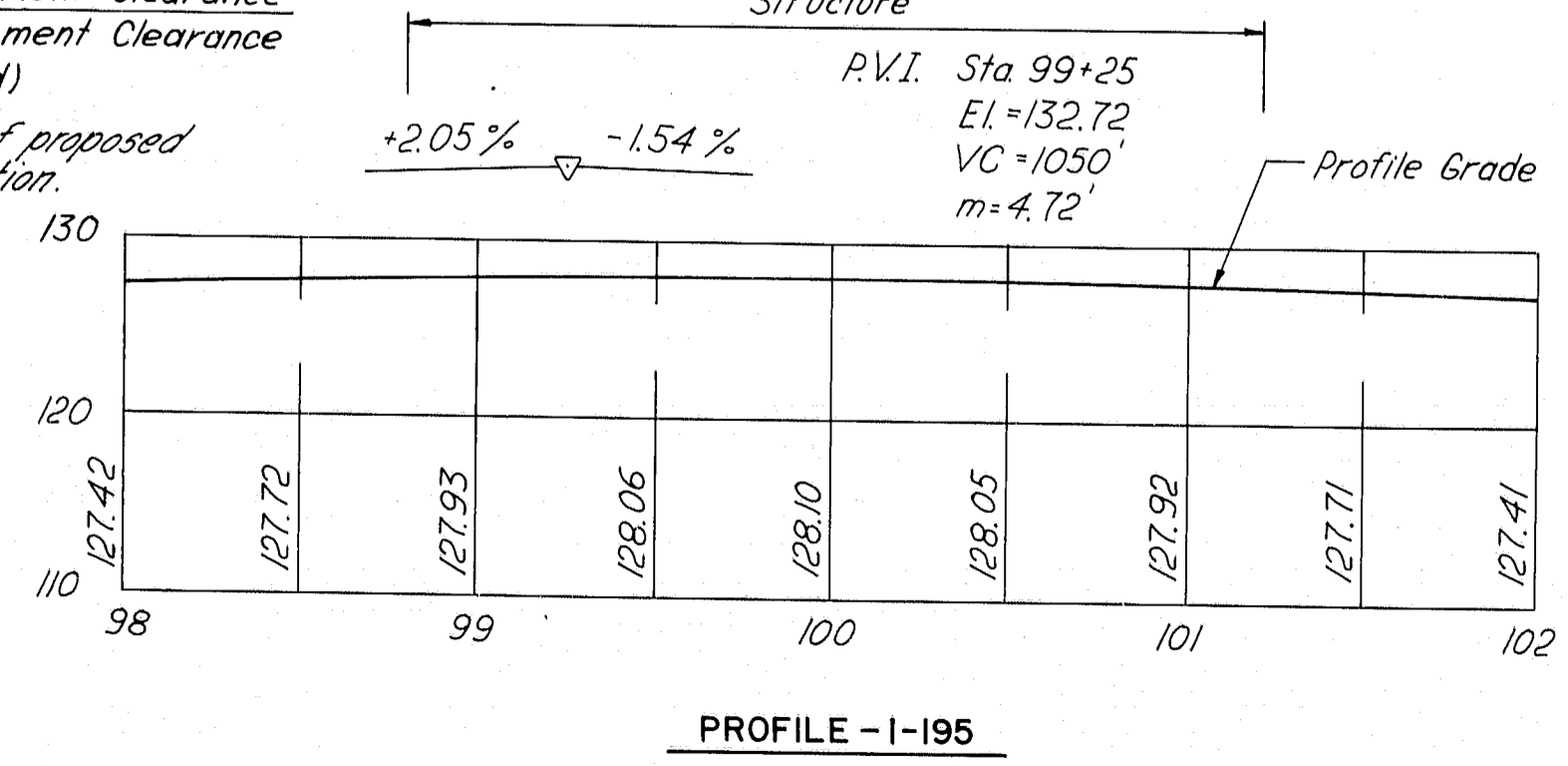
STRUCTURAL STEEL
Welded Girders ASTM { A572, Grade 50 Flanges
A36 Webs
High Strength Bolts - ASTM A325
Joints - ASTM A588 (any grade)
All Others - ASTM A36

BASIC ALLOWABLE STRESSES

REINFORCING STEEL
 $f_s = 24,000$ p.s.i.

STRUCTURAL STEEL
ASTM A588 $f_s = 27,000$ p.s.i.
ASTM A36 $f_s = 20,000$ p.s.i.
ASTM A325 $f_v = 25,000$ p.s.i.
ASTM A572 $f_s = 27,000$ p.s.i.

CONCRETE
 $f_c = 1,200$ p.s.i. $n = 9$



R92-193
HOWARD NEEDLER TAMMEN & BERENDORF
CONSULTING ENGINEERS
BOSTON

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

ROUTE I-195
OVER
MAINE TURNPIKE

GENERAL PLAN
SACO, MAINE

SHEET 2 OF 16 AUGUSTA, MAINE

STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
MAINE	16-195-2(9)	3	37

BRIDGE QUANTITIES			
ITEM NUMBER	DESCRIPTION	QUANTITIES	UNIT
203.26	Gravel Borrow	442	C.Y.
206.081	Structural Earth Excavation - Abutments and Retaining Walls	584	C.Y.
206.10	Structural Earth Excavation - Piers	206	C.Y.
502.21	Structural Concrete, Abutments & Retaining Walls	427	C.Y.
502.23	Structural Concrete Piers	225	C.Y.
502.26	Structural Concrete, Roadway & Sidewalk Slabs on Steel Bridges	Nec.	L.S.
502.29	Structural Concrete, Wearing Surface on Bridges	Nec.	L.S.
502.3101	Structural Concrete, Approach Slabs	1	L.S.
503.12	Reinforcing Steel, Fabricated and Delivered	196,400	Lb.
503.13	Reinforcing Steel Placing	196,400	Lb.
504.70	Structural Steel, Fabricated and Delivered	Nec.	L.S.
504.71	Structural Steel, Erection	Nec.	L.S.
505.08	Shear Connectors	Nec.	L.S.
506.141	Field Painting, Structural Steel	Nec.	L.S.
507.141	Aluminum Bridge Railing, Type 'A'	935	L.F.
512.07	French Drains (Stones Only)	36	C.Y.
513.09	Slope Protection Portland Cement Concrete	880	S.Y.
514.06	Curing Box For Concrete Cylinder	0.5	Ea.
515.20	Protective Coating For Concrete Surfaces	2370	S.Y.
520.201	Expansion Devices	Nec.	L.S.
609.13	Vertical Bridge Curb - Type 1	948	L.F.
653.22	2" Thick Polystyrene Plastic Insulation	206	S.Y.
659.05	Labor, Straight Time	20	M.Hr.
659.10	Air Compressor (incl. op.)	10	Hour
659.11	Air Tool (incl. op.)	10	Hour
659.13	Gradozer (incl. op.)	10	Hour
659.17	Truck - small (incl. op.)	10	Hour
659.22	Front End Loader (incl. op.)	10	Hour
659.09	Field Office, Type B	10	Hour
645.67	Removing Pavement Markings	.5	Each
645.70	Temporary & Painted Pavement Markings, White or Yellow	730	S.F.
652.30	Flashing Arrow Board	2400	L.F.
652.31	Type I Barricades	2	Each
652.33	Drums	50	Each
652.35	Construction Signs	10	Each
652.36	Maintenance of Traffic Control Devices	660	S.F.
		260	C.D.
659.10	Mobilization	.75	L.S.
660.21	On-the-job Training (Bid)	1000	M.Hr.
630.0606	Traffic Officers (Non-bid Item)	100	M.Hr.
632.08	Warning Lights	2	Grp.

NOTES:

Estimated Quantity of Structural Steel, Fab, Del, Erected & Painted.
 ASTM A36 351,000 lbs; ASTM A572 424,000 lbs; ASTM A588 8,000 lbs.
 Estimated Quantity of Shear Connectors 5,800 Ea. (5,684 lbs).
 Estimated Quantity of Concrete Item 502.26 - 625 C.Y.
 Estimated Quantity of Concrete Item 502.29 - 192 C.Y.
 Estimated Quantity of Concrete Item 502.31 - 55 C.Y.

NO.	REVISION	BY	DATE
		MADE	F.C.S. 7-81
		TRACED	
		CHECKED	V.G. 7-81
		IN CHARGE OF	J.R.E.

R92-194

HNTB
 HOWARD NEEDLES TAMMEN & BERGENDOFF
 CONSULTING ENGINEERS BOSTON

STATE OF MAINE
 DEPARTMENT OF TRANSPORTATION

ROUTE 1-195
 OVER
 MAINE TURNPIKE

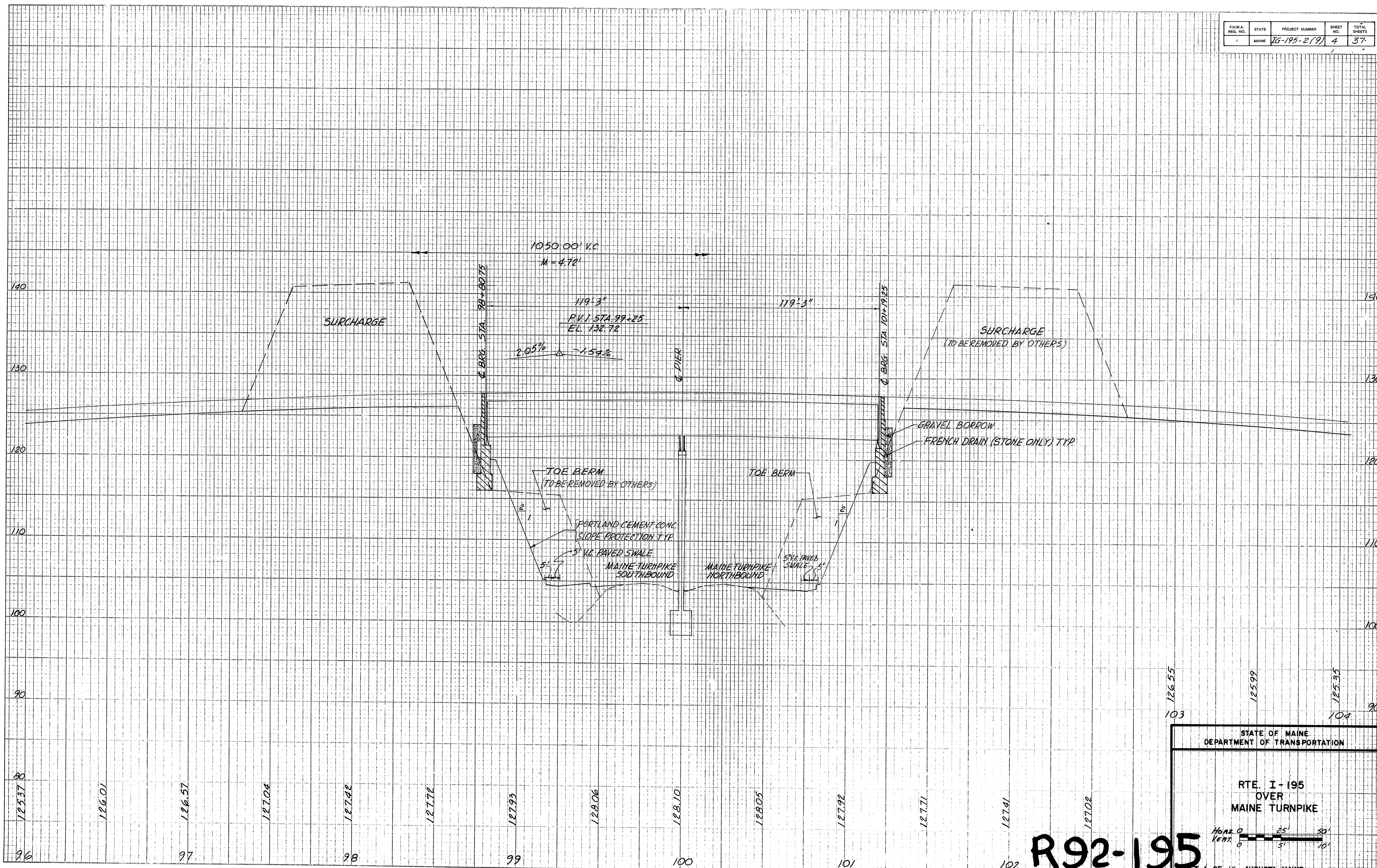
ESTIMATED QUANTITIES
 SACO, MAINE

SHEET 3 OF 16 AUGUSTA, MAINE

F.W.A. PROJ. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
	MAINE	16-195-2(1)	4	37

DATE	BY
DATE	BY
DATE	BY
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DATE	BY

DATE	BY
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DATE	BY



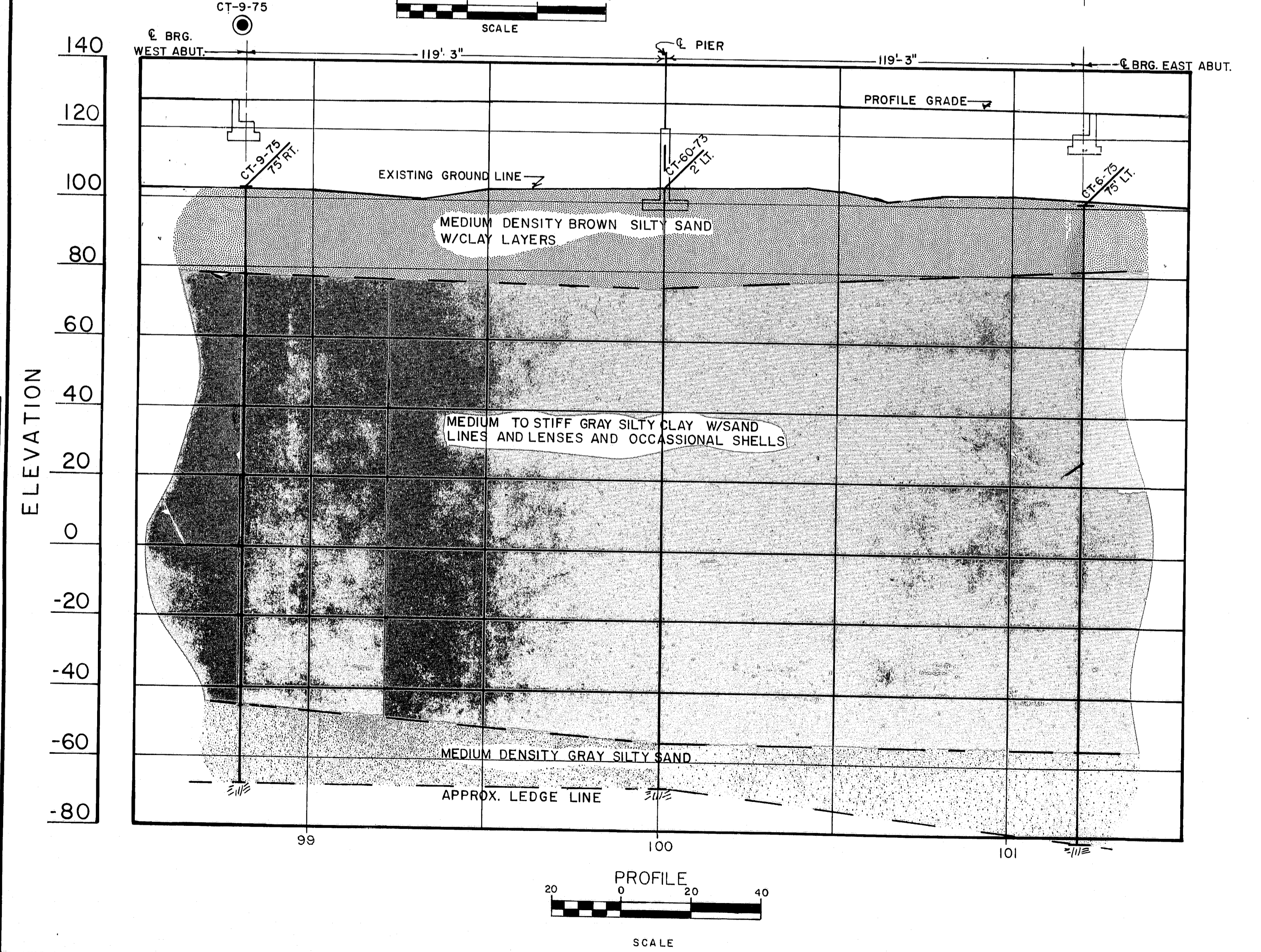
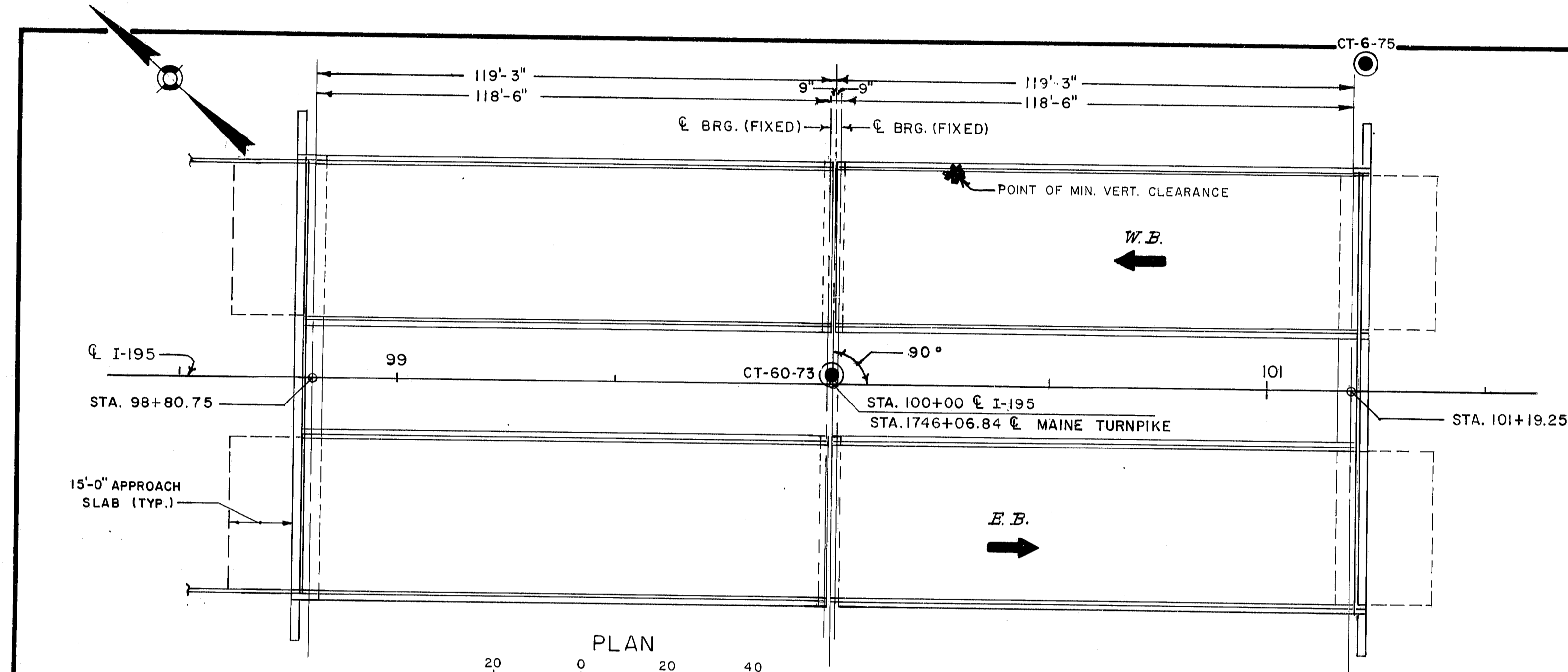
STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

RTE. I-195
OVER
MAINE TURNPIKE

Horizontal Scale: 1" = 25'
Vertical Scale: 1" = 5'

SHEET 4 OF 16, AUGUSTA, MAINE
Saco I-195-2(1)

R92-195



BORING CT-9-75 STATION 98+80 75' RT.				BORING CT-60-73 STATION 100+00 2' LT.				BORING CT-6-75 STATION 101+20 75' LT.					
DRIVING RESISTANCE-VANE SHEAR STRENGTH-WATER CONTENT				DRIVING RESISTANCE-VANE SHEAR STRENGTH-WATER CONTENT				DRIVING RESISTANCE-VANE SHEAR STRENGTH-WATER CONTENT					
Blows/Ft.	Tons/Sq. Ft.	Percent		Blows/Ft.	Tons/Sq. Ft.	Percent		Blows/Ft.	Tons/Sq. Ft.	Percent			
20	0.4	0.8	20	40	0.4	0.8	20	40	20	0.4	0.8	20	40
ELEV. 103.2				ELEV. 104.0				ELEV. 101.0					
1U	MEG. DENSITY FINE BRN. SAND			1D	MEG. DENSITY FINE SILTY SAND			1U	MEG. DENSITY BRN. SILTY SAND W/CLAY LAYERS			1U	MEG. DENSITY BRN. SILTY SAND W/CLAY LAYERS
2U	MEG. DENSITY FINE SAND			2U	DENSE TO MED. SAND			2U	MEG. DENSITY BRN. SILTY SAND W/CLAY LAYERS			2U	MEG. DENSITY BRN. SILTY SAND W/CLAY LAYERS
3U	FINE GRAY SILTY SAND			3U	MEG. DENSITY SAND			3U	MEG. DENSITY BRN. SILTY SAND W/CLAY LAYERS			3U	MEG. DENSITY BRN. SILTY SAND W/CLAY LAYERS
4U	FINE GRAY SILTY SAND			4U	MEG. DENSITY SAND			4U	MEG. DENSITY BRN. SILTY SAND W/CLAY LAYERS			4U	MEG. DENSITY BRN. SILTY SAND W/CLAY LAYERS
5U	MEG. TO STIFF GRAY SILTY CLAY			5U	MEG. TO STIFF GRAY SILTY CLAY			5U	MEG. TO STIFF GRAY SILTY CLAY			5U	MEG. TO STIFF GRAY SILTY CLAY
6U	MEG. TO STIFF GRAY SILTY CLAY			6U	MEG. TO STIFF GRAY SILTY CLAY			6U	MEG. TO STIFF GRAY SILTY CLAY			6U	MEG. TO STIFF GRAY SILTY CLAY
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40U	MEG. TO STIFF GRAY SILTY CLAY			40U	MEG. TO STIFF GRAY SILTY CLAY			40U	MEG. TO STIFF GRAY SILTY CLAY			40U	MEG. TO STIFF GRAY SILTY CLAY

BORING NOTES

All samples and vane are made ahead of casing
 2-1/2" & 4" casing used
 Number of blows required to drive extra heavy casing one foot with 400ft. lbs. of energy per blow
 Location of sample or sample attempt
 Number and type of dry sample
 ID S & H Sampler #1290's
 IC 2" O.D. 16ga. seamless tubing
 IU 3-1/2" O.D. 16ga. seamless tubing
 IW Wash sample and number
 MD Unsuccessful sample attempt and type of sampler
 Number of blows required to drive spoon or tubing one foot with 350 ft. lbs. of energy per blow
 H Sampling spoon or seamless tubing driven by static weight of 30lb. rods and hammer
 P Piston sampler
 F Field vane test
 Bottom of boring (may not be bottom of soil strata)
 Locations cored by diamond bit and percent recovery of rock

SHEAR NOTES

Field vane shear strengths
 Laboratory vane shear strengths
 Shear strengths in excess of capacity of equipment
 Triaxial shear strengths

WATER CONTENT NOTES

Natural water content, given as percent of dry weight
 Plastic and liquid limits

PROJECT DESIGN ENGINEER	BY	DATE
DESIGN - DETAILED		
CHECKED		
REVISIONS		
FIELD CHANGES		

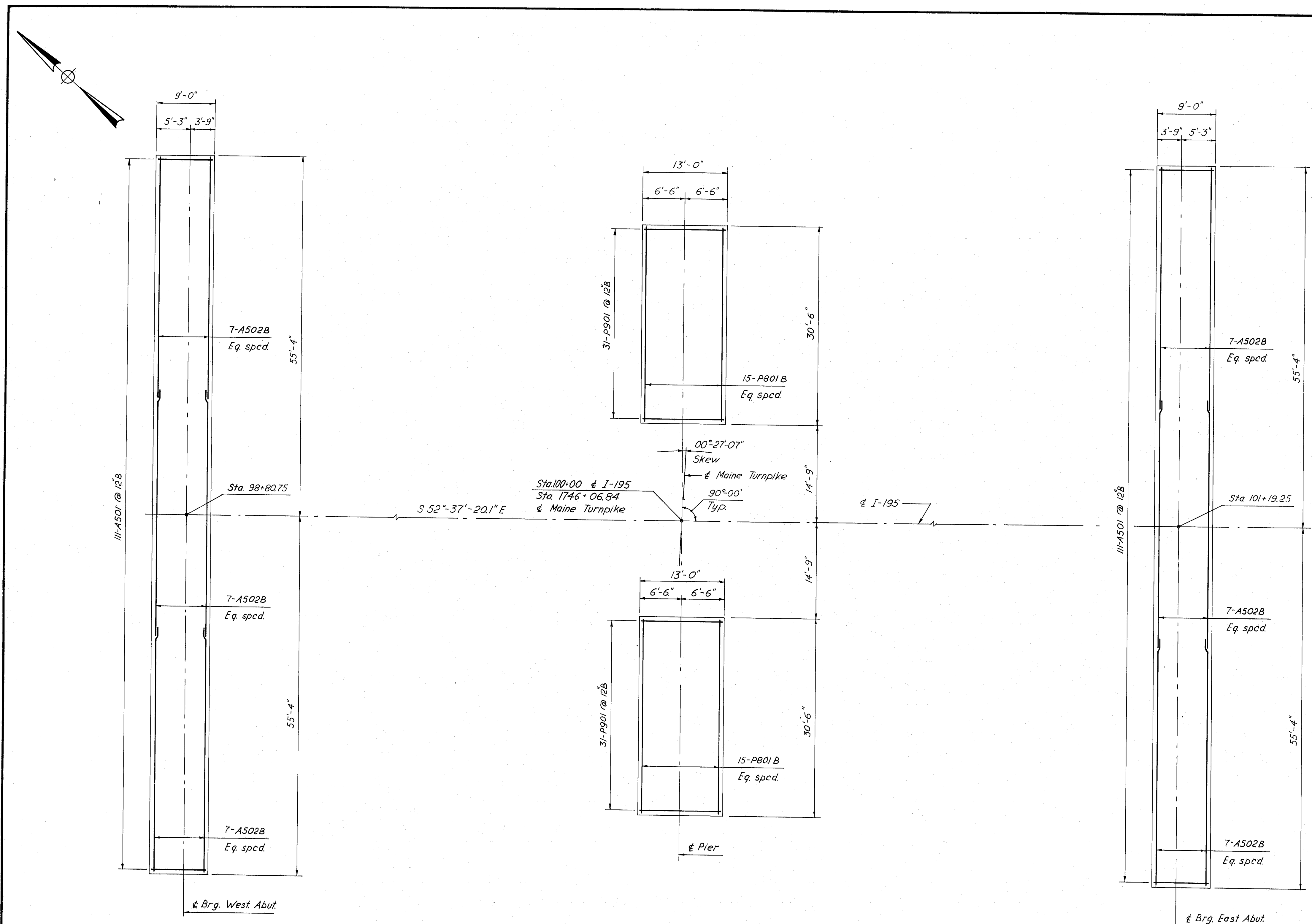
STATE OF MAINE
 DEPARTMENT OF TRANSPORTATION

ROUTE I-195
 OVER
MAINE TURNPIKE
 IN THE CITY OF
SACO
YORK COUNTY
 FOUNDATION SURVEY

SHEET 5 OF 16 AUGUSTA, MAINE

R92-196

F.H.W.A. DES. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	I-195-2(9)	6	37



- NOTES:**
- Abutment Notes:
1. Chamfer all exposed edges of concrete a consistent dimension between $\frac{1}{2}$ " and $\frac{3}{4}$ " inclusive, unless otherwise indicated.
 2. Reinforcing steel shall have 2" cover unless otherwise indicated.
 3. Place reinforcing steel in bridge seats to clear anchor bolts.
 4. Polyvinylchloride waterstops as shown on Standard Details BD 104 shall be placed in all vertical construction joints.
 5. Waterstops are not required in horizontal construction joints.
 6. The top portion of the abutment backwall shall be placed after all superstructure structural slab concrete is in place and after all necessary adjustments to the joint armor have been made, unless another method, which allows for the proper alignment and adjustment of the joint armor, is approved by the Engineer.
 7. Maximum calculated footing pressure is $2\frac{T}{a}$
 8. N.F. denotes near face; F.F. denotes far face; E.F. denotes each face; B & T denotes bottom & top bars.
 9. Protective coating for concrete surfaces shall be applied to the top of backwall.
 10. For approach slab details see Sh. 9.
 11. For joint details see Sh. 14.
 12. For construction joint details & shear keys see Standard Detail Sheet BD 104-77.

FOOTING PLAN

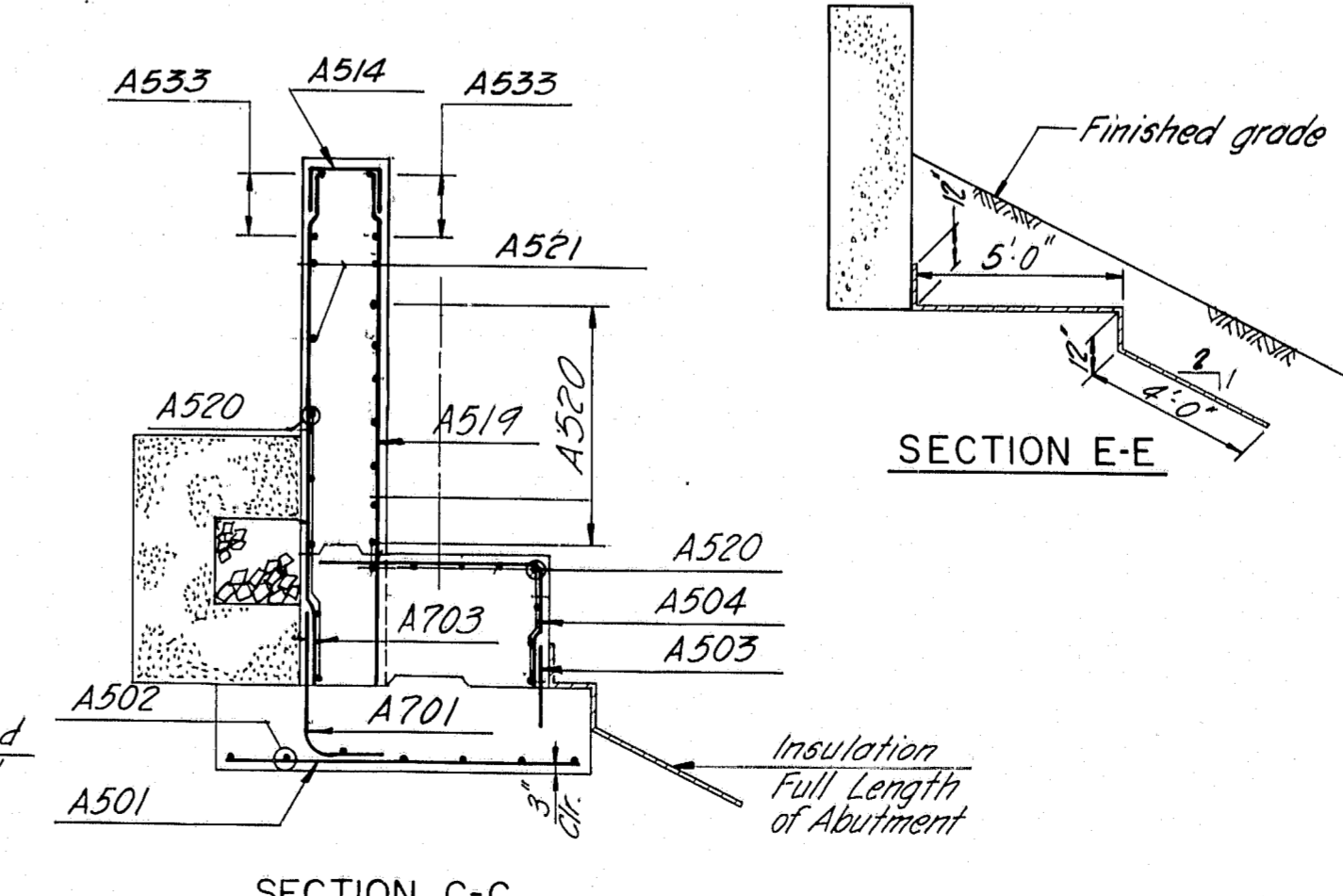
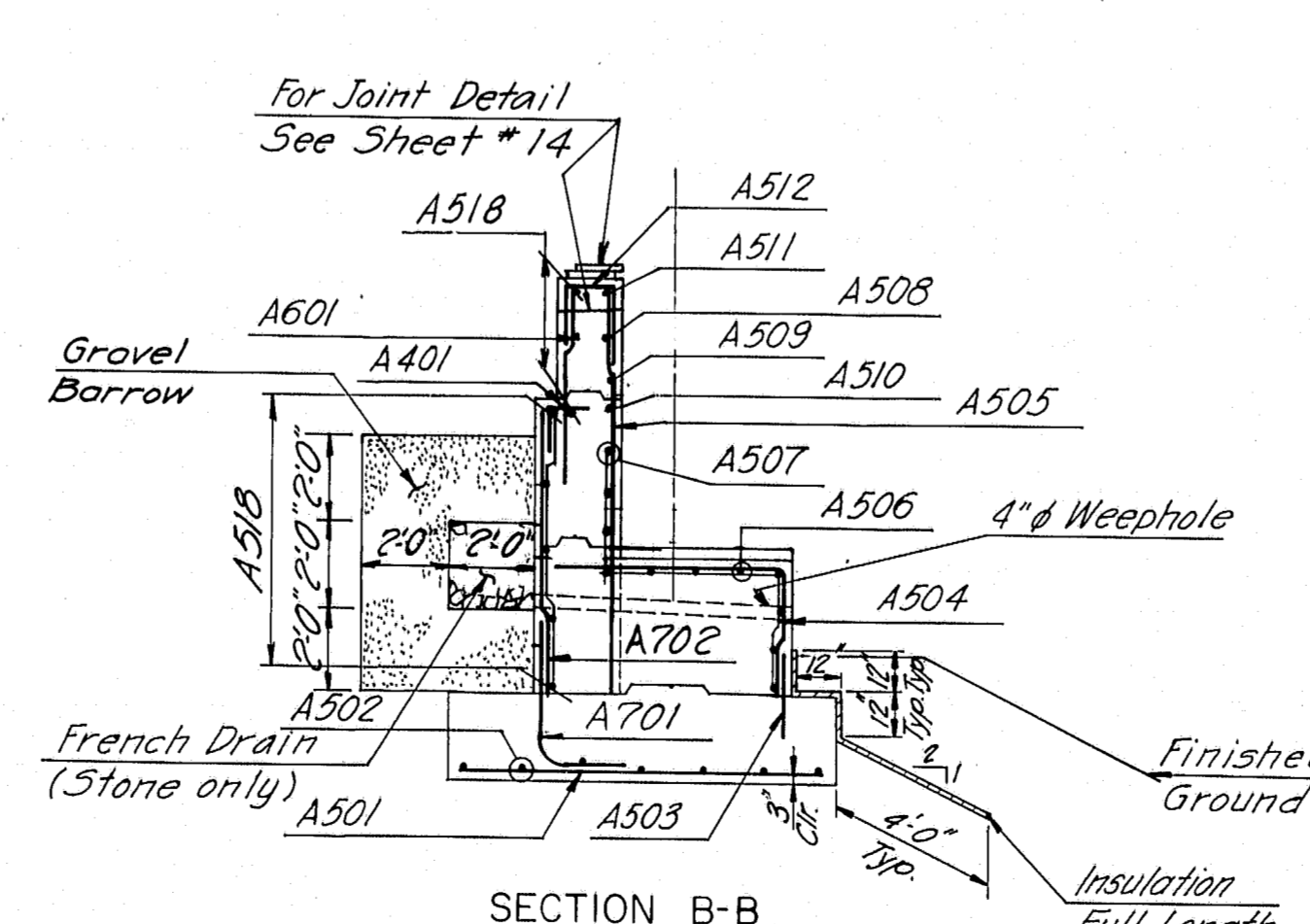
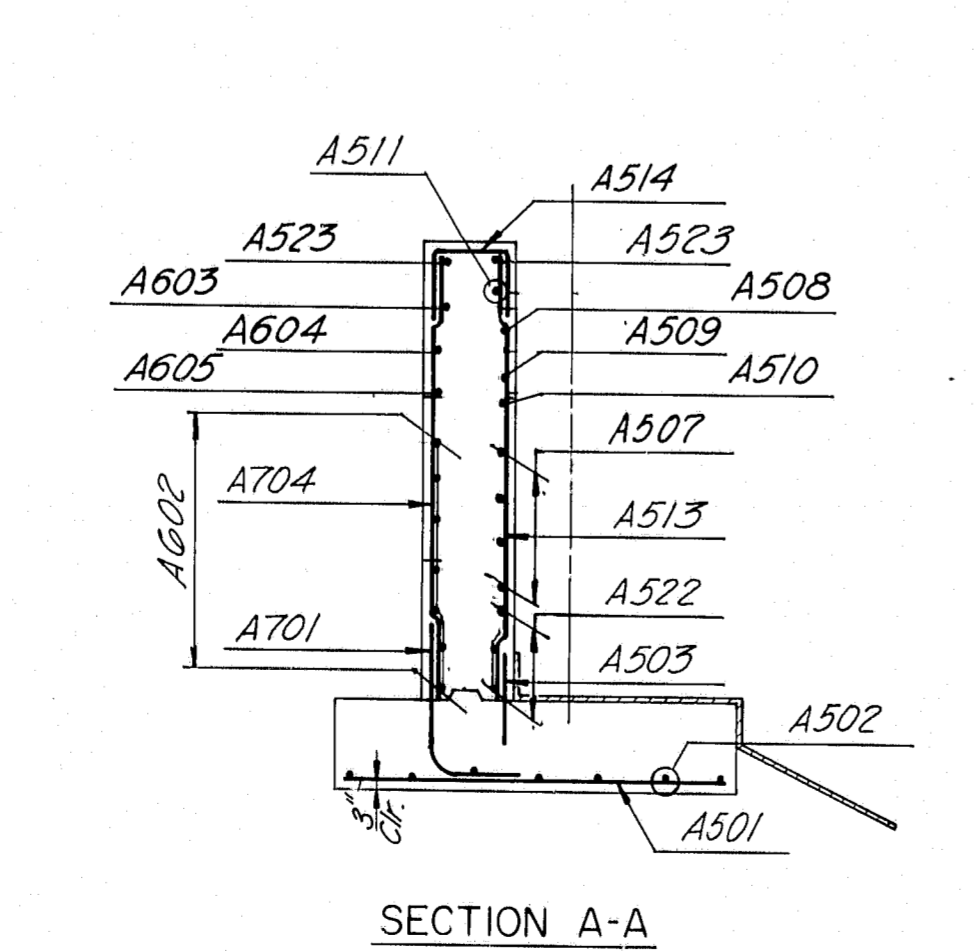
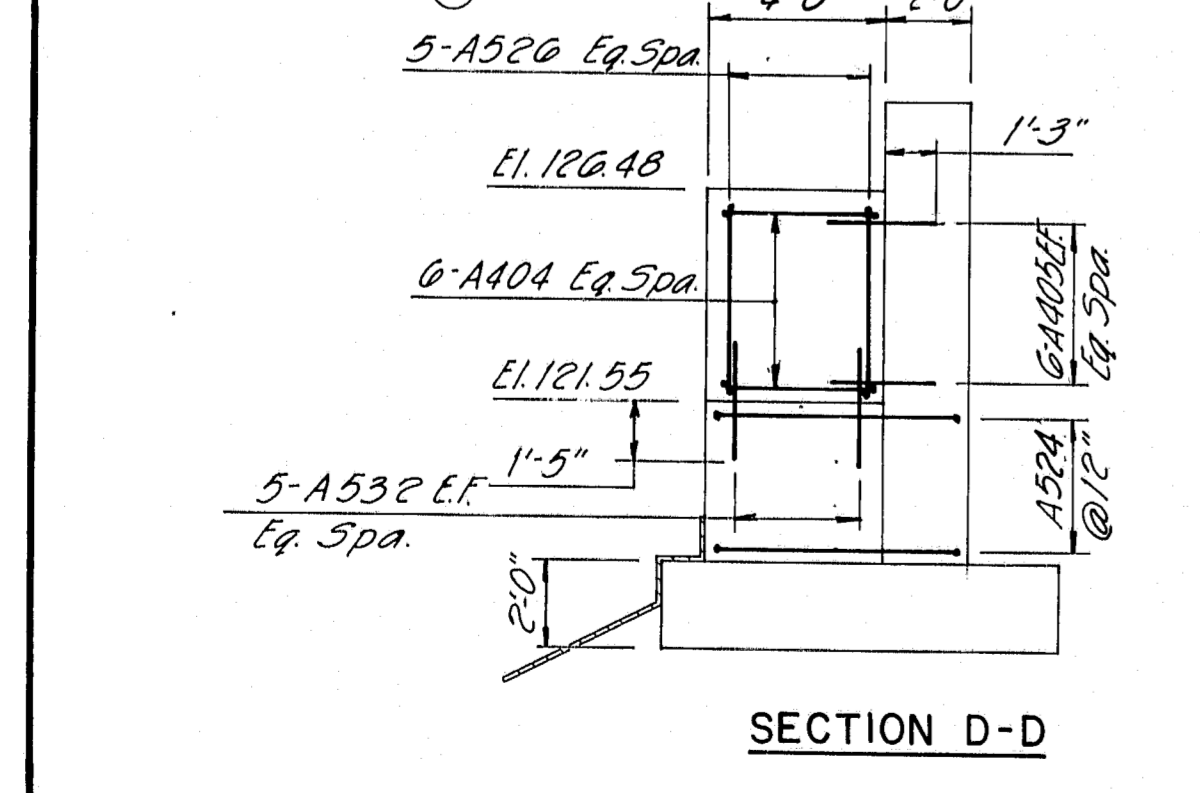
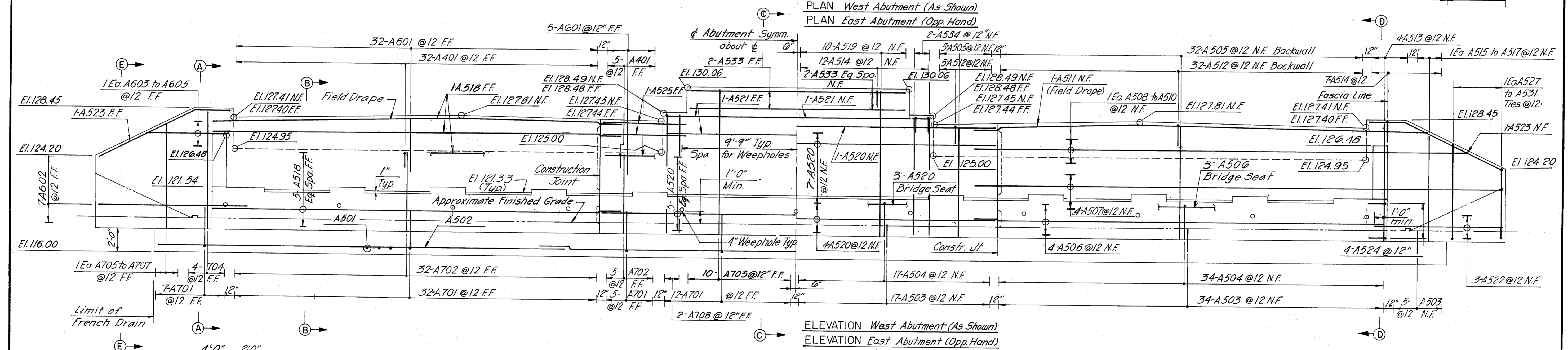
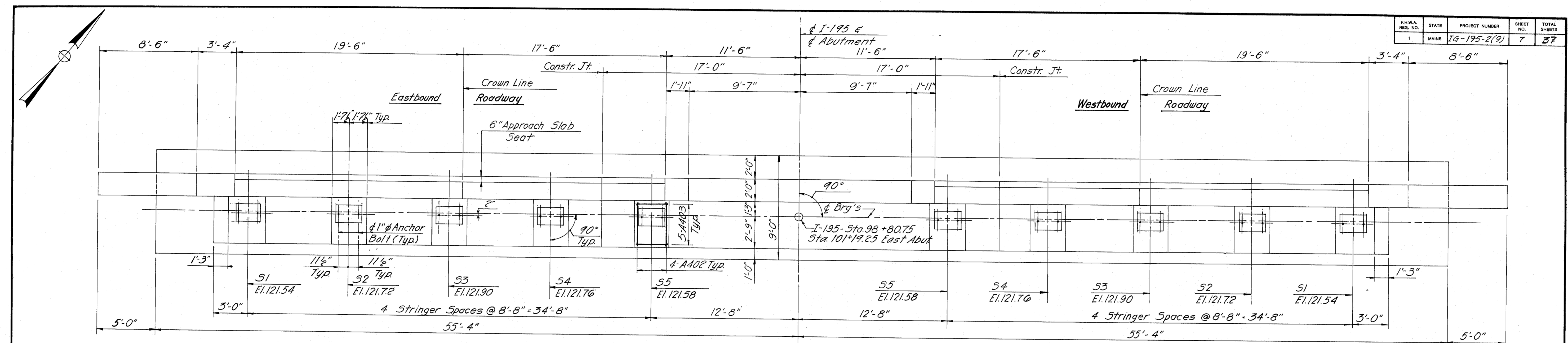
NO.	REVISION	BY	DATE	IN CHARGE OF	J.A.E.
		MADE	V.G.	4-81	
		TRACED			
		CHECKED	N.F.O.	4-81	

R92-197

HNTB
HOWARD NEEDLES TAMMEN & BERGENDOFF
CONSULTING ENGINEERS BOSTON

STATE OF MAINE DEPARTMENT OF TRANSPORTATION
ROUTE I-195 OVER MAINE TURNPIKE
FOOTING PLAN
SACO, MAINE
SHEET 6 OF 16 AUGUSTA, MAINE

FEDERAL REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	16-195-2(9)	7	37



NOTES:

- For Abutment Notes and Footing Reinforcement, see Sht. No. 6.
- Insulation to be 2" polystyrene plastic insulation with 20 gauge 6" wire mesh screening on both sides, except on surfaces which are to be in contact with concrete. Insulating panels to be placed on existing embankment whose surface shall be free of rocks which would cause damage to the panels.

NO.	REVISION	BY	DATE
		MADE	S.A. 4-81
		TRACED	
		CHECKED	N.F.O. 4-81
		IN CHARGE OF J.A.E.	

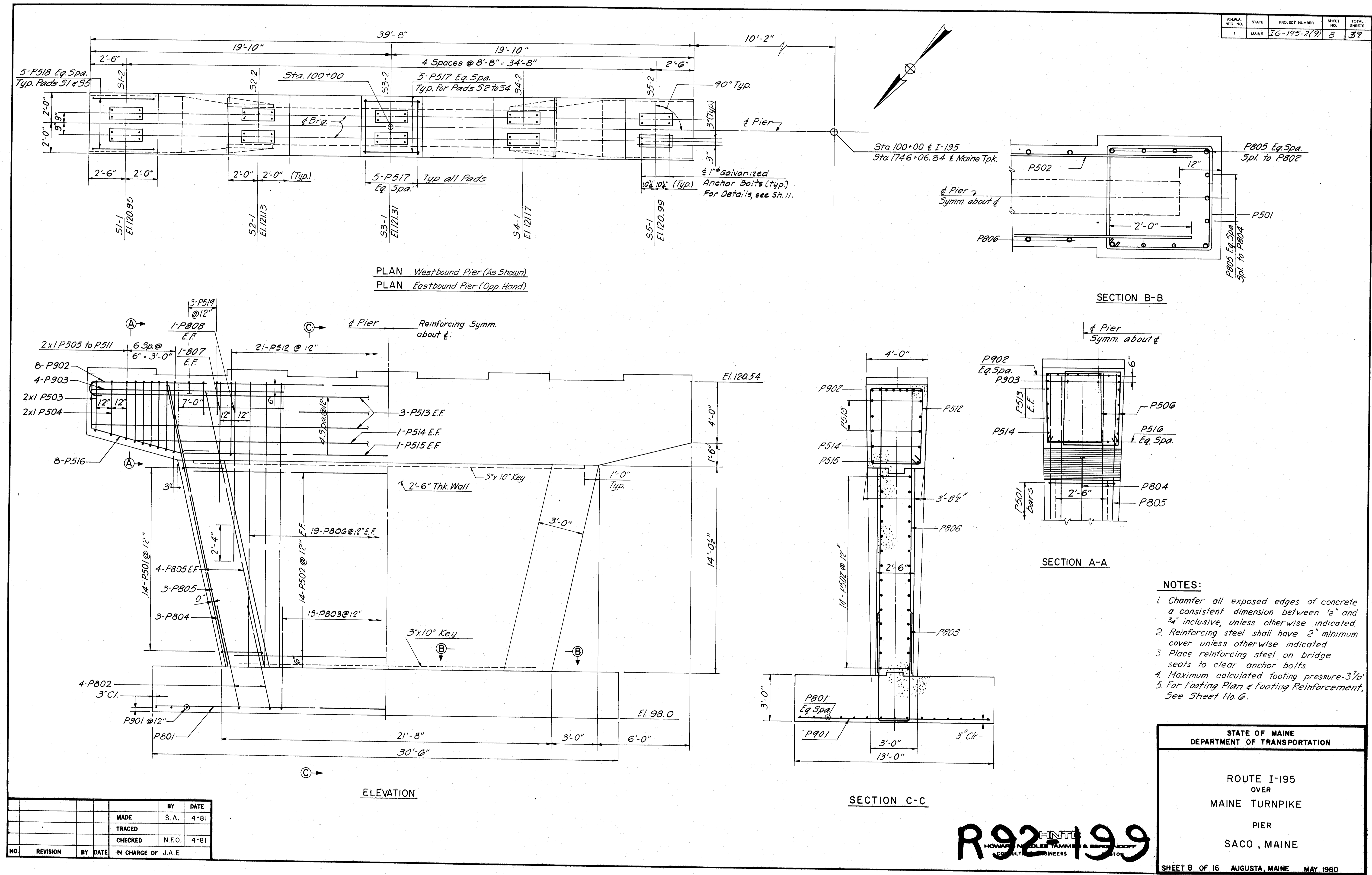
R92198

**STATE OF MAINE
DEPARTMENT OF TRANSPORTATION**

ROUTE I-195
OVER
MAINE TURNPIKE
ABUTMENTS
SACO, MAINE

SHEET 7 OF 16 AUGUSTA, MAINE JUNE 1980

FORMA. REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	IG-195-2(9)	8	37



- NOTES:**
1. Chamfer all exposed edges of concrete a consistent dimension between 1/2" and 3/4" inclusive, unless otherwise indicated.
 2. Reinforcing steel shall have 2" minimum cover unless otherwise indicated.
 3. Place reinforcing steel on bridge seats to clear anchor bolts.
 4. Maximum calculated footing pressure - 3 1/2'.
 5. For Footing Plan & Footing Reinforcement, See Sheet No. 6.

NO.	REVISION	BY DATE	IN CHARGE OF
		MADE BY DATE	
		TRACED S.A. 4-81	
		CHECKED N.F.O. 4-81	

R92199

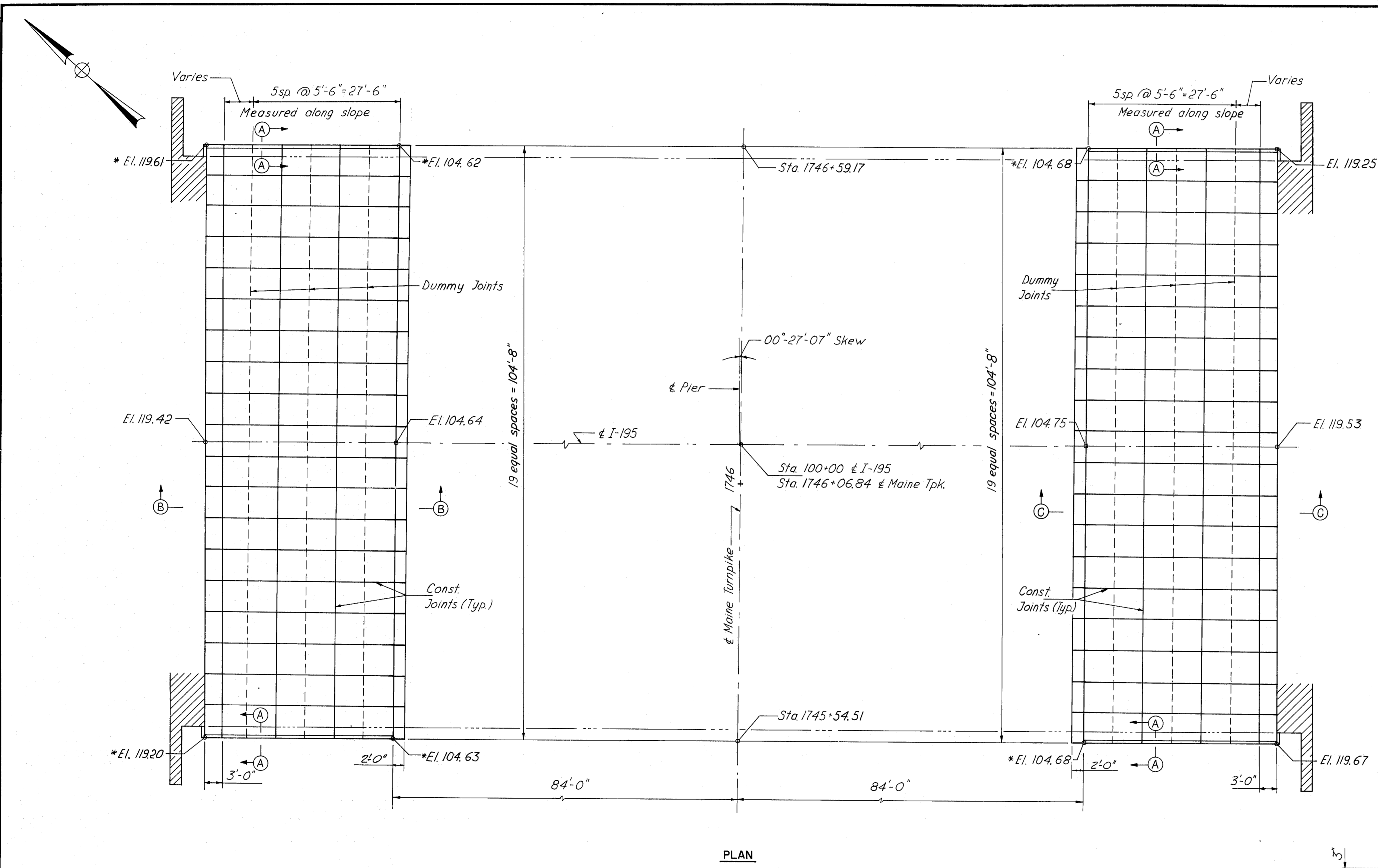
STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

ROUTE I-195
OVER
MAINE TURNPIKE

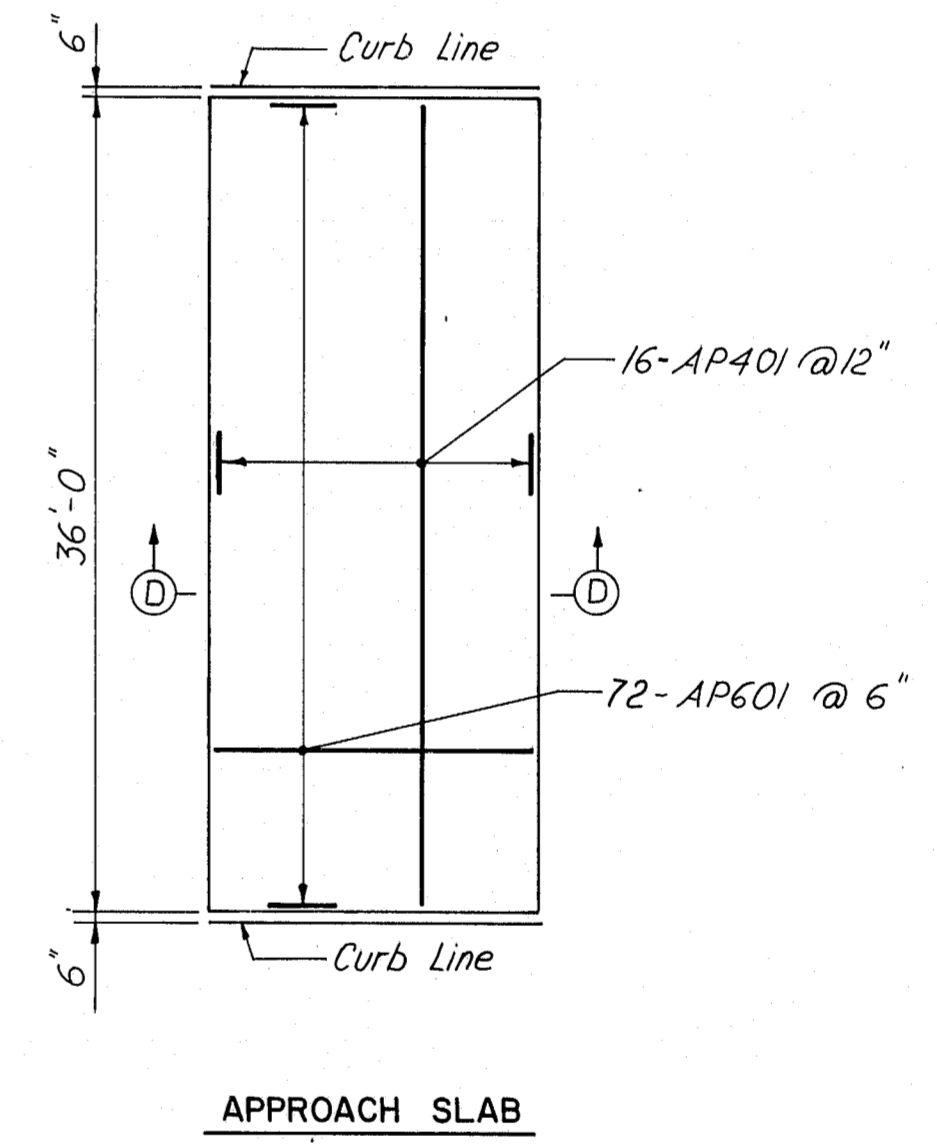
PIER
SACO, MAINE

SHEET 8 OF 16 AUGUSTA, MAINE MAY 1980

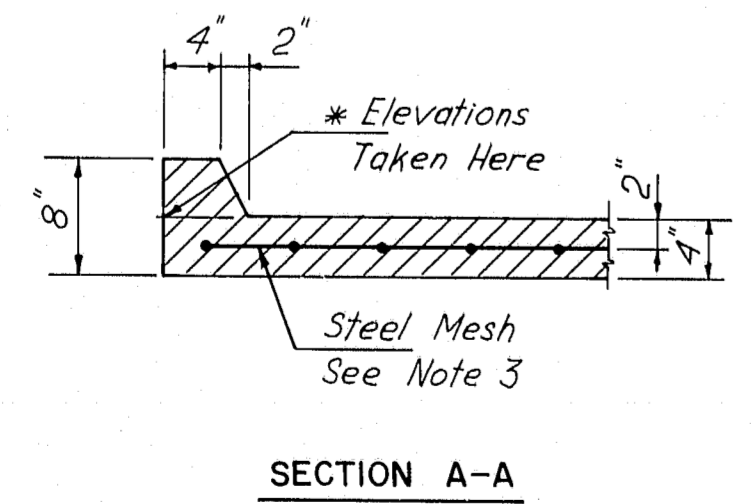
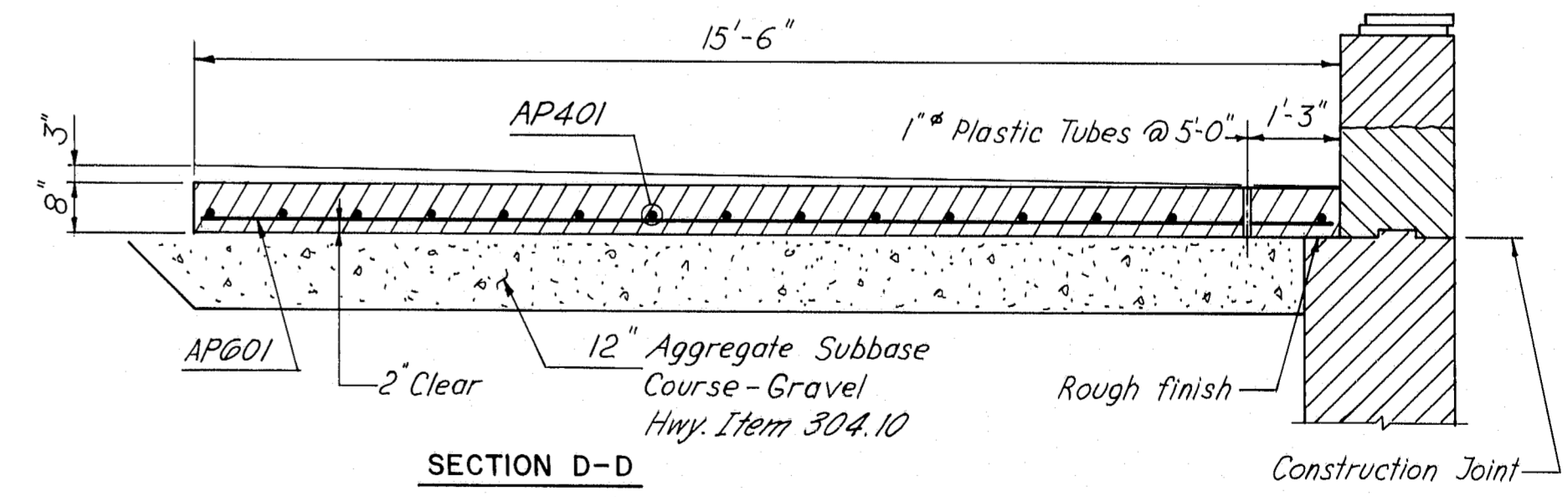
FIG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	I-195-2(9)	9	37



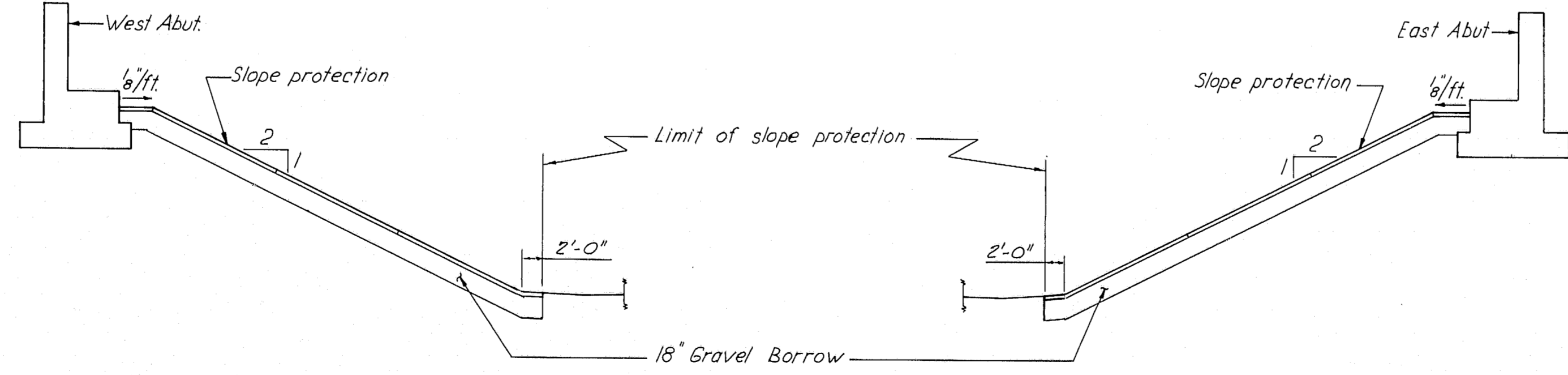
- NOTES:**
1. The 18" of Gravel Borrow under Slope Protection may be reduced or omitted if in the opinion of the Engineer the existing material is suitable.
 2. Break band at construction joints with a coat of asphalt paint.
 3. Reinforce with #10 gage 6"x6" steel mesh, not to pass through construction joints.
 4. Dummy joints shall be made with a groover to a depth of 1/4".
 5. Edges of construction joints shall be finished with a sidewalk edging tool to a depth of 1/4".



APPROACH SLAB
Approach Slab at West Abut shown.
Approach Slab at East Abut similar.



SECTION A-A



SECTION B-B

SECTION C-C

NO.	REVISION	BY	DATE	IN CHARGE OF	J.A.E.
		MADE	V.G.	4-81	
		TRACED			
		CHECKED	N.F.O.	4-81	

R92200

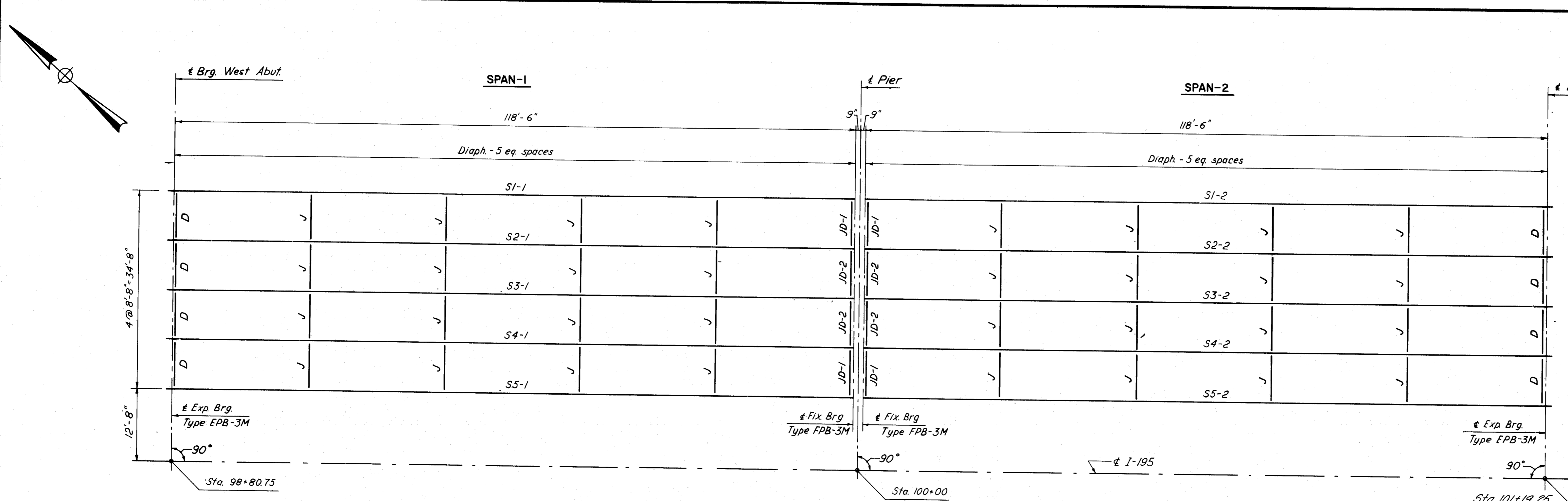
STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

ROUTE I-195
OVER
MAINE TURNPIKE

SLOPE PROTECTION
APPROACH SLAB
SACO, MAINE

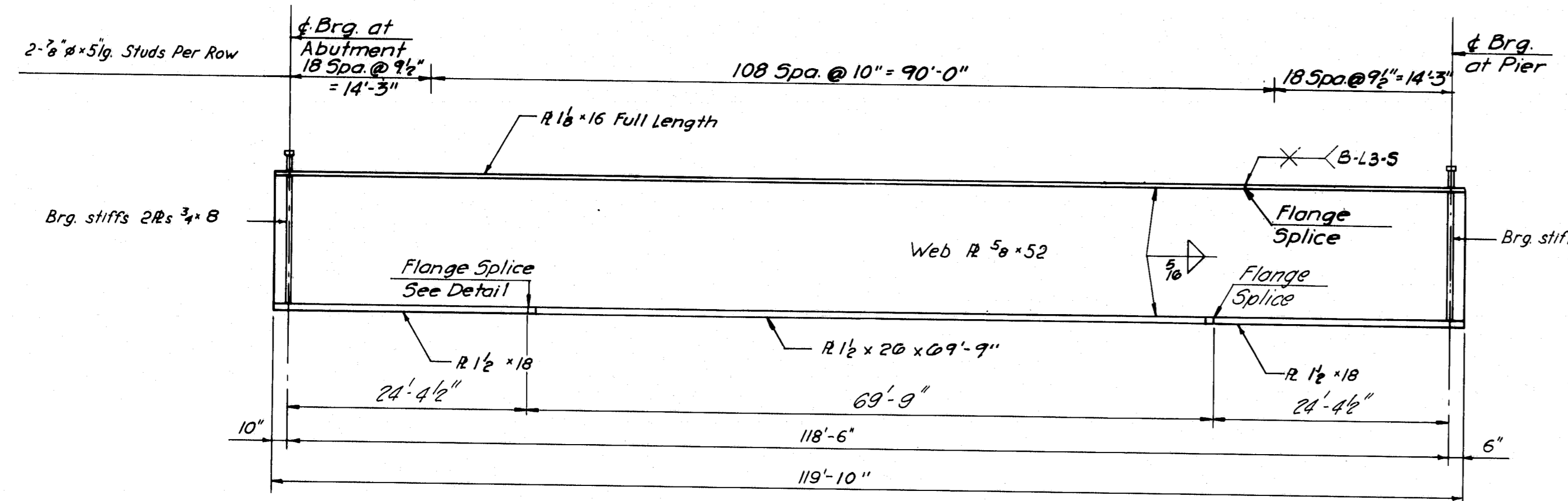
SHEET 9 OF 16 AUGUSTA, MAINE

FHWA REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	I-195-2(9)	10	37



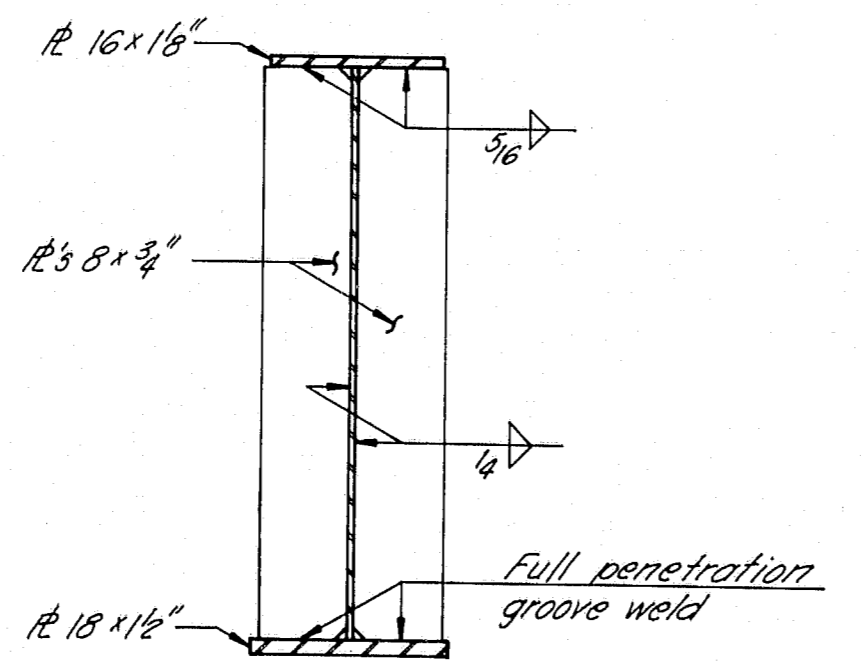
- STRUCTURAL STEEL NOTES:**
- No transverse butt weld splices will be allowed in the flange plates or web plates within 10 feet from the point of maximum positive moment.
 - Butt weld splices in flanges shall be not less than one foot from transverse welds in the web plate.
 - Bearing stiffeners shall be plumb after erection and dead loading of the structure.
 - Cross frame or diaphragm connection plates may be either plumb or normal to the top flange.
 - For shear connector details see std. drawing BD 104-77.
 - For details of JD-1 & JD-2 diaphragms see Sheet 11.
 - For details of diaphragms D, cross frames J, bearing stiff. and conn. P's, see Std. Dwg. BD 113-78.
 - All structural steel to conform to ASTM A36, f_y 20,000 psi, except as follows:
Girder Flange P's to be ASTM A572 Grade 50 f_y 27,000 psi.
Bridge Joints to be ASTM A588, f_y 27,000 psi.
Bolts to be ASTM A325, f_v 25,000 psi.

FRAMING PLAN
Westbound Roadway Shown
Eastbound Roadway Similar

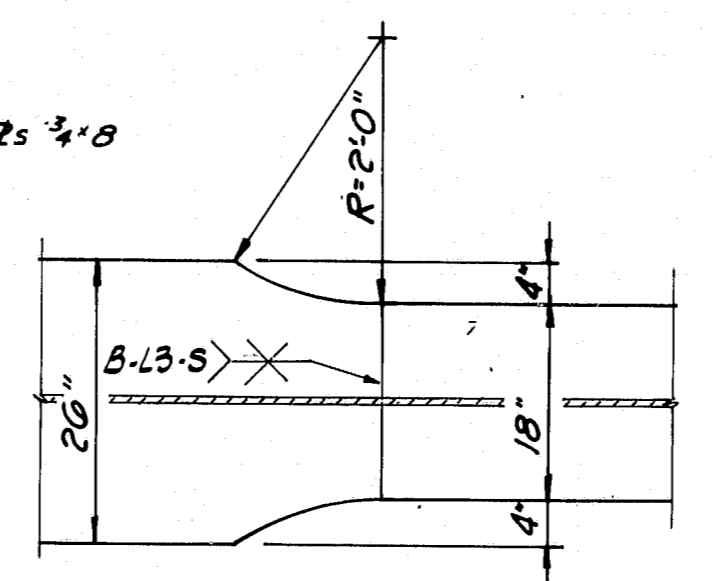


STRINGER ELEVATION
Typ. for all stringers

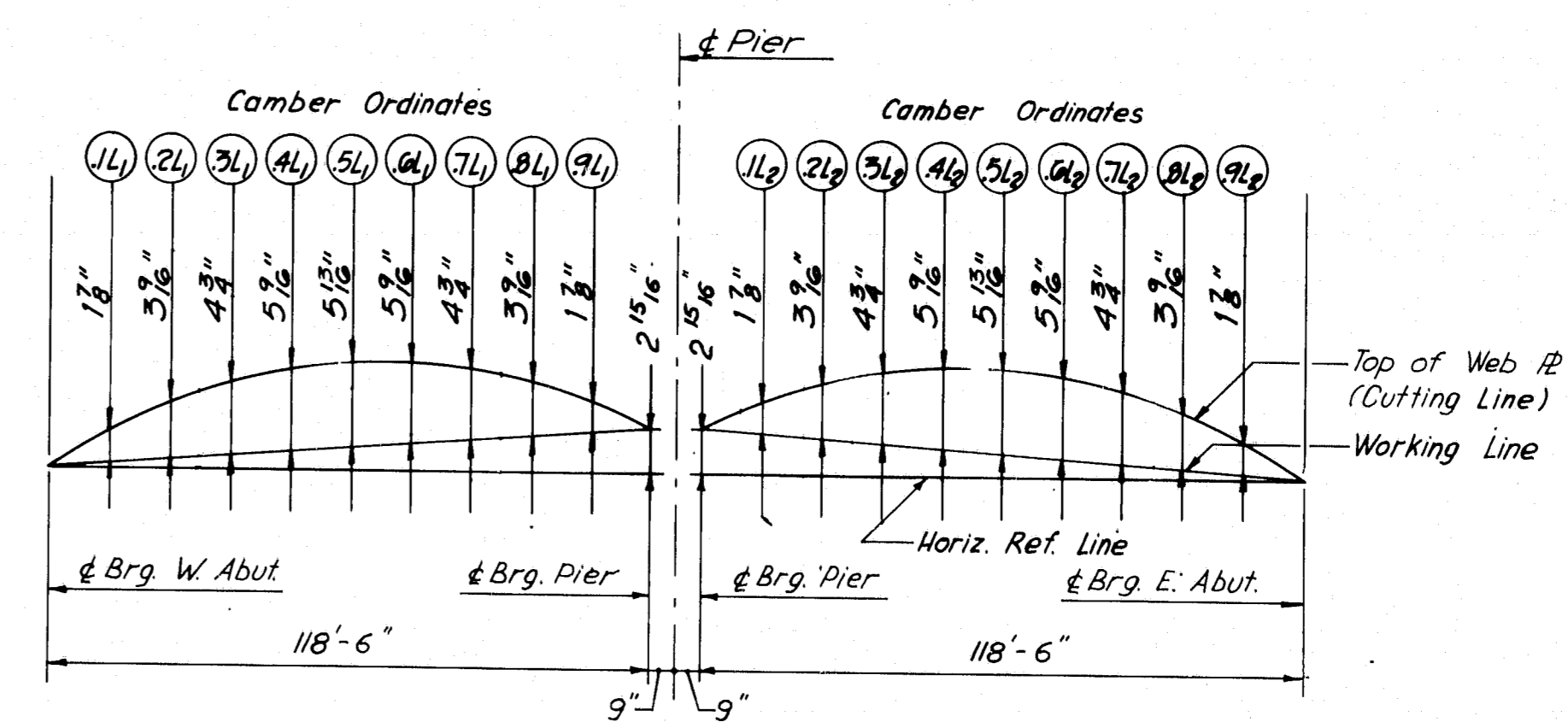
Total shear connectors required = 290 x 10 girders = 2900
Total weight of shear connectors = 2,842 lbs.



BEARING STIFFENER DETAIL



BOTTOM FLANGE SPLICE



CAMBER DIAGRAM

NOTE:
Camber ordinates shown include total dead load deflection and vertical curvature. All dimensions are horizontal or vertical.

NO.	REVISION	BY	DATE
		MADE	V.G. 4-81
		TRACED	
		CHECKED	N.F.O. 4-81
		IN CHARGE OF	J.A.E.

R92-201

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CONSULTING ENGINEERS
BOSTON

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

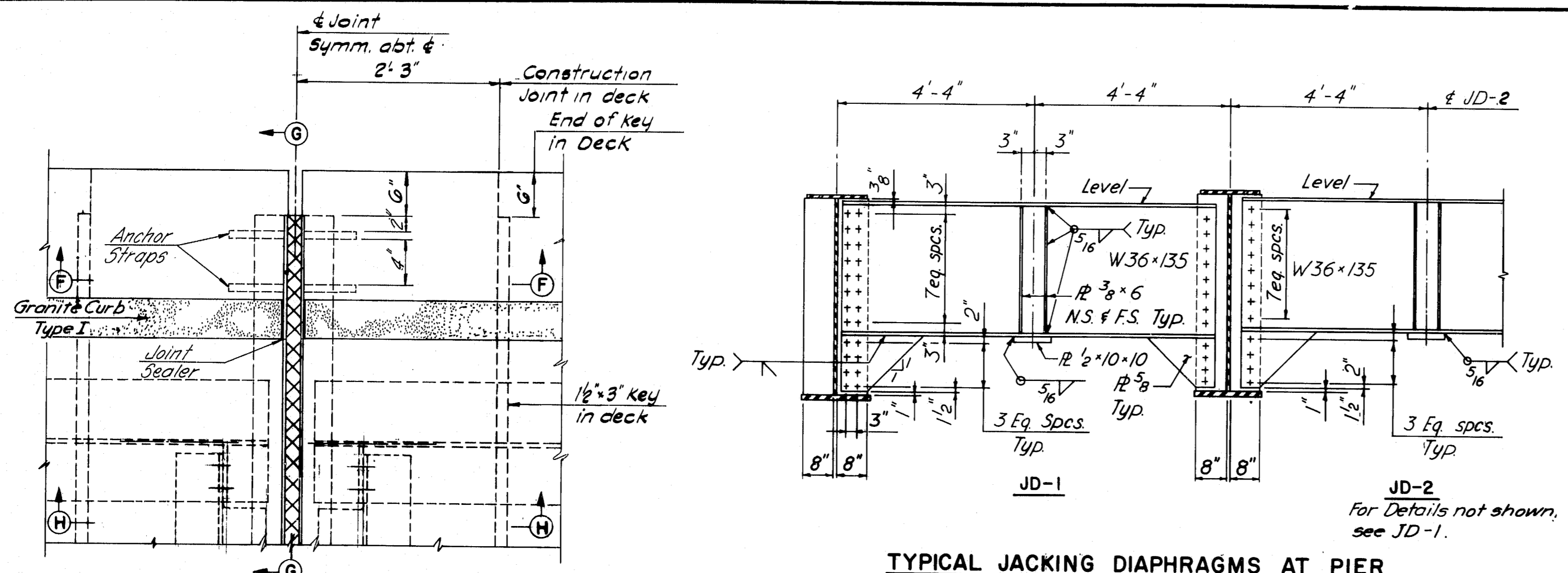
ROUTE I-195
OVER
MAINE TURNPIKE

FRAMING PLAN & DETAILS

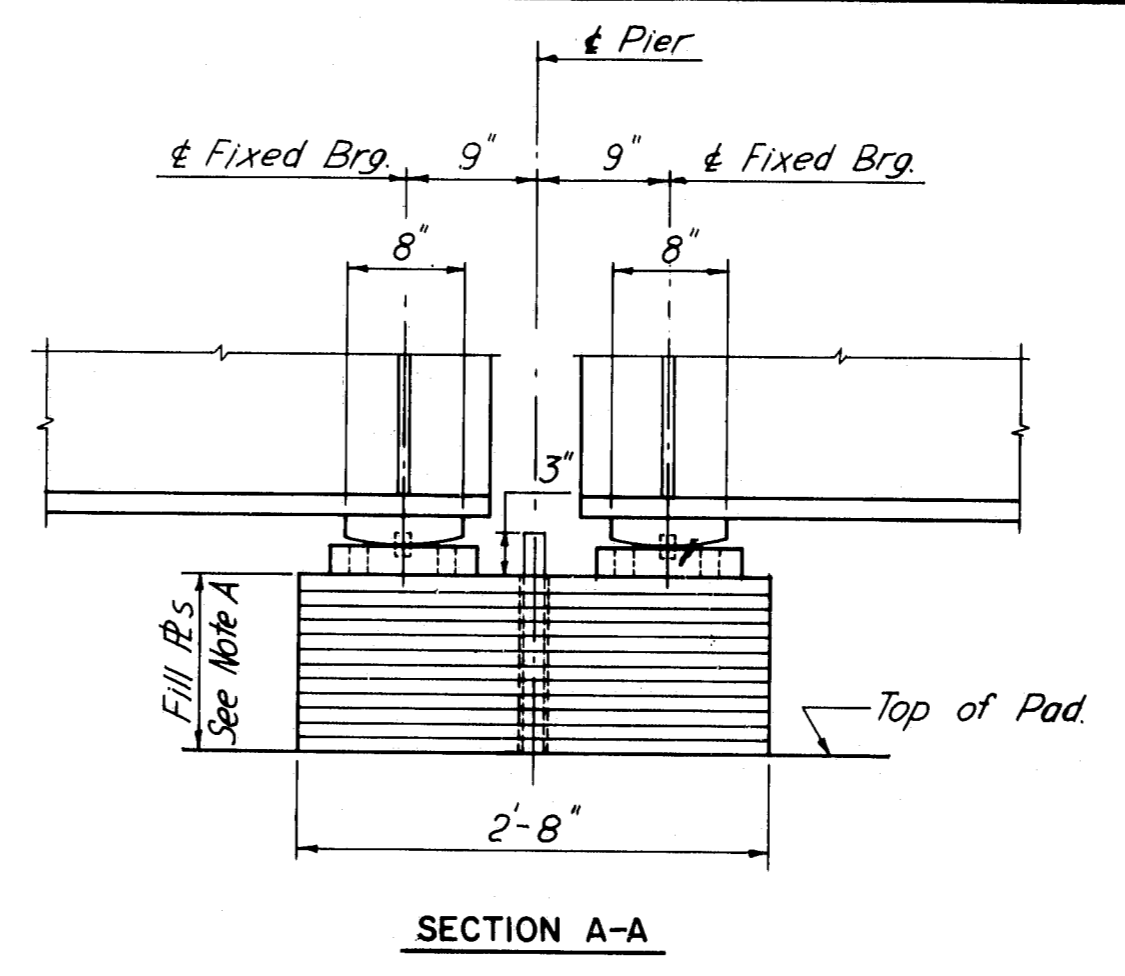
SACO, MAINE

SHEET 10 OF 16 AUGUSTA, MAINE

PANA	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
MADE	16-195-2(9)	11	37	

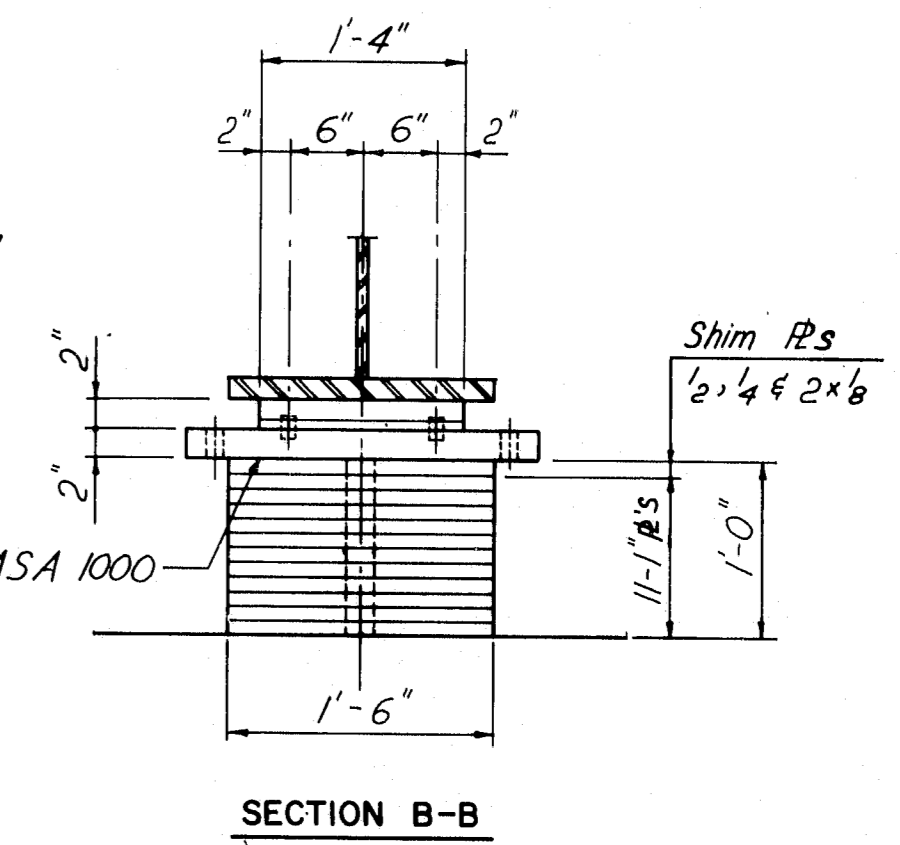


TYPICAL JACKING DIAPHRAGMS AT PIER



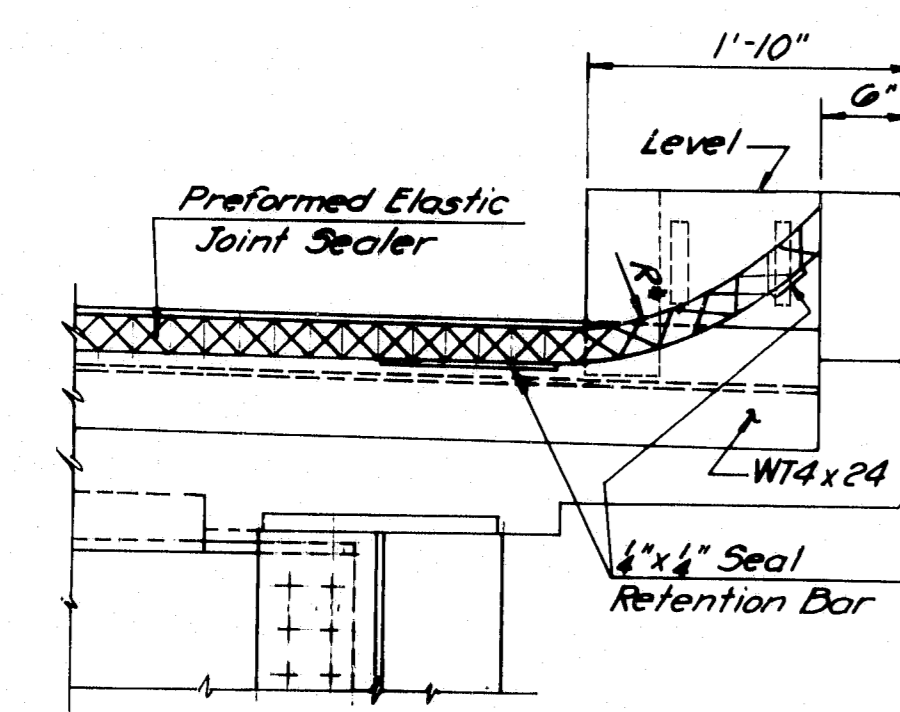
SECTION A-A

NOTE A:
Top & Bottom of each fill plate to be planed, straightened or otherwise treated to secure true level surfaces. Fill R's are for future adjustment as may be necessary to compensate for anticipated differential settlement between Pier & Abut. e.g. whenever differential settlement is 2" at the Abut. remove two 1" fill R's of the Pier.

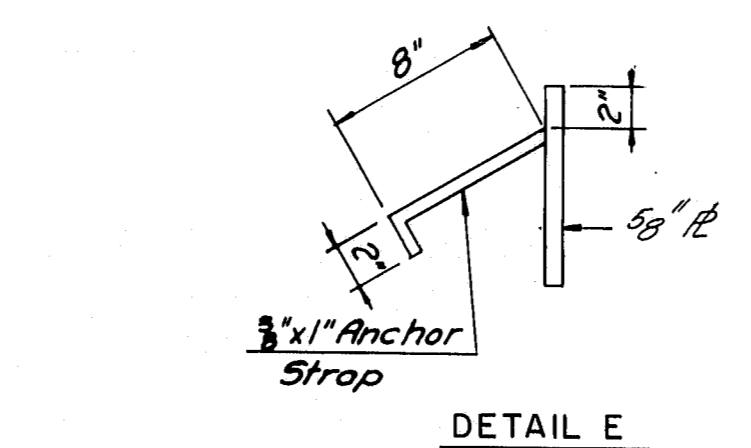


SECTION B-B

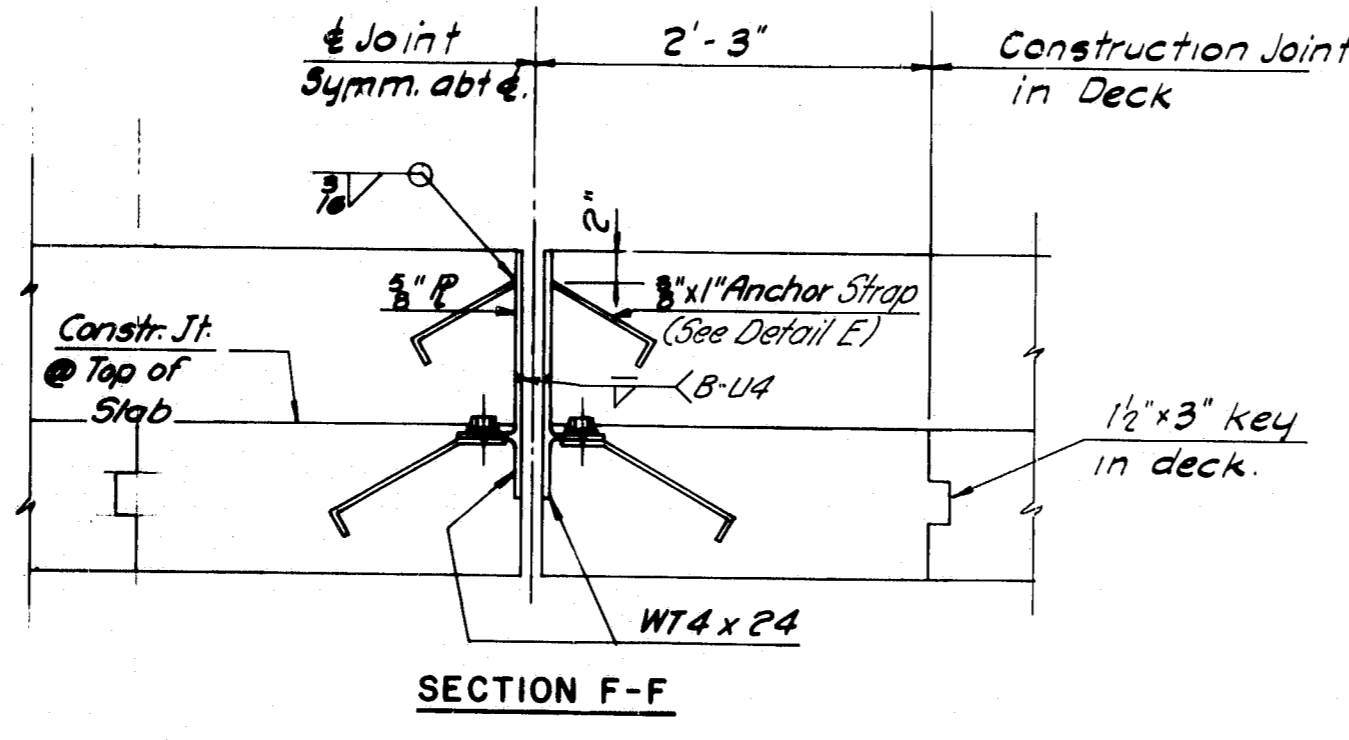
PLAN
All exposed welds and welds in contact with preformed elastic joint sealer to be ground smooth.



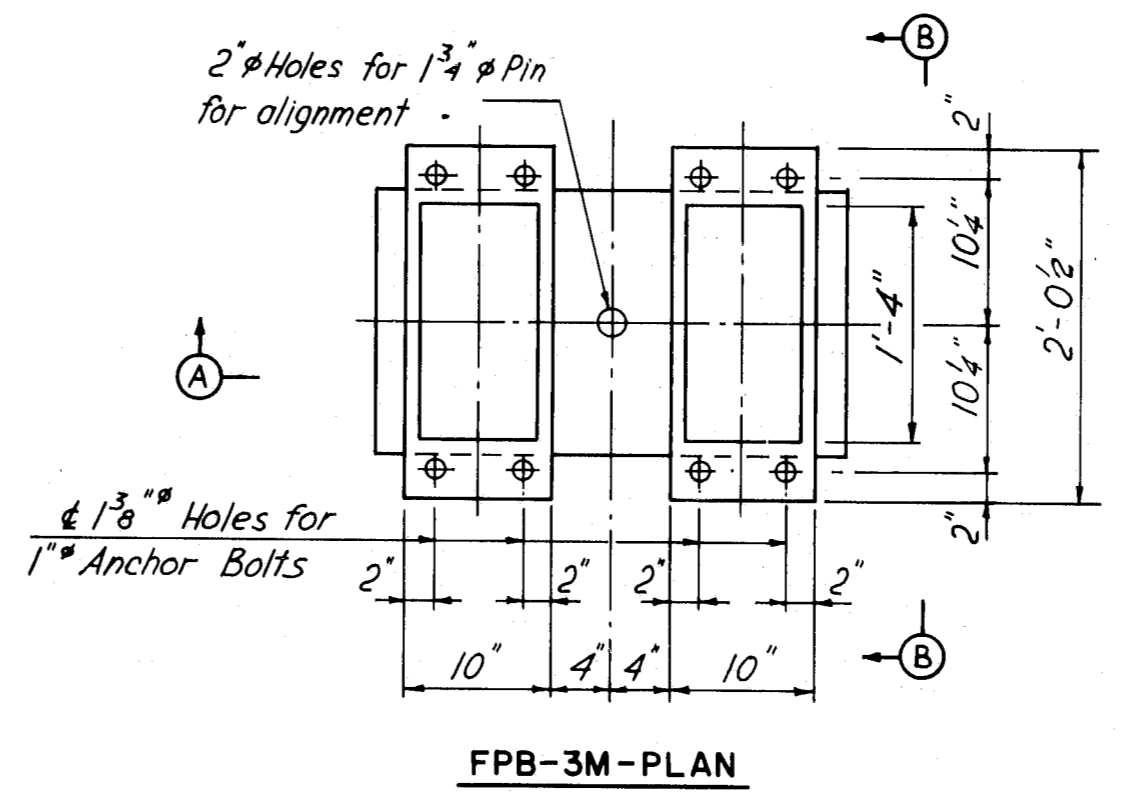
SECTION G-G
Radius R, to be large enough to prevent cutting, crimping or breaking joint sealer.



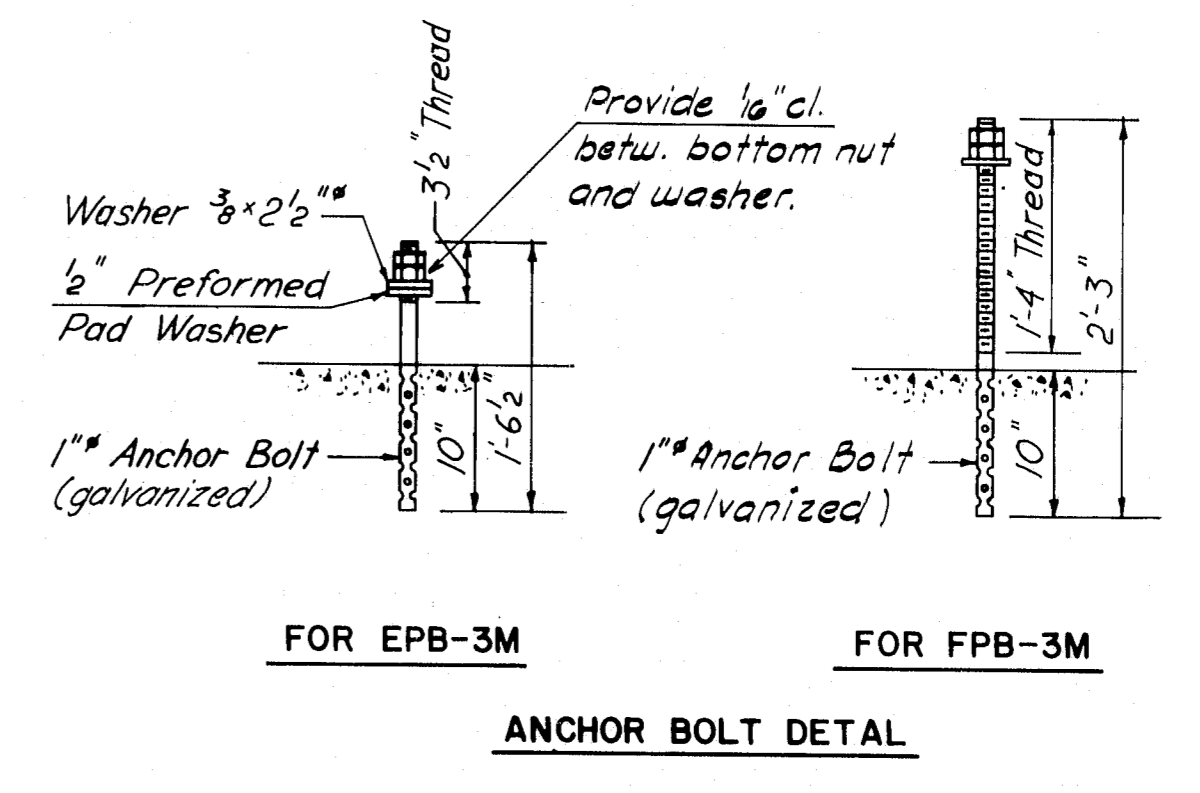
DETAIL E



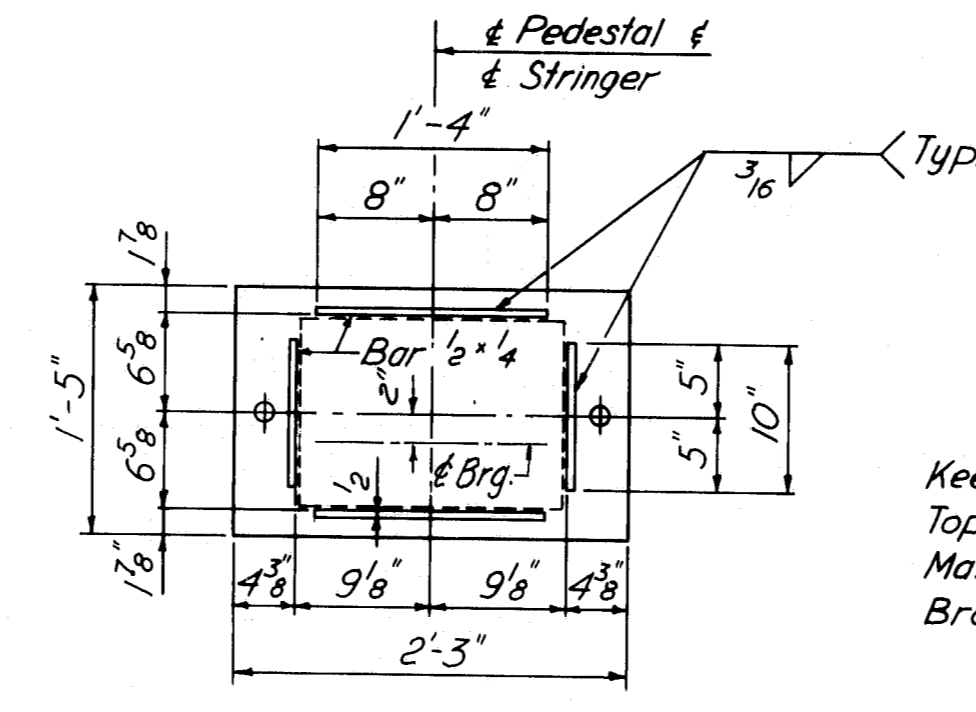
SECTION F-F



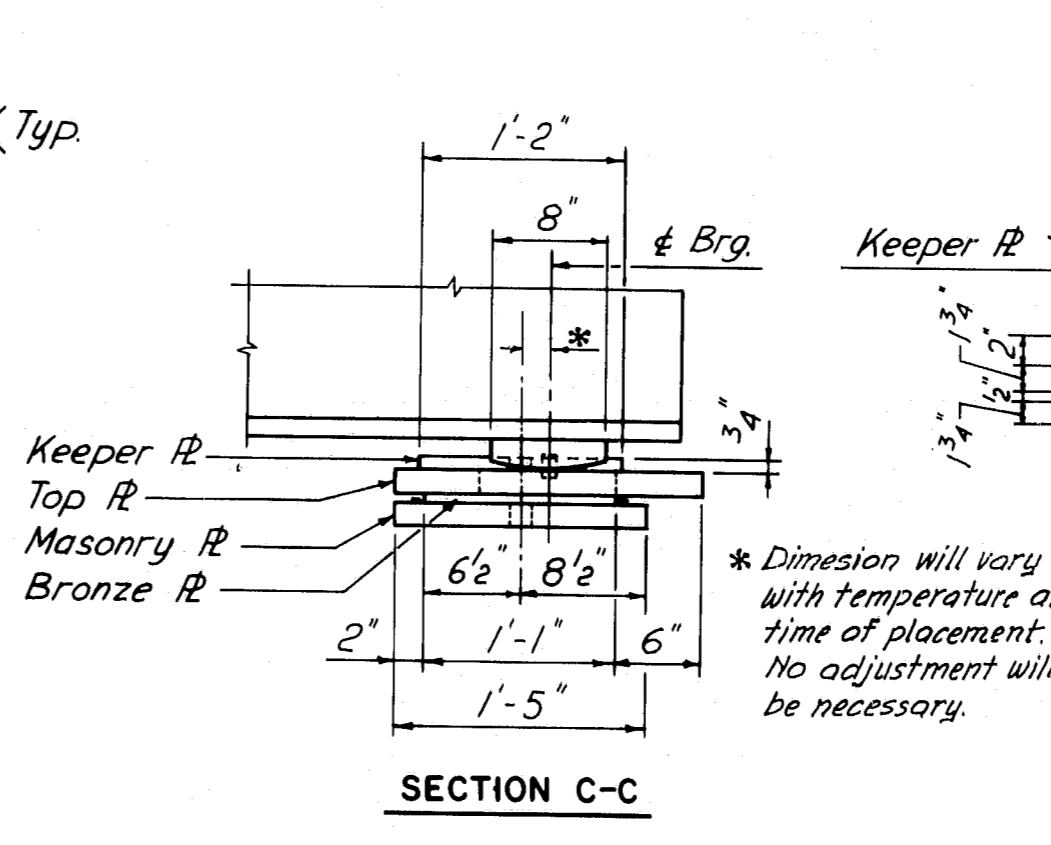
FPB-3M-PLAN



ANCHOR BOLT DETAIL

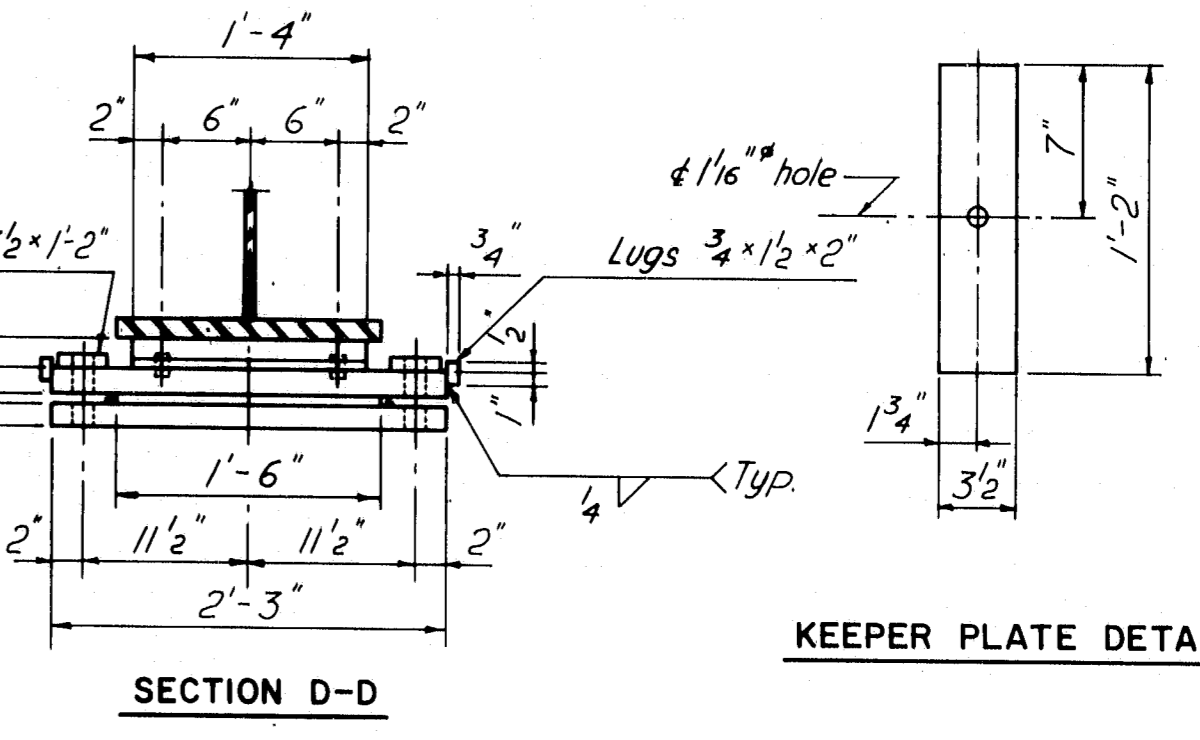


MASONRY PLATE - PLAN



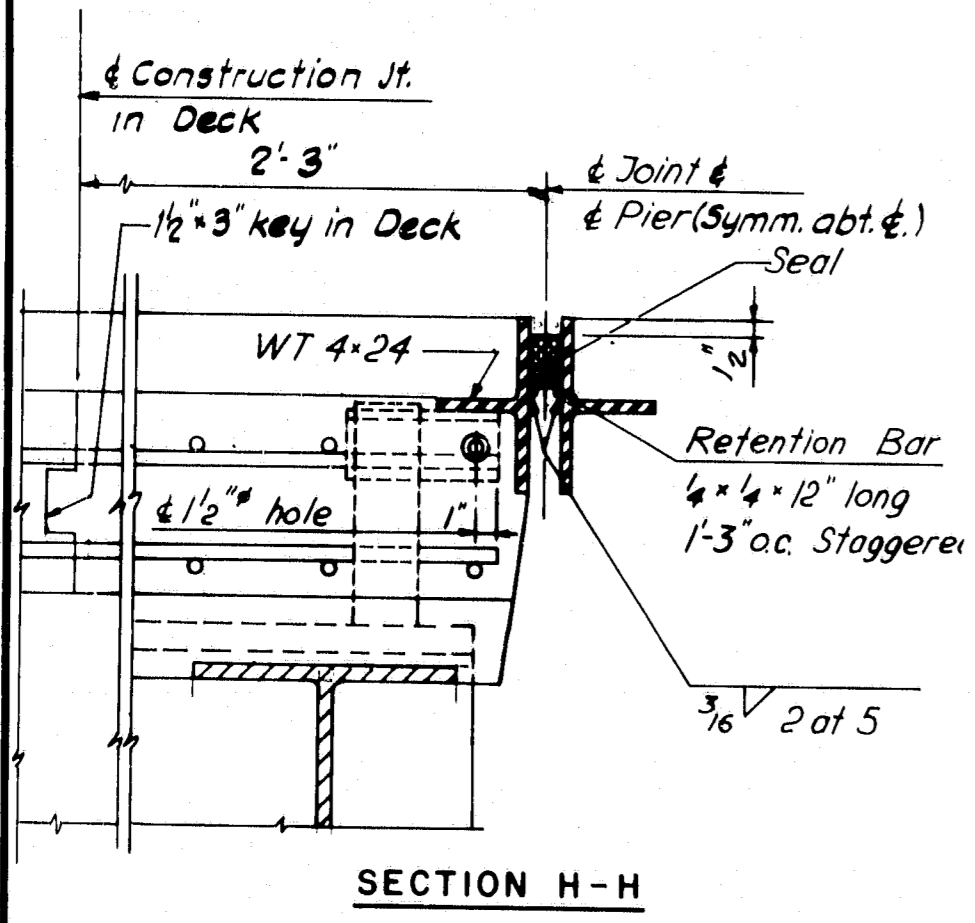
SECTION C-C

*Dimension will vary with temperature at time of placement. No adjustment will be necessary.



SECTION D-D

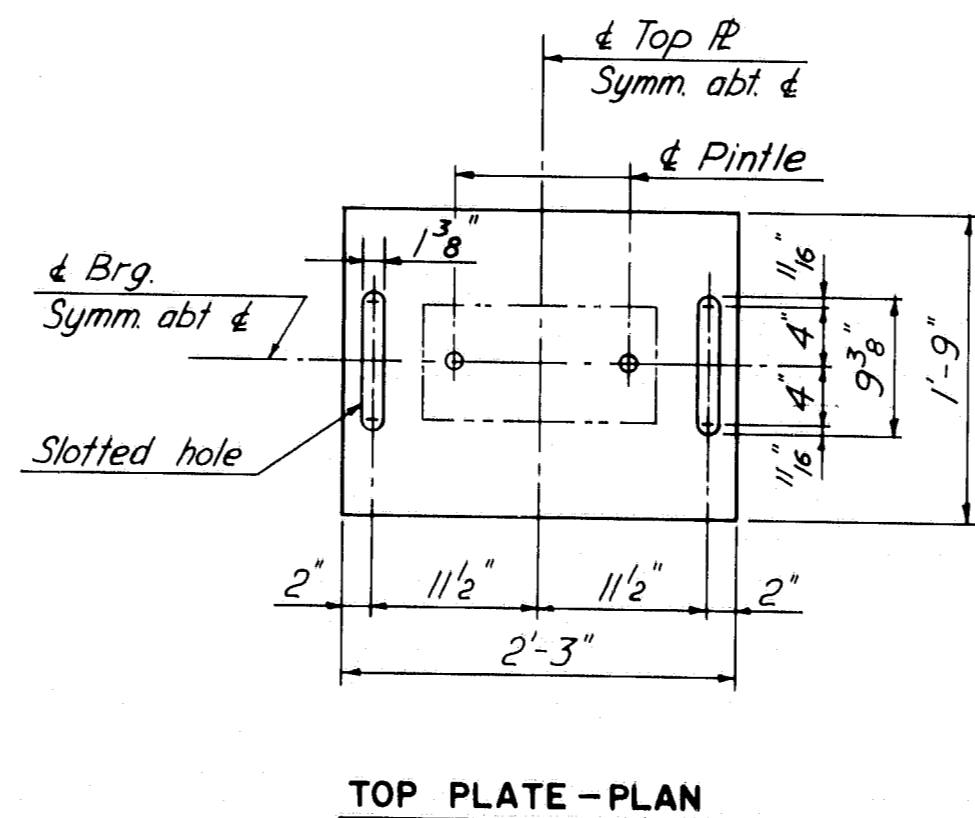
KEEPER PLATE DETAIL



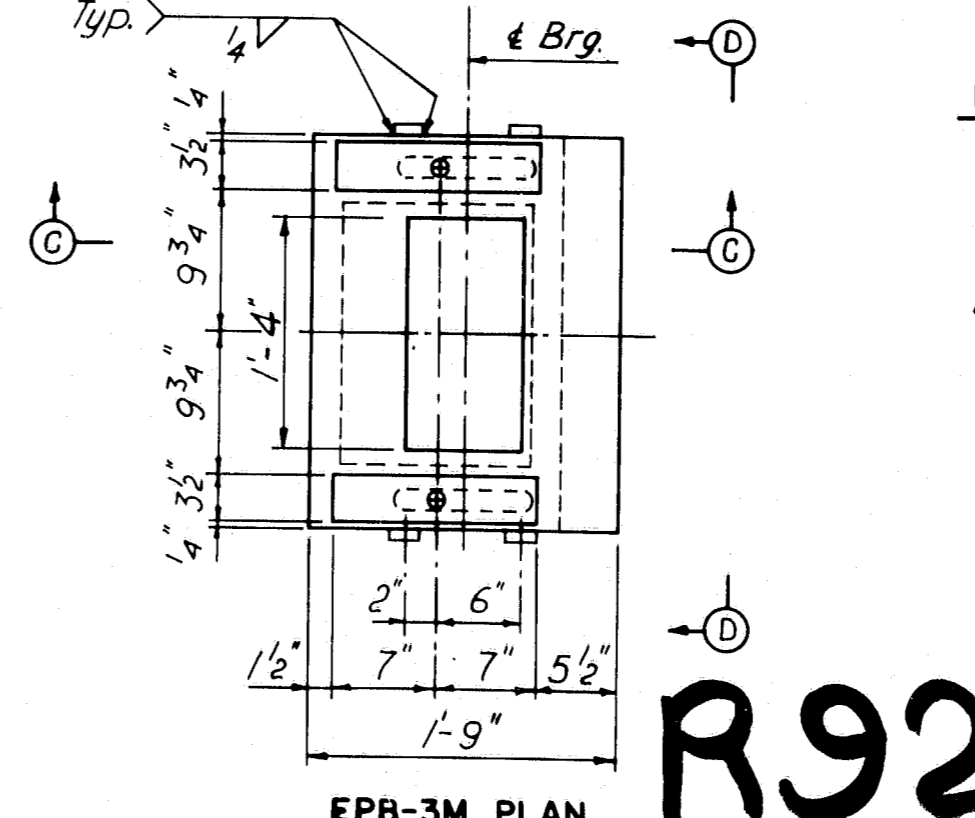
SECTION H-H

- JOINT NOTES:**
- The seal furnished shall have a minimum movement rating of 1".
 - The movement rating noted is for design only and is subject to change due to differences in seals as supplied by various manufacturers. Do not use for setting the joint opening during construction.
 - The seal characteristics shall be submitted to the Engineer for approval prior to the fabrication of the Armored Joint.
 - Set joint opening according to the joint opening shown on the approved "Armored Joint" shop detail drawings.
 - The Armored Joint and seal shall be assembled in the shop prior to shipment. The contractor shall be responsible for the proper placement and final opening of the armored joint. Joint to be installed after main part of deck is poured to within 2'-9" of Pier.
 - For additional joint details not shown, see standard Detail Sheet BD 104-77.

ARMORED JOINT UNIT TYPE A AT PIER



TOP PLATE - PLAN



EPB-3M PLAN

STANDARD PEDESTAL MODIFICATION

- NOTES:**
- For dimensions and details not shown, see FPB-3, EPB-3 & anchor bolt detail on Standard Detail Sheet BD 101-74.
 - Bearing pedestal to be placed normal to stringer.
 - For treatment of Bearing Pads, see 5th Detail Sheet BD 101-74.

NO.	REVISION	BY	DATE	IN CHARGE OF	J.A.E.
		MADE	V.G.	4-81	
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		CHECKED	N.F.O.	4-81	

R92-202

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HOWARD NEEDLES TAMMEN & BERENSON
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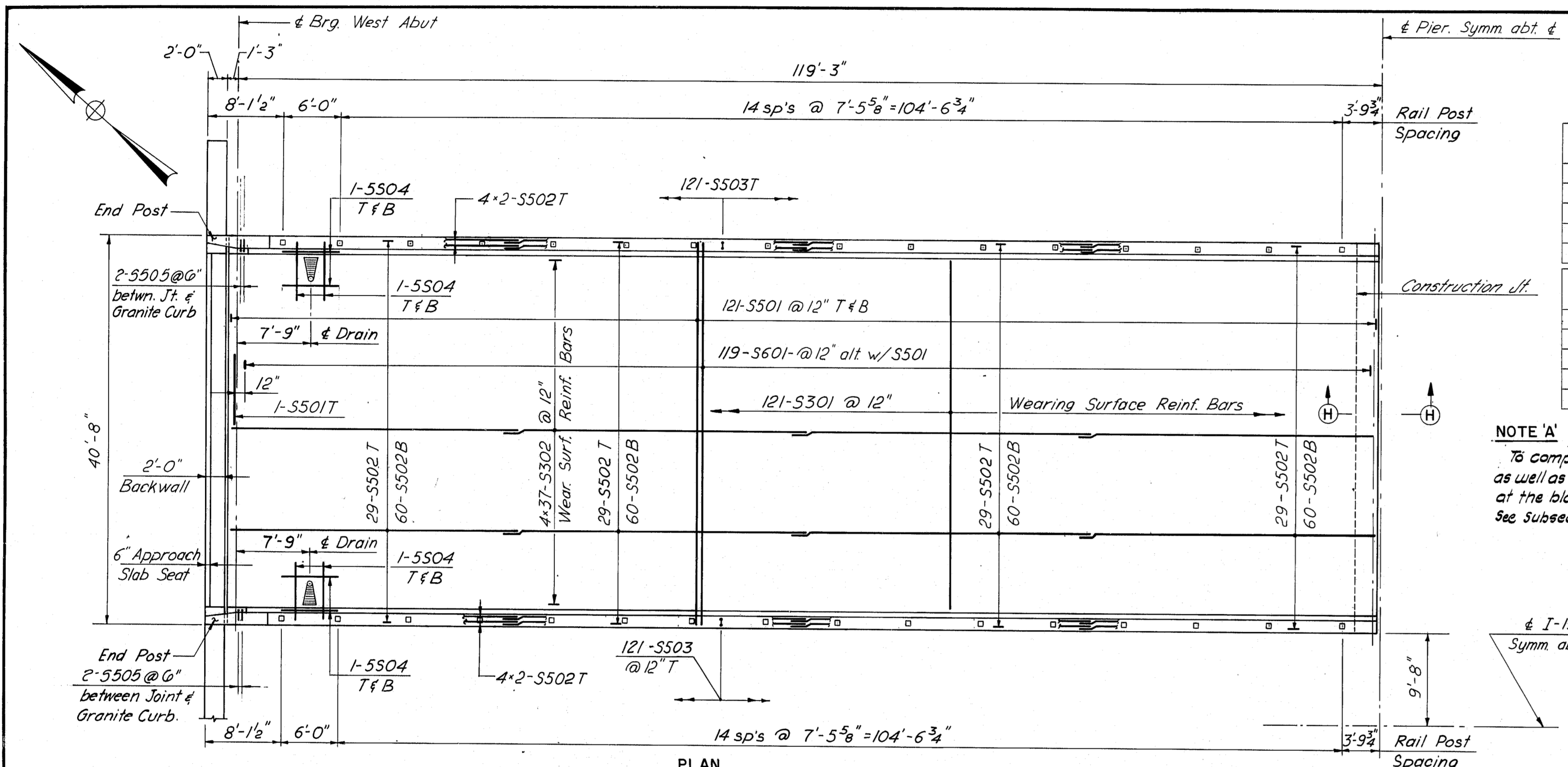
STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

ROUTE I-195
OVER
MAINE TURNPIKE

STRUCTURAL STEEL DETAILS
SACO, MAINE

SHEET 11 OF 16 AUGUSTA, MAINE

F.A.M.A. RES. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	IG-195-2(9)	12	37



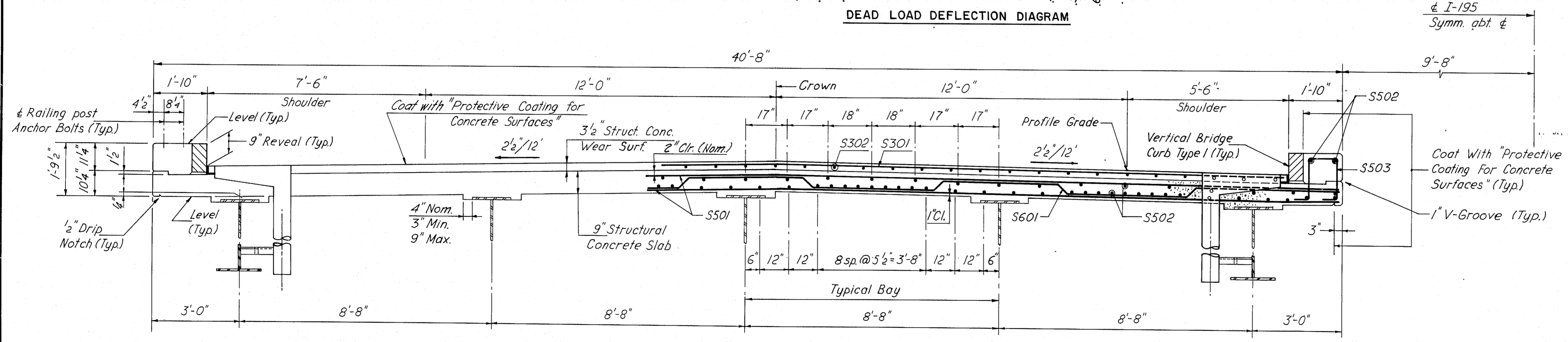
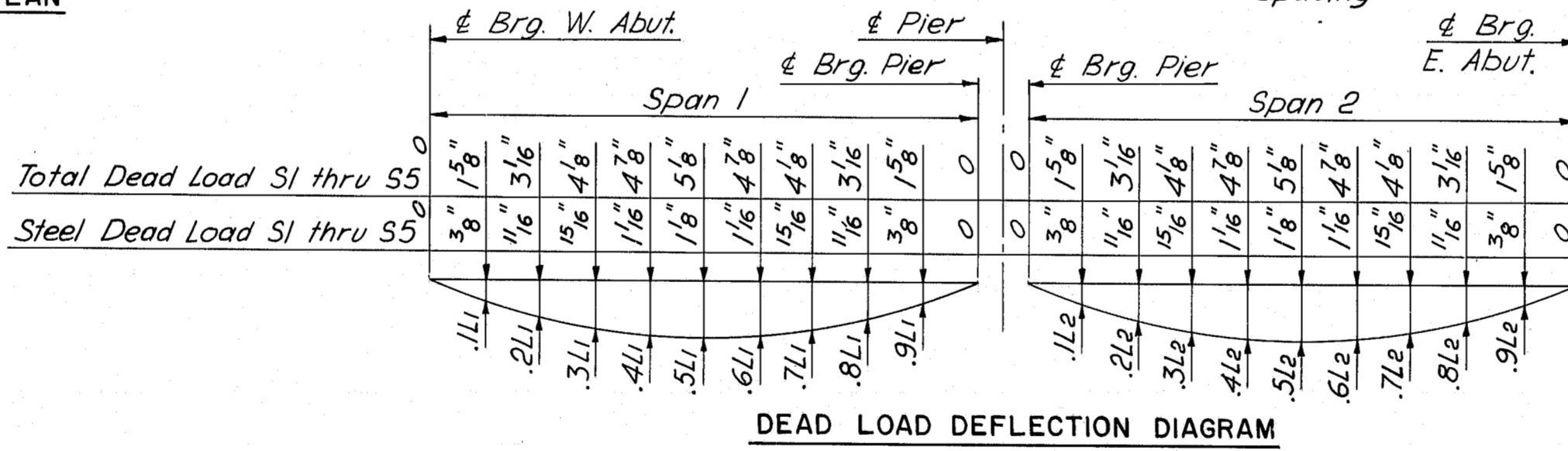
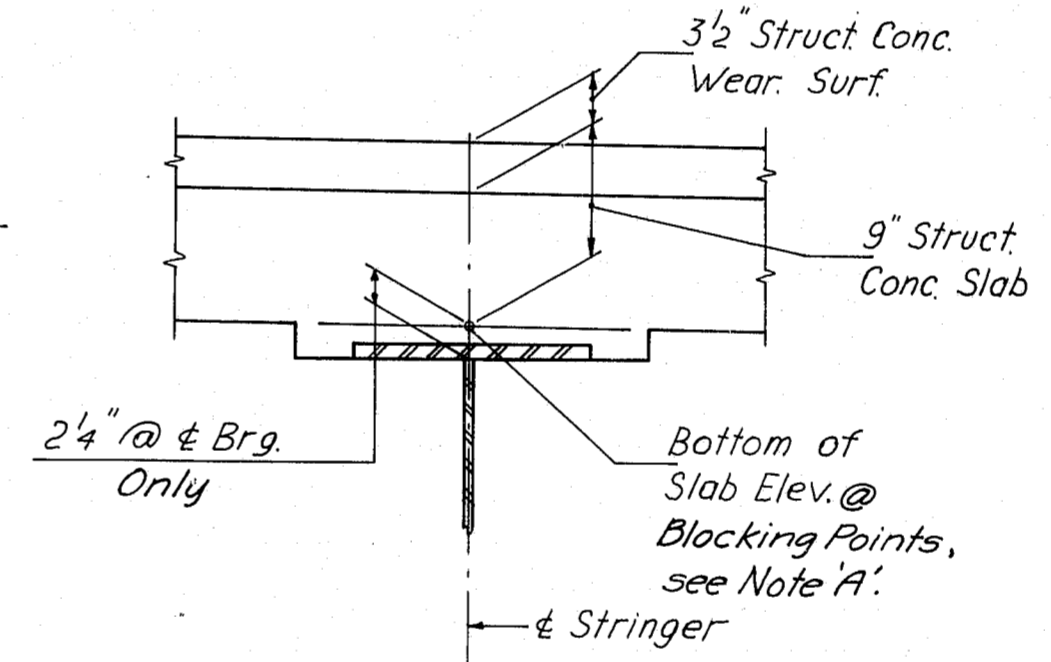
BOTTOM OF SLAB ELEVATIONS AT BLOCKING POINTS

Stringer No	E. Brg. W. Abut.	Span 1									E. Brg. Pier
		.1L1	.2L1	.3L1	.4L1	.5L1	.6L1	.7L1	.8L1	.9L1	
S1	126.69	126.84	126.97	127.08	127.16	127.20	127.21	127.17	127.12	127.03	126.93
S2	126.87	127.02	127.15	127.26	127.34	127.38	127.39	127.35	127.30	127.21	127.11
S3	127.05	127.20	127.33	127.44	127.52	127.56	127.57	127.53	127.48	127.39	127.29
S4	126.91	127.06	127.19	127.30	127.38	127.42	127.43	127.39	127.34	127.25	127.15
S5	126.73	126.88	127.01	127.12	127.20	127.24	127.25	127.21	127.16	127.07	126.97

Stringer No	E. Brg. Pier	Span 2									E. Brg. E. Abut.
		.1L2	.2L2	.3L2	.4L2	.5L2	.6L2	.7L2	.8L2	.9L2	
S1	126.93	127.03	127.12	127.17	127.21	127.20	127.16	127.08	126.97	126.84	126.69
S2	127.11	127.21	127.30	127.35	127.39	127.38	127.34	127.26	127.15	127.02	126.87
S3	127.29	127.39	127.48	127.53	127.57	127.56	127.52	127.44	127.33	127.20	127.05
S4	127.15	127.25	127.34	127.39	127.43	127.42	127.38	127.30	127.19	127.06	126.91
S5	126.97	127.07	127.16	127.21	127.25	127.24	127.20	127.12	127.01	126.88	126.73

NOTE 'A'
 To compensate for all dead load deflections except structural steel, as well as possible irregularities in girders, set the bottom of slab elevations at the blocking points indicated before any of the slab formwork is started. See subsection 502.10(a) of the Standard Specifications.

- SUPERSTRUCTURE NOTES:**
1. Chamfer all exposed edges of concrete a consistent dimension between 1/2" & 3/4" inclusive, unless otherwise indicated.
 2. Form a 1 inch V-groove at the horizontal joint between the curb & slab.
 3. Reinforcing steel to have a minimum cover of 2" unless otherwise indicated.
 4. Mortar for bedding and for joints in the granite curb shall contain an approved non-shrink additive.
 5. The superstructure slab concrete shall be placed continuously and kept plastic until the entire slab concrete has been placed. Set retarding admixture shall be used when authorized by the Engineer.
 6. Adjust reinforcing steel to fit around the drains in a manner approved by the Engineer. Do not cut transverse reinforcing bars.
 7. Protective Coating for Concrete Surfaces shall be applied to the following areas: Top of concrete curb, fascia down to drip notch and concrete wearing surface.



- NOTES:**
1. For additional details & dimensions not shown for curb and roadway see std. detail sheet BD 104-77.
 2. For 2-bar Alum. Bridge Railing details see std. detail sheet BD 114-77.
 3. For End Post Details see sheet No. 13.
 4. For Joint Details see sheet No. 14.
 5. For Reinforcing Steel Schedule see sheets No. 15 & 16.
 6. For Drain Details see sheet BD 104-77.
 7. For Section H-H, see Sht. No. 11.

NO.	REVISION	BY	DATE	IN CHARGE OF	J.A.E.
		MADE	V.G.	4-81	
		TRACED			
		CHECKED	N.F.O.	4-81	

TRANSVERSE SECTION

R92-203

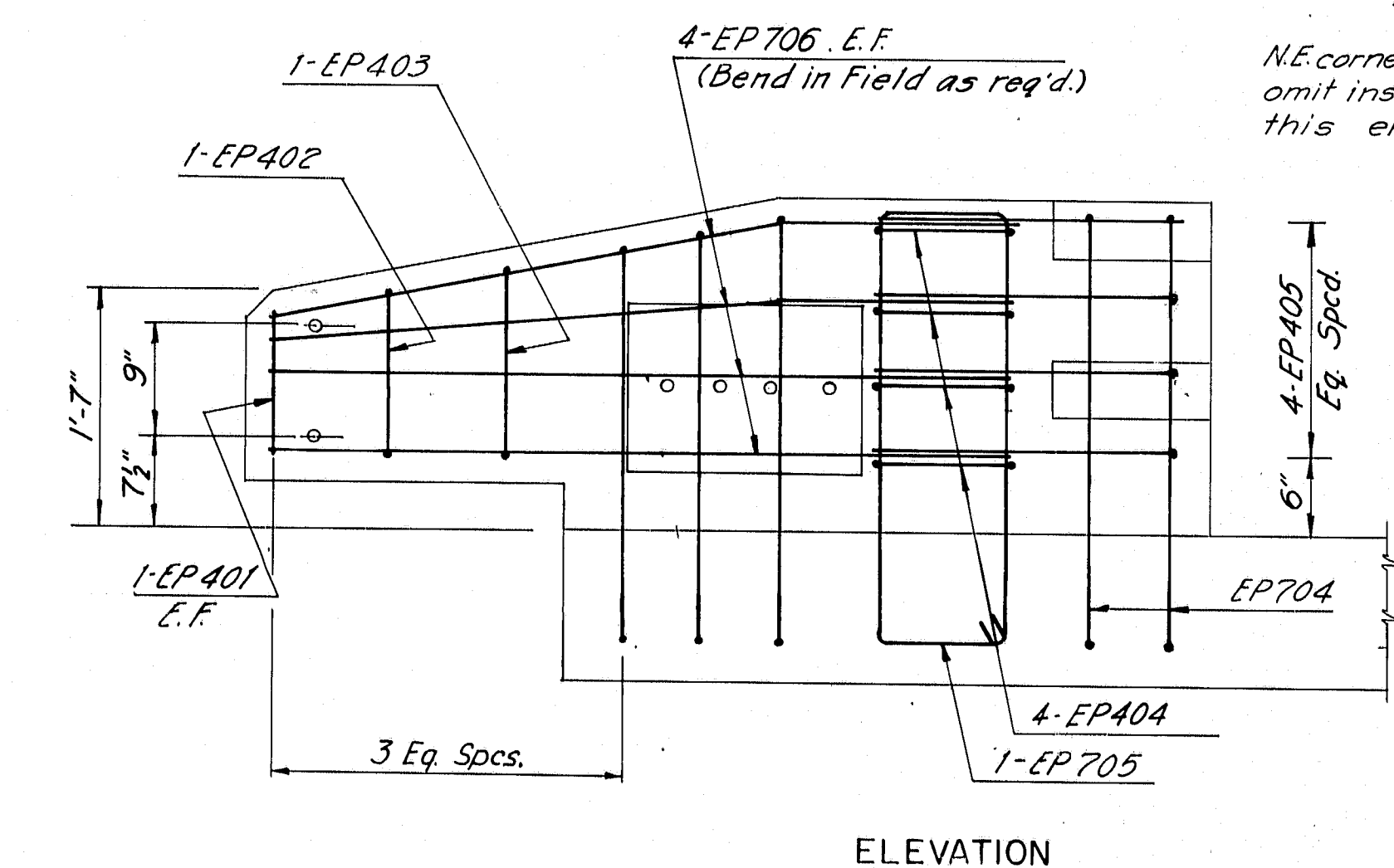
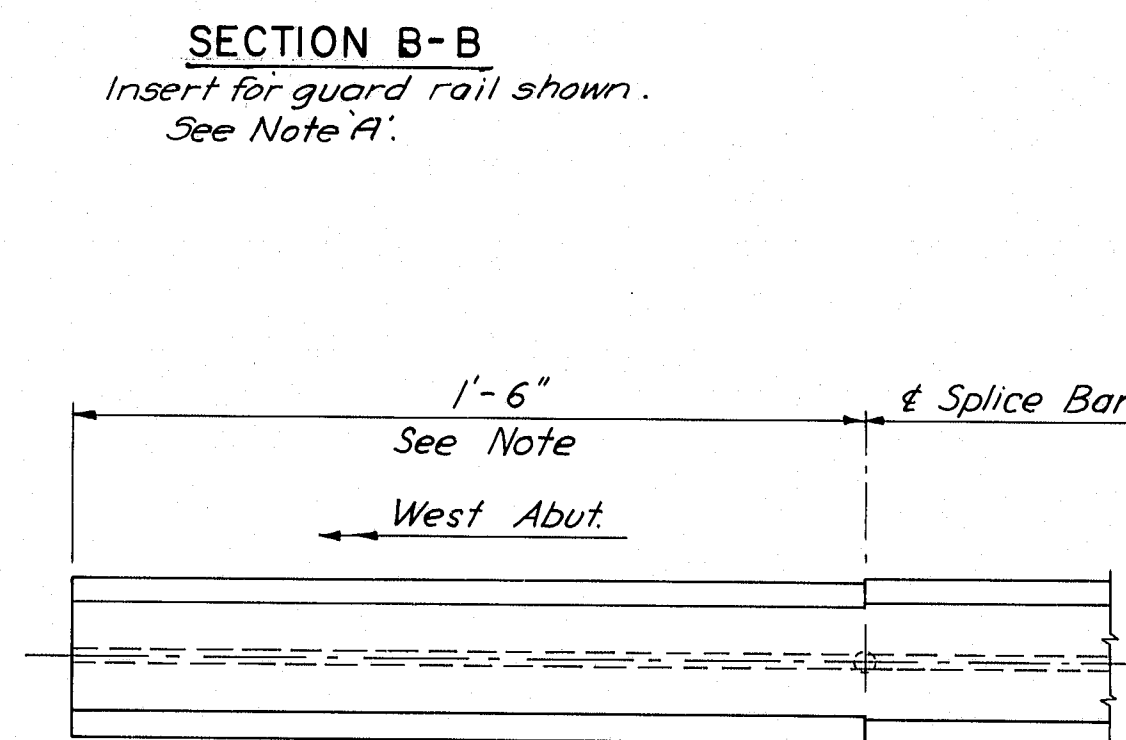
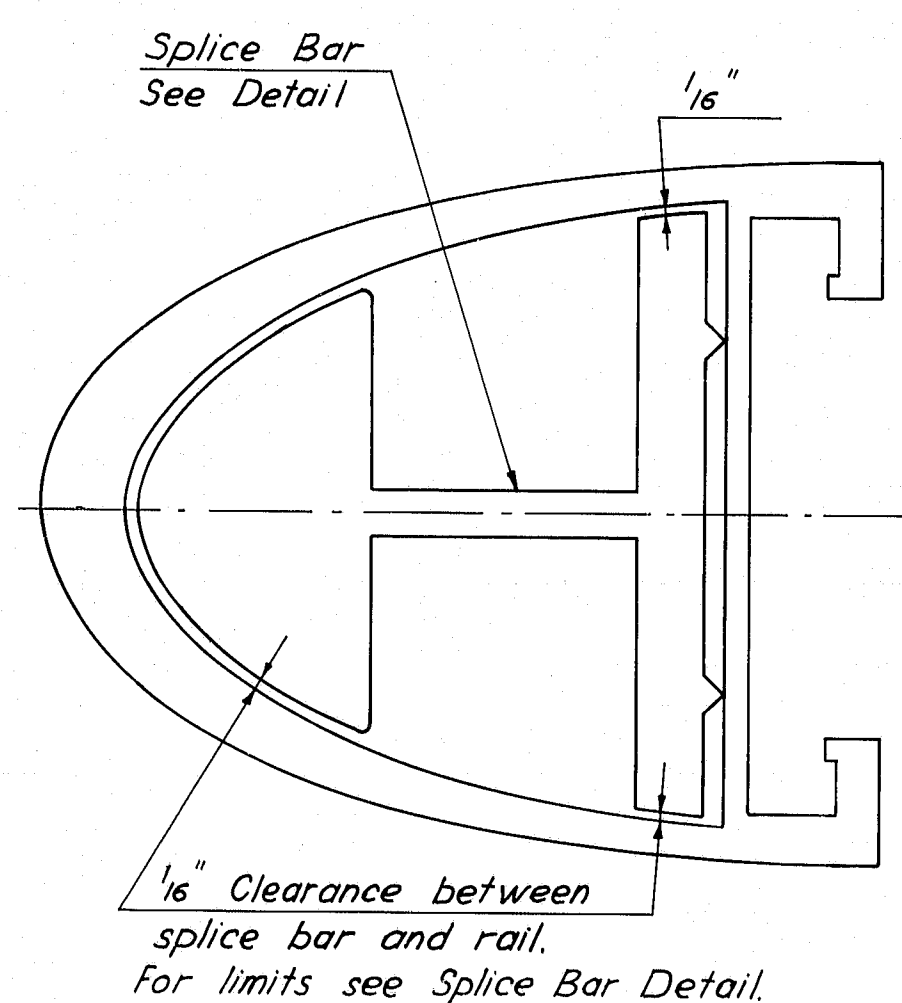
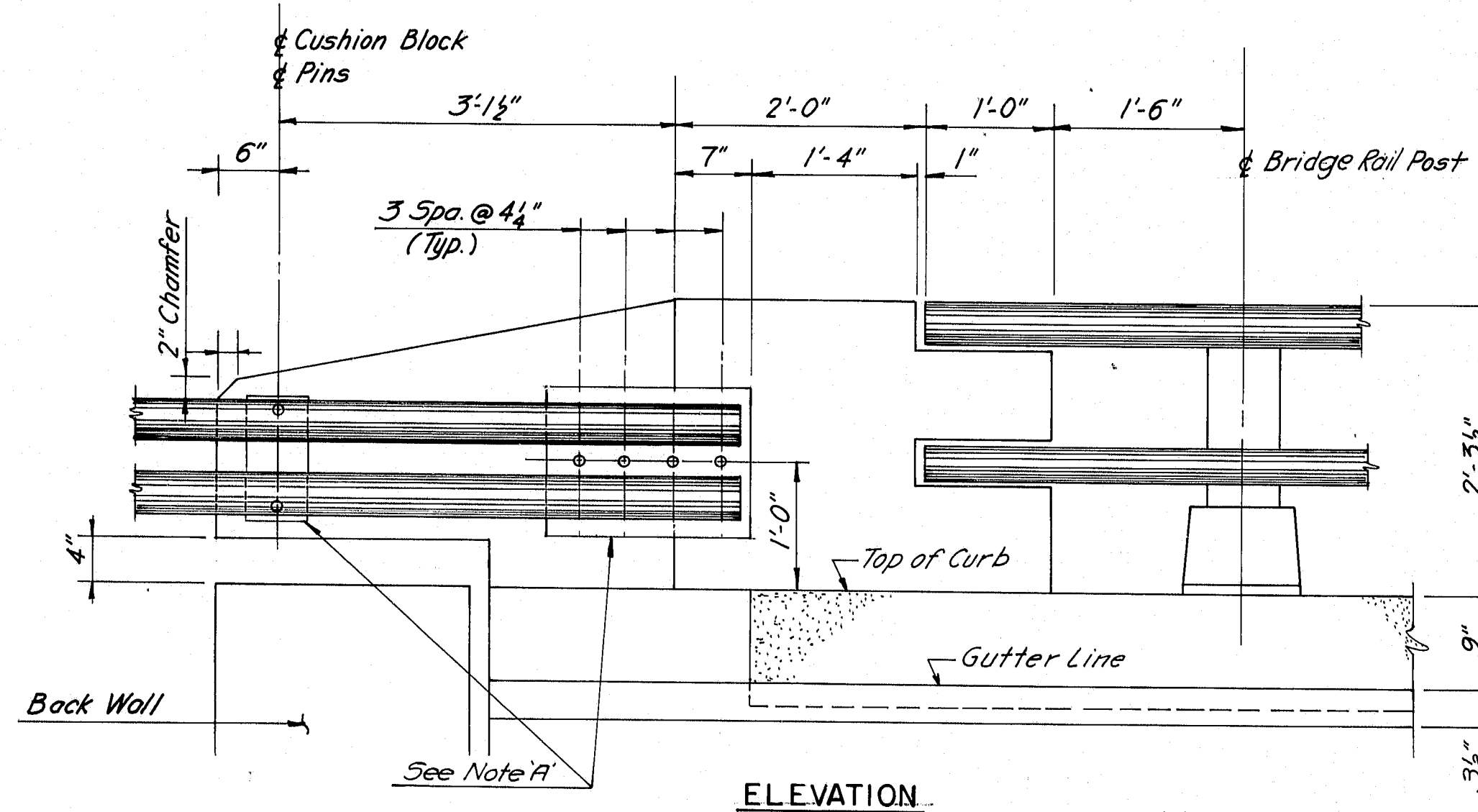
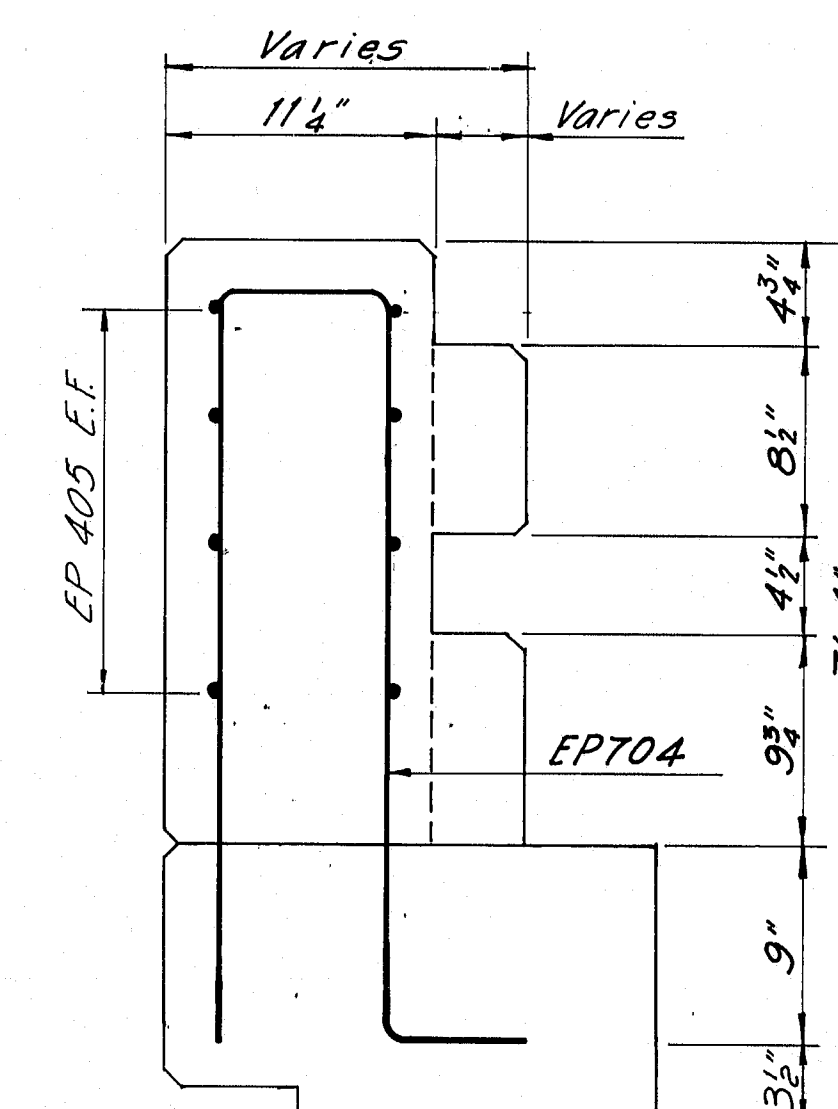
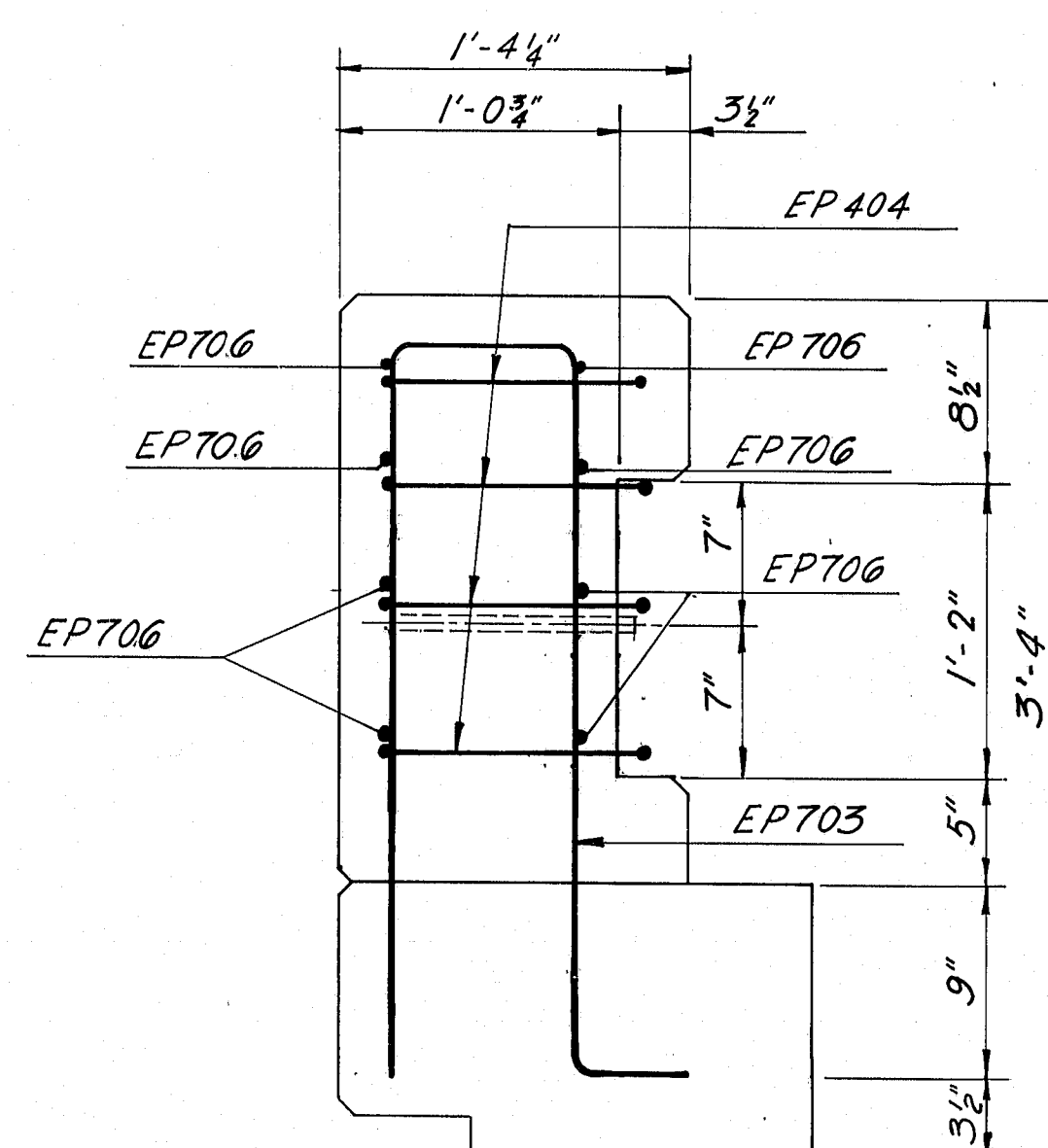
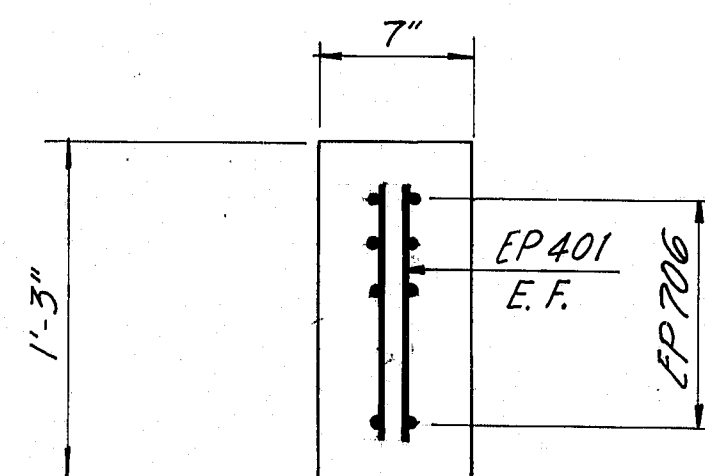
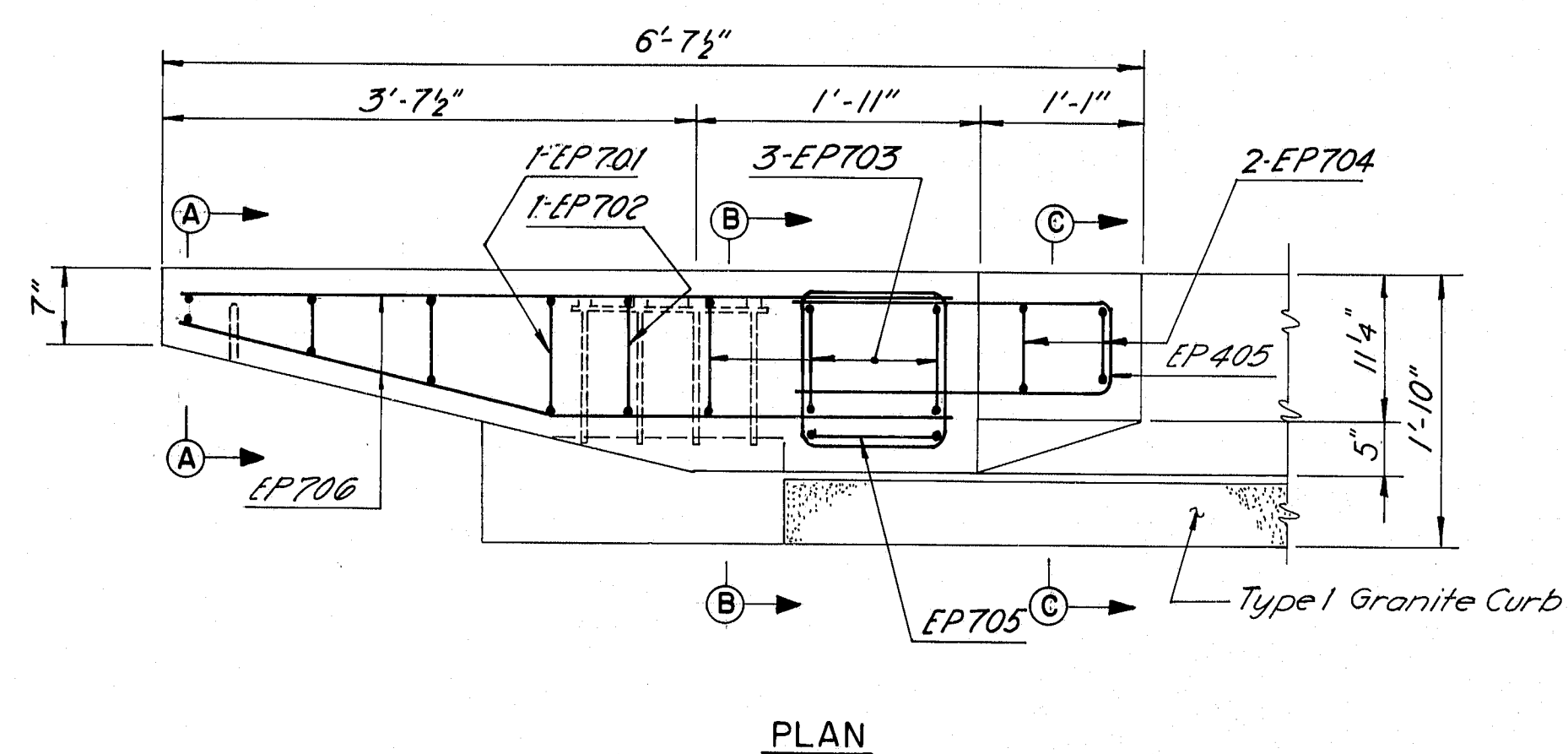
HNTB
 HOWARD NEEDLES TAMMEN & BERGENDOFF
 CONSULTING ENGINEERS
 BOSTON

STATE OF MAINE
 DEPARTMENT OF TRANSPORTATION

ROUTE I-195
 OVER
 MAINE TURNPIKE
 SUPERSTRUCTURE DETAILS
 SACO, MAINE

SHEET 12 OF 16 AUGUSTA, MAINE

FED. AID PROJ. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	16-195-2(9)	13	37



Note A'
Beam guard rail not required at N.E. corner of E.B. Rdwy, therefore omit insert and wood block at this end post.

Insert for guard rail shown. See Note A'.

Notes:
For dimensions and details not shown, see Concrete End Posts on Standard Detail Sheet BD 120-79.

Note:
This portion of the splice bar to be finished to provide a 1/8" min. clearance between splice bar and rail section. Remaining portion and additional details to be as shown on Standard Detail Sheet BD 114-77.
The 1/8" min. clearance is provision for each 2" of differential settlement which may occur between the Abutment and Pier.

NO.	REVISION	BY DATE	IN CHARGE OF
			J.A.E.
		MADE BY DATE	
		TRACED S.A. 4-81	
		CHECKED N.F.O. 4-81	

R92-204

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CONSULTING ENGINEERS BOSTON

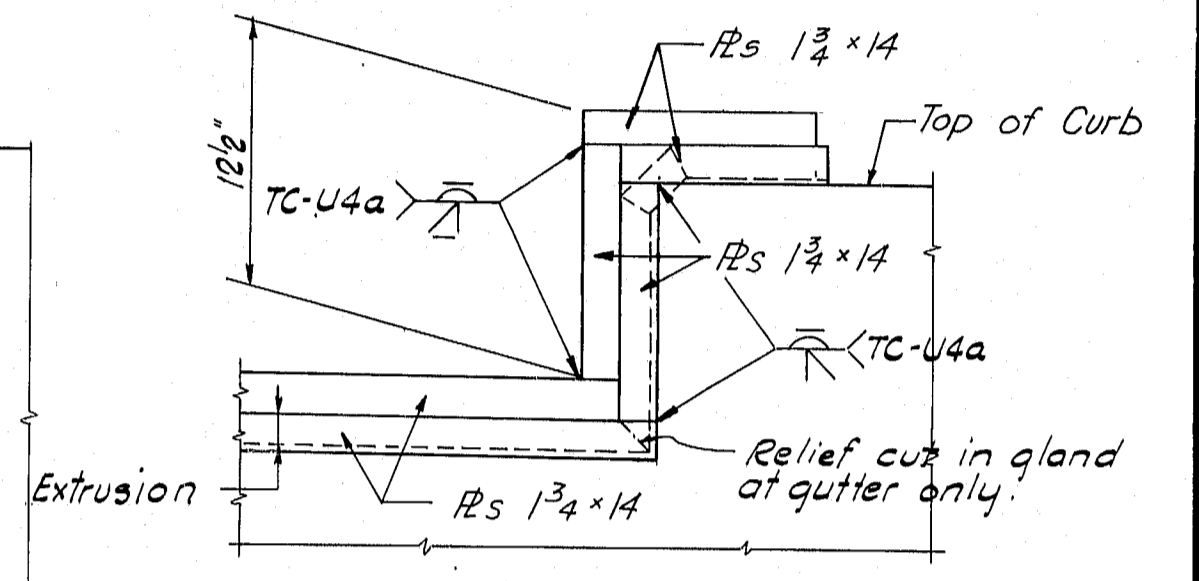
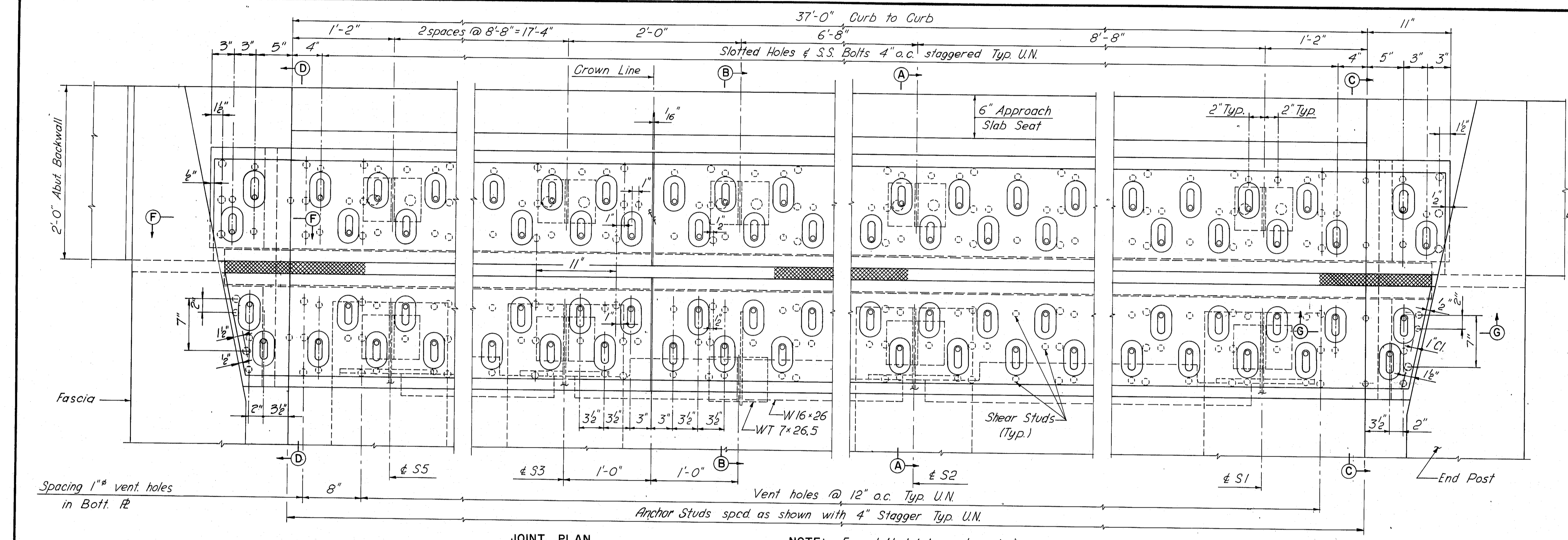
STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

ROUTE I-195
OVER
MAINE TURNPIKE

END POST DETAIL
SACO, MAINE

SHEET 13 OF 16 AUGUSTA, MAINE MAY 1980

PROJECT NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	IG-195-2(9)	14	37

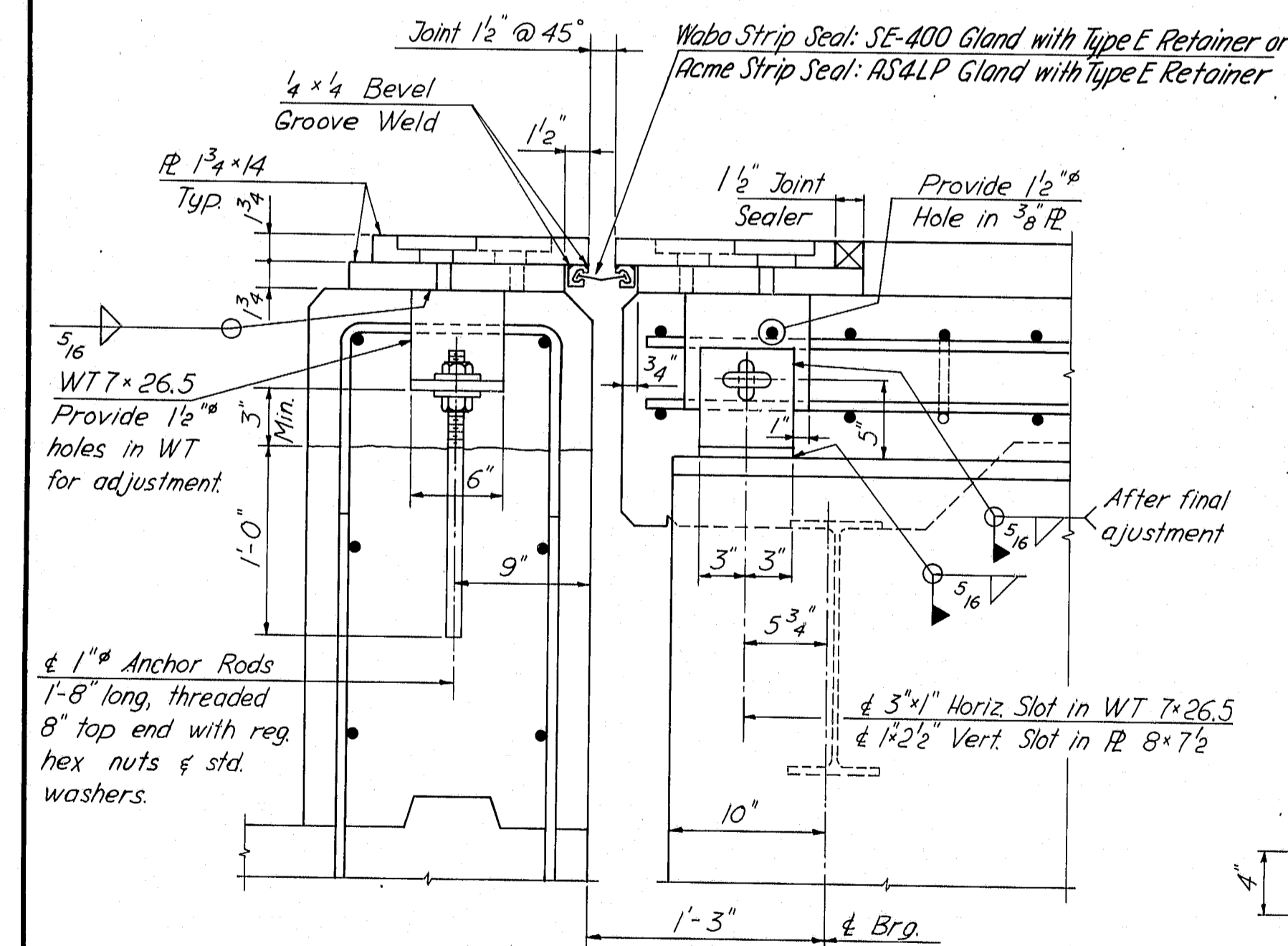


SECTIONS F-F & G-G
Slotted holes, anchor studs & vent holes are not shown.

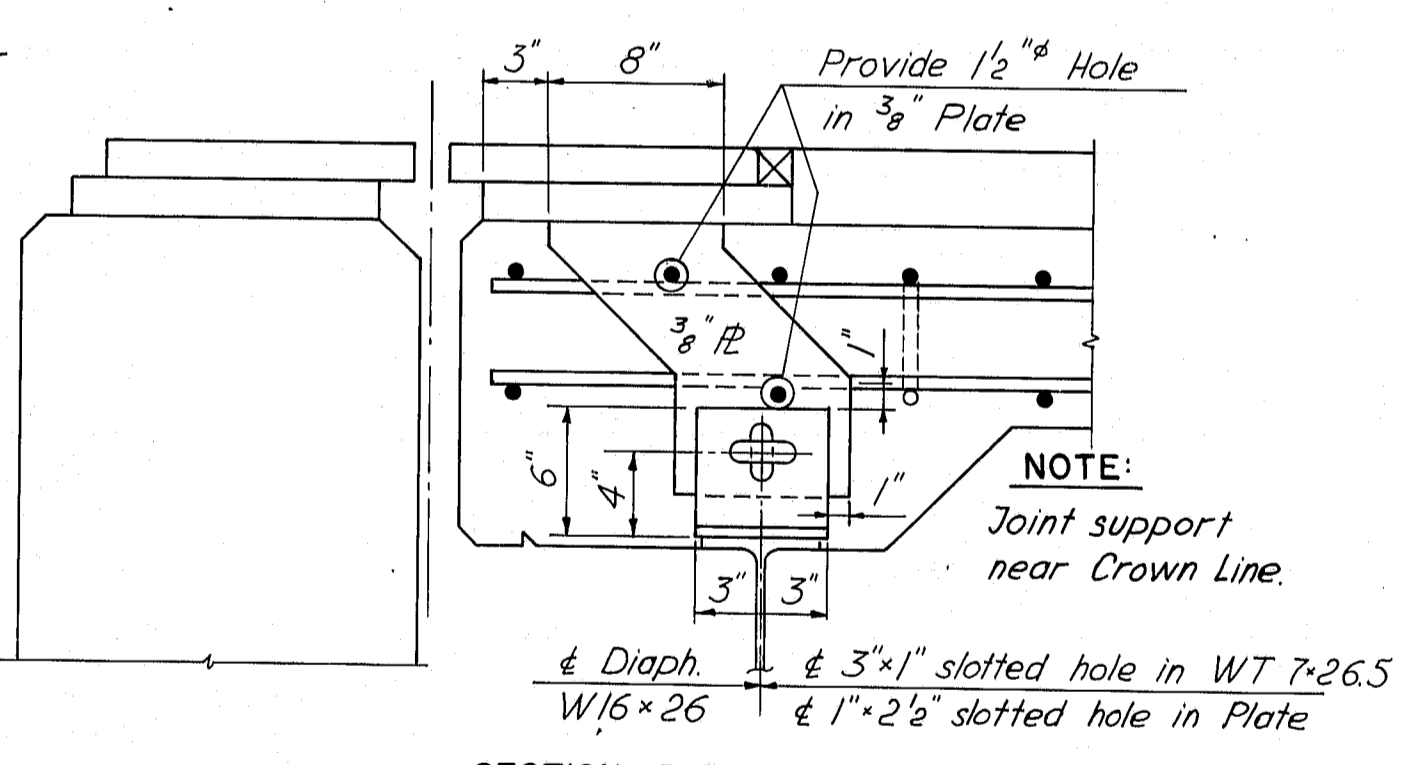
- NOTES:**
- Structural steel to conform to ASTM A588, any grade.
 - Slots in upper plate for stainless steel bolts allow for future adjustment as may be necessary to compensate for movement due to settlement. When placing concrete, upper plate shall not be in place and threaded holes shall be plugged with temporary bolts.
 - Joint plates to be planed, straightened or otherwise treated to secure true level surfaces.
 - Use $\frac{3}{8}$ " H.S. bolts in joint support over stringer and diaphragm.
 - For Type A Joint @ Pier, see Sh. No. 11.

JOINT PLAN
Joint at West Abut. W.B. Rdwy or Joint at East Abut. E.B. Rdwy shown.
Joint at West Abut. E.B. Rdwy or Joint at East Abut. W.B. Rdwy opposite hand.

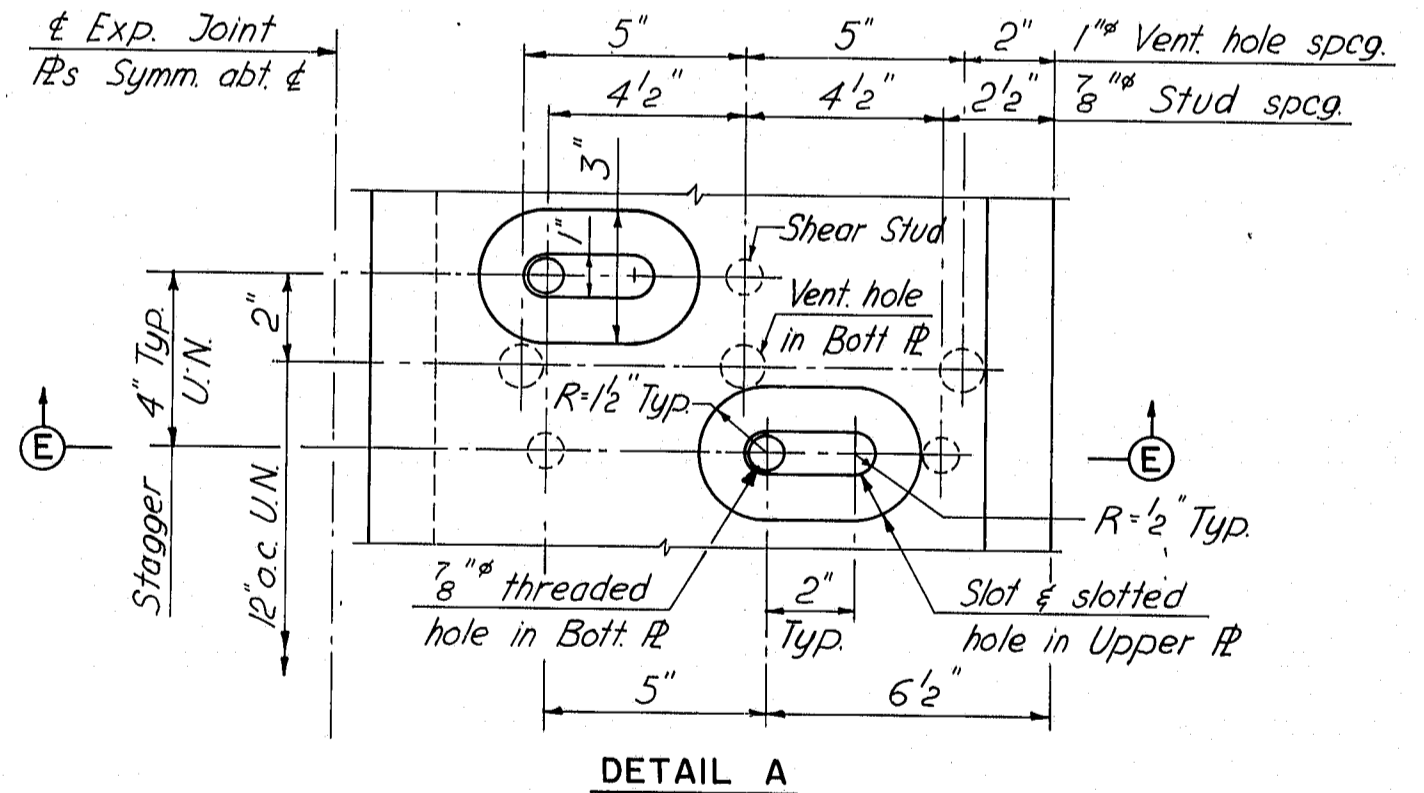
NOTE: For slotted holes, anchor studs & vent holes details see Detail A



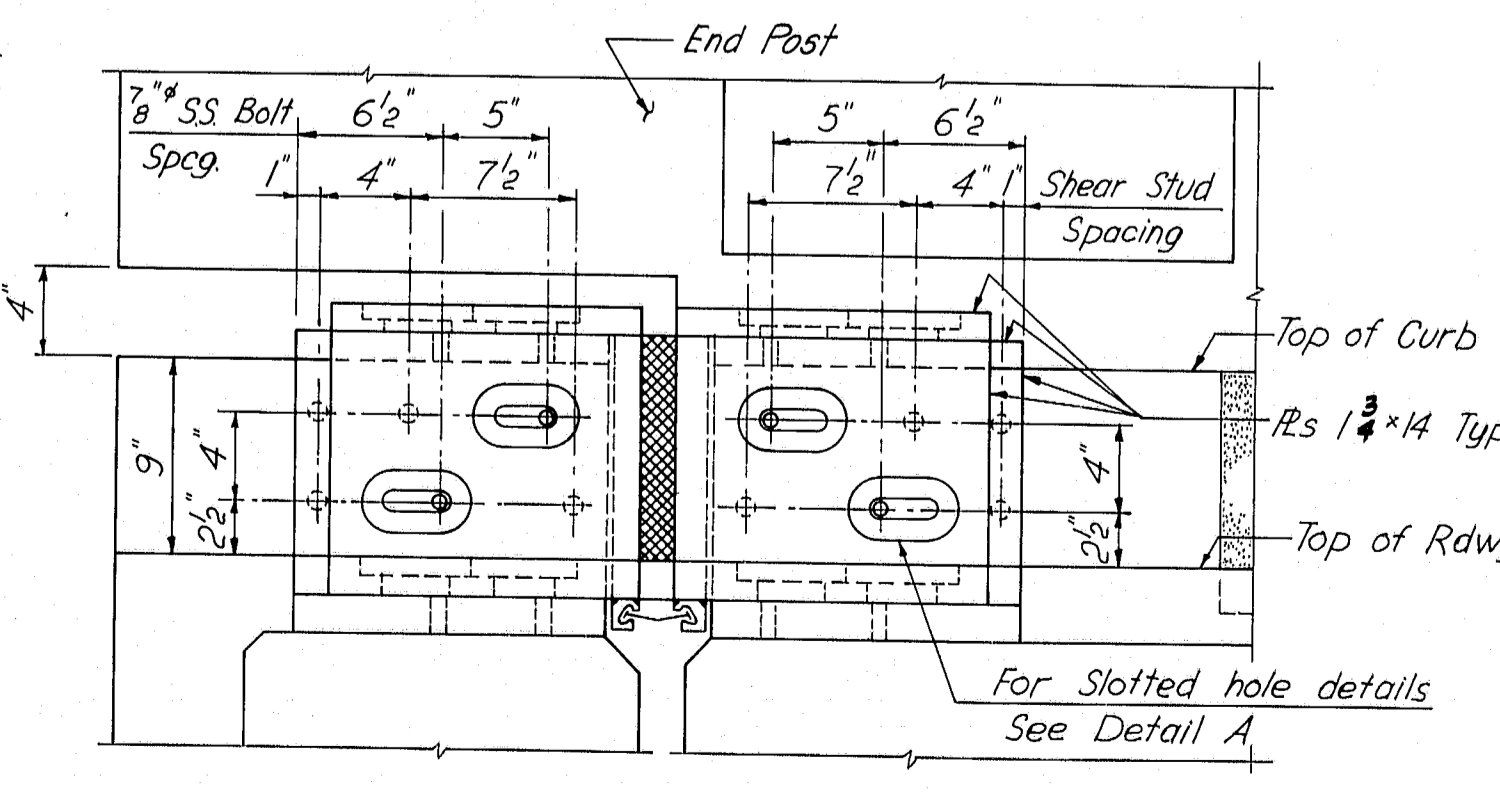
SECTION A-A
For Shear Stud location and additional Plate details see Detail A



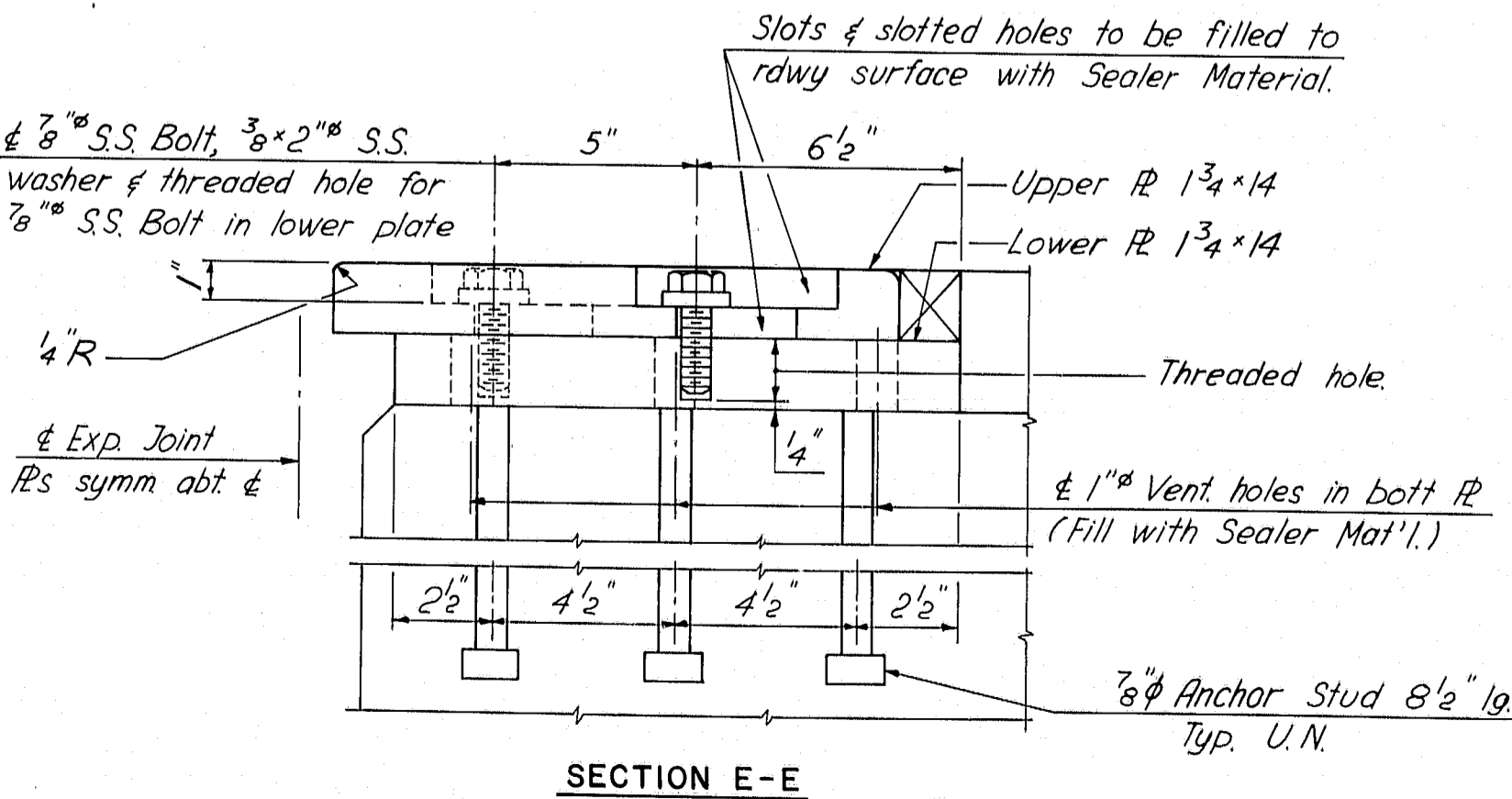
SECTION B-B
For details not shown see Section A-A



DETAIL A



SECTION C-C
Section D-D similar



SECTION E-E

Use 2 1/4" lg. 7/8" S.S. Bolts for Rdwy IRs.
Use 1 1/4" lg. 7/8" S.S. Bolts for Curb IRs.

NO.	REVISION	BY	DATE	IN CHARGE OF
		MADE	V.G.	4-81
		TRACED		
		CHECKED	N.F.O.	4-81
		BY DATE		J.A.E.

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

ROUTE I-195
OVER
MAINE TURNPIKE

JOINT DETAILS

SACO, MAINE

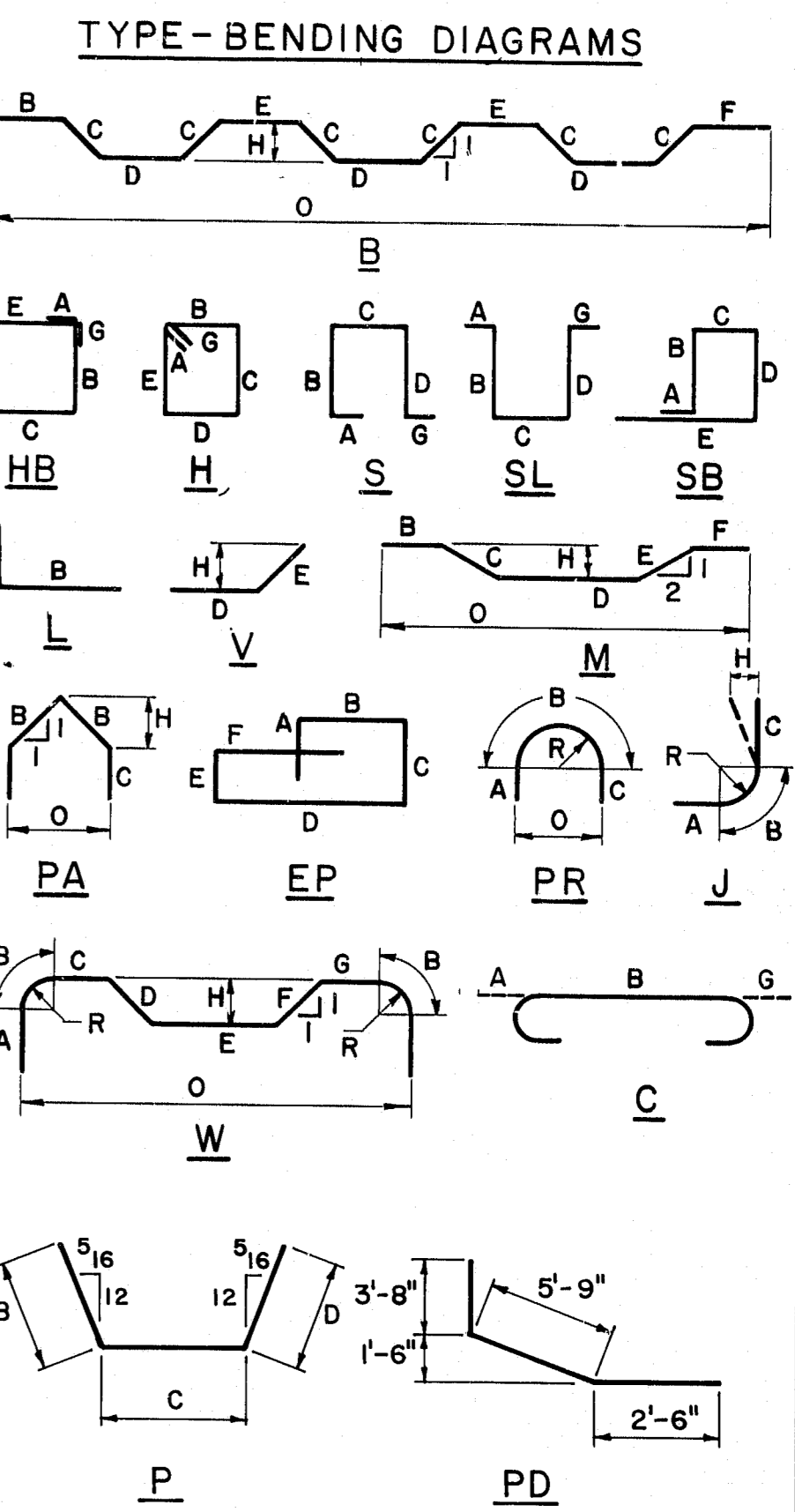
SHEET 14 OF 16 AUGUSTA, MAINE

R92205

REINFORCING STEEL SCHEDULE

STRAIGHT BARS												BENT BARS															
MARK	NO.	LENGTH	LOCATION	MARK	NO.	LENGTH	LOCATION	MARK	NO.	LENGTH	LOCATION	MARK	NO.	LENGTH	TYPE	A	B	C	D	E	F	G	H	O	R	LOCATION	
WEST ABUTMENT												WEST ABUTMENT															
A405	24	2-6	Dowels	P502	28	20-3 to 20-3	2 Groups of 14 - Shaft	A401	74	2-0	L	1-0	1-0													Approach Slab Seat	
				P513	6	39-4	Cap	A402	40	7-8	S	0	2-0	3-8	2-0											Bridge Seat	
				P514	2	35-0	"	A403	50	6-11	S	0	2-0	2-11	2-0											" "	
				P515	2	26-0	"	A404	12	8-7	S	0	3-10	0-11	3-10											Curtain Wall	
A501	111	8-6	Footing					A504	102	8-4	L	3-1	5-3													Breastwall	
A502	21	38-0	"					A512	74	4-3	S	0	1-9	1-2	1-4											Backwall	
A503	112	2-0	Dowels					A514	38	4-4	S	0	1-4	1-8	1-4											"	
A505	74	9-2	Backwall	P801	15	30-0	Footing	A523	4	12-6	V			9-4	3-2							1-5				Wingwall	
A506	14	35-0	Breastwall	P804	6	7-2	Shaft	A524	8	8-4	S	0	1-4	5-8	1-4											Breastwall	
A507	8	45-0	Backwall	P805	22	19-4	"	A526	10	10-1	S	0	4-9	0-10	4-9											Curtain Wall	
A508	2	39-7	"	P806	38	19-3	"	A527	2	16-3	H	0-5 1/2	1-8	6-0	1-8	6-0										Wingwall	
A509	2	41-7	"	P807	4	7-7	Cap	A528	2	17-3	H	0-5 1/2	1-8	6-6	1-8	6-6										"	
A510	2	43-7	"	P808	4	12-0	"	A529	2	18-3	H	0-5 1/2	1-8	7-0	1-8	7-0										"	
A511	2	38-5	"					A530	2	19-3	H	0-5 1/2	1-8	7-6	1-8	7-6										"	
A513	8	10-2	"					A531	2	20-3	H	0-5 1/2	1-8	8-0	1-8	8-0										"	
A515	2	9-8	Wingwall	P901	31	12-6	Footing	A701	112	4-10	J	1-0	0-10	3-0												0-6 Dowels	
A516	2	9-2	"	P903	8	12-10	Cap																				
A517	2	8-8	"																								
A518	16	35-1	Backwall																								
A519	20	11-11	"																								
A520	20	33-8	Breastwall	EASTBOUND RDWY. PIER same as Westbound Rdwy. Pier																							
A521	3	22-8	Backwall																								
A522	6	11-0	Wingwall																								
A525	6	7-3	Backwall																								
A532	20	2-10	Dowels																								
A533	4	18-10	Backwall																								
A534	4	10-4	Backwall																								
A601	74	4-9	Backwall					P501	28	11-7 to 13-0	H	0-5 1/2	2-8 to 3-4	2-8 to 3-4	2-8 1/2												2 Groups of 14 - Shaft
A602	14	12-10	Wingwall					P503	4	13-11	H	0-5 1/2	2-7	3-10	2-9	3-10											Cap
A603	2	5-2	"					P504	4	14-5	H	0-5 1/2	2-7	4-1	2-9	4-1											"
A604	2	7-2	"					P505	4	14-10	H	0-5 1/2	2-6	4-4	2-9	4-4											"
A605	2	9-2	"					P506	4	15-2	H	0-5 1/2	2-6	4-6	2-9	4-6											"
								P507	4	15-4	H	0-5 1/2	2-6	4-7	2-9	4-7											"
								P508	4	15-8	H	0-5 1/2	2-6	4-9	2-9	4-9											"
								P509	4	15-10	H	0-5 1/2	2-6	4-10	2-9	4-10											"
								P510	4	16-2	H	0-5 1/2	2-6	5-0	2-9	5-0											"
								P511	4	16-4	H	0-5 1/2	2-6	5-1	2-9	5-1											"
A702	74	6-10	Backwall					P512	21	18-1	H	0-5 1/2	3-4	5-1	3-8	5-1											"
A703	20	11-11	"					P516	16	11-11	PD			See Bending Diagram													"
A704	8	10-2	"					P517	40	7-8	S	0	2-0	3-8	2-0												Bridge Seat
A705	2	8-8	Wingwall					P518	10	8-2	S	0	2-0	4-2	2-0												" "
A706	2	9-2	"					P519	6	6-2	S	0	1-3	3-8	1-3												Cap
A707	2	9-8	"					P802	8	16-10	P		7-2	2-6	7-2												Footing
A708	4	10-4	Backwall					P803	15	16-4	S	0	7-2	2-0	7-2												"
								P902	8	41-10	C	1-3	39-4														Cap
EAST ABUTMENT same as West Abutment												EAST ABUTMENT same as West Abutment															
EASTBOUND RDWY. PIER same as Westbound Rdwy. Pier												EASTBOUND RDWY. PIER same as Westbound Rdwy. Pier															

FWHA REG. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	26-195-2(9)	15	37



All dimensions are out to out of reinf. bar
 Bending details and hooks shall conform to the recommendations of ACI Standard 318-71
 Reinforcing Bar: ASTM A615 Grade 60

GENERAL NOTES

1. First digit(s) following the letter of the Mark indicates size of reinf. bar.
 Mark (A 502) bar size - #5
 Mark (P 1001) bar size - #10
 Mark (S 603) bar size - #6
2. Letter of Marks A, P & S locates bars of Abutments, Piers, and Superstructure parts respectively.

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

ROUTE 1-195
OVER
MAINE TURNPIKE

REINFORCING STEEL SCHEDULE
SACO, MAINE

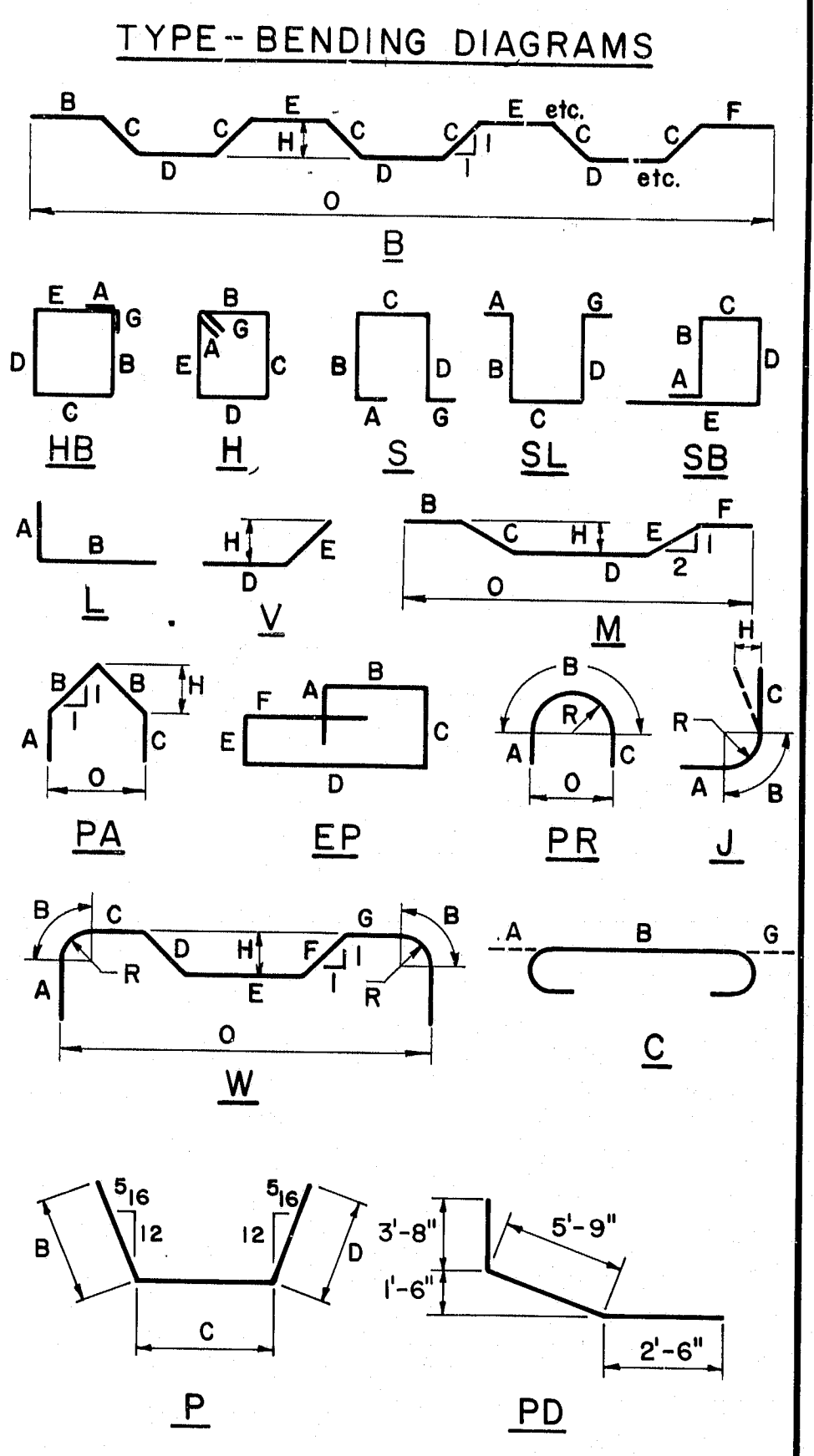
R92-206

DATE	BY	DESIGN-DETAIL	REVISIONS	FIELD CHANGES
4-81				

REINFORCING STEEL SCHEDULE

STRAIGHT BARS										BENT BARS												
MARK	NO.	LENGTH	LOCATION	MARK	NO.	LENGTH	LOCATION	MARK	NO.	LENGTH	TYPE	A	B	C	D	E	F	G	H	O	R	LOCATION
WESTBOUND RDWY. APPROACH SLABS																						
AP401	32	35-8	Approach Slab																			
AP601	144	15-0	Approach Slab																			
EASTBOUND RDWY. APPROACH SLABS same as Westbound Rdwy. Approach Slabs																						
WESTBOUND RDWY. DECK SLABS																						
5301	242	36-8	Wearing Surface																			
5302	290	30-0	" "																			
5501	480	40-4	Slab																			
5502	744	31-4	Slab & Curb																			
5504	32	6-0	Slab																			
EASTBOUND RDWY. DECK SLABS same as Westbound Rdwy. Deck Slabs																						
WESTBOUND RDWY. END POSTS																						
EP401	8	1-1																				
EP706	32	5-3																				
EASTBOUND RDWY. END POSTS same as Westbound Rdwy. End Posts																						
WESTBOUND RDWY. DECK SLABS																						
5503	484	5-1	S	0-0	1-0	1-1	1-0															Curb
5505	8	2-9	L	1-3	1-0																	"
5601	238	42-0	B		4-8	0-8 1/2	4-0	3-8	4-8									0-6	40-4			Slab
EASTBOUND RDWY. DECK SLABS same as Westbound Rdwy. Deck Slabs																						
WESTBOUND RDWY. END POSTS																						
EP402	4	3-9	H	0-4 1/2	0-4	1-2	0-4	1-2														
EP403	4	4-5	H	0-4 1/2	0-6	1-4	0-6	1-4														
EP404	16	4-9	H	0-4 1/2	1-0	1-0	1-0	1-0														
EP405	16	5-1	S	0	2-3	0-7	2-3															
EP701	4	0-5	S	0	2-8	0-7 1/2	2-8															
EP702	4	0-7	S	0	2-9	0-7 1/2	2-9															
EP703	12	0-9	S	0	2-10	0-7 1/2	2-10															
EP704	8	0-8	S	0	2-10	0-6	2-10															
EP705	4	8-5	H	0-5 1/2	2-10	0-11	2-10	0-11														
EASTBOUND RDWY. END POSTS same as Westbound Rdwy. End Posts																						

FWWA RES. NO.	STATE	PROJECT NUMBER	SHEET NO.	TOTAL SHEETS
1	MAINE	IG-195-2(9)	16	37



All dimensions are out to out of reinf. bar.
 Bending details and hooks shall conform to the recommendations of ACI Standard 318-71.
 Reinforcing Bar: ASTM A615 Grade 60

- GENERAL NOTES**
- First digit(s) following the letter of the Mark indicates size of reinf. bar.
 Mark (A 502) bar size - #5
 Mark (P 1001) bar size - #10
 Mark (S 603) bar size - #6
 - Letter of Marks A, P & S locates bars of Abutments, Piers, and Superstructure parts respectively.

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

**ROUTE 1-195
OVER
MAINE TURNPIKE**

REINFORCING STEEL SCHEDULE
SACO, MAINE

SHEET 16 OF 16 AUGUSTA, MAINE

R92-207

DATE: 4-81
 BY: N.F.O.
 DESIGN - DETAIL
 REVISIONS
 FIELD CHANGES
 PLANS