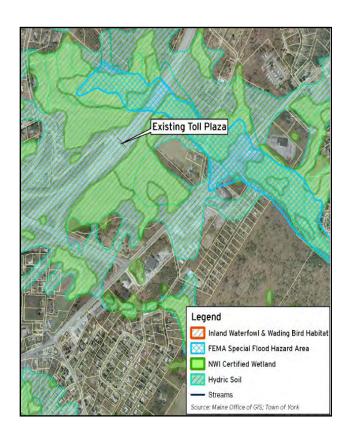
# MAINE TURNPIKE SOUTHERN TOLL PLAZA

# DRAFT - PHASE ONE REPORT

For submittal to the U.S. Army Corps of Engineers







November 5, 2009

The Maine Turnpike Authority



Prepared by: HNTB Corporation



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## PART 1 INTRODUCTION AND OVERVIEW

## **SECTION 1 - INTRODUCTION**

In 1941 the Maine State Legislature passed An Act to Create the Maine Turnpike Authority. Thus was born a new, independent state agency charged with constructing a highway from "some point at or near Kittery to some point at or near Fort Kent." Subsequent actions have defined the Maine Turnpike ending in Augusta.

The York Toll Plaza is the Maine Turnpike Authority"s southern-most toll plaza and has now served beyond its useful life. The plaza is processing more than three times the traffic it did when it first opened in 1969, is suffering from numerous operational and structural deficiencies, and is increasingly a safety concern. As a result of these factors, several years ago the Maine Turnpike Authority (MTA) decided to stop all non-critical repairs and to comprehensively evaluate the existing plaza issues and investigate how to most effectively move forward with a replacement that meets the Authority"s goal of operating a safe, efficient and modern, southern toll plaza.

Contained within this report are findings of the various phases of investigation from existing conditions, to a repair strategy (no-build), to upgrade strategies, to new construction on new location. Ultimately, this report details a recommendation for advancing a shortlist of options and/or alternatives into a draft Permit Application and Phase 2 investigation as detailed in the U.S. Army Corps of Engineers" (USACE) Highway Methodology Process. The report also details a number of critical items necessary to fully evaluate the physical and operational characteristics of the existing York Toll Plaza. These include: the Standards and Best Practices for design of a toll plaza; the purpose and need for addressing the toll plaza; the toll plaza"s operation and what influences that operation; its safety history; and, the proposed size of the new toll plaza given its expected life span.

The Phase 1 report is organized into four Parts to align itself with the progression of the overall project and steps completed to date.

- 1. Part 1 includes this introduction along with a brief background of the York Toll Plaza and a summary of the existing conditions of the plaza.
- 2. Part 2 is the Existing Site Evaluation (ESE), dated June 16, 2009 and accepted by the Maine Turnpike Authority on September 9, 2009. The ESE begins with an introduction highlighting the project"s history, including public participation and coordination with the Maine Legislature. It documents the Project Purpose and Need as required by USACE. The USACE"s Basic Project Purpose statement can be found in Appendix B. The ESE provides a full analysis of the physical and operational deficiencies of the existing toll plaza. Finally, the ESE documents rehabilitation and reconstruction options ranging from a "do-nothing" option to a variety of upgrade options at the existing location.
- 3. Part 3 is the Alternate Site Evaluation (ASE). The ASE documents the identification and analysis of alternate toll plaza sites.

4. Part 4 is the comparative screening of the recommendations from the ESE in Part 2 with the recommendations from the ASE in Part 3. Part 4 also contains the final recommendation for concluding the Phase 1 report to the USACE, which is a shortlist of options and/or alternates recommended to be carried into Phase 2 of USACE"s Highway Methodology Process.

## **SECTION 2 – BACKGROUND**

The Maine Turnpike Authority is a quasi-state agency created by the Maine Legislature in 1941 to construct, manage, and operate the 109-mile toll highway from Kittery to Augusta. Completed in 1947, the Maine Turnpike became the second superhighway built in America - the Pennsylvania Turnpike was the first. Since its inception, the Maine Turnpike has set national standards for the way it is financed, maintained, and continually improved.

The history of the Maine Turnpike is a testament to Yankee ingenuity, foresight, and pride. It was a modern marvel when it opened more than 60 years ago and it has established a tradition of excellence by incorporating the latest advancements in modern highway construction and state-of-the-art technology to keep pace with ever increasing traffic volumes.

These high standards of operation have placed the Maine Turnpike among an elite group of the most highly credit-rated agencies in the nation, and have resulted in consistently high marks on annual customer satisfaction surveys.

The existing 17 lane York Toll Plaza is the first toll plaza encountered when entering Maine from the south and the last toll plaza when leaving Maine. The plaza processes over 16 million vehicles per year which equates to \$34 million in revenue (nearly 39% of total Maine Turnpike revenue). Truck traffic accounts for nearly 15% of the plaza"s use. Today at the plaza, approximately 58% of total vehicles and 80% of truck traffic utilize E-ZPass, the Maine Turnpike"s form of Electronic Toll Collection (ETC). The traffic processed by this plaza is nearly an equal blend of in-state and out-of-state travelers. Recreational traffic increases dramatically during the summer months (June through September), with traffic peaking northbound on Friday evenings and southbound on Sunday afternoons. Two-way traffic through the plaza peaks during the mid-day hours on Saturdays.

The existing York Toll Plaza is situated seven miles from the New Hampshire border on the Maine Turnpike (I-95). The existing toll plaza began as an 11 lane temporary structure constructed on the Maine Turnpike in 1969 for the purposes of paying back the bonds used to construct the Maine Turnpike. In the early 1980"s the Maine Legislature decided to continue the use of tolls to fund the operation and maintenance of the turnpike as well as to fund the turnpike Modernization and Widening, and Interchange Program.

In 2006, the MTA began to meet with officials from Kittery, York, Ogunquit, and Wells to discuss the likely reconstruction and potential relocation of the York Toll Plaza. During that time, the 123<sup>rd</sup> Maine Legislature passed, and the Governor signed into Law, a Resolve "Directing the Maine Turnpike Authority to Study the Relocation of the York Toll Booth". The MTA then prepared a report to the Legislature"s Joint Standing Committee on Transportation

titled "Response to LD 534", which analyzed numerous aspects of such a toll plaza replacement. The technical report in response to Maine LD 534 can be found in Appendix K.

This Phase 1 Report builds on the legislative report by documenting an evaluation of various existing site options, as well as comprehensively reviewing the options within the surrounding area of the existing location. In addition, this report documents the investigation of potential alternate sites for the relocation of the York Toll Plaza. The Phase 1 Report was prepared at the MTA"s directive, including consideration of a request from the York Board of Selectpersons, to evaluate all possible solutions. The time that has elapsed since the submission of the legislative report has provided the opportunity to further study elements that would normally occur later in the design process, typically in Phase II of the Highway Methodology, such as the refined plaza sizing and refined configuration.

The MTA"s goal, as it relates to the York Toll Plaza, is to have a safe and efficient toll plaza at the southern end of the Maine Turnpike. The MTA"s Enabling Legislation in its Legislative Findings (23 M.R.S.A. §1961) makes the following findings of fact: the economic and social wellbeing of the citizens of the State requires that the transportation system be developed in a comprehensive manner and depends upon the <u>safety</u>, <u>efficiency</u>, <u>and modern functional state of the turnpike</u>. To that end, it is necessary to approach this study in the following steps:

- Define the purpose and need for the project identify the deficiencies in the existing plaza and define the improvements needed and the benefits of making these improvements.
  - a) A formal Purpose and Need request was made to the U.S. Army Corps of Engineers (USACE) and has since been formally determined (Appendix B);
  - b) Formal Purpose and Need statement developed by USACE;
  - c) Determine the appropriate design standards;
  - d) Determine a tolling option that best fits the current and anticipated future conditions; and,
  - e) Adequately size the plaza for current and future conditions.
- 2) Evaluate the options at the existing site against the Purpose and Need.
- 3) If the existing site options fail to meet the objectives of the Purpose and Need, identify and evaluate other potential sites that meet the criteria.

This report evaluates the existing conditions at the York Toll Plaza, documents the industry standard design guidelines as they apply to toll plaza layout and location, evaluates the feasibility of varying levels of upgrades of the existing York Toll Plaza, identifies alternate locations that meet the basic engineering guidelines and the Basic Project Purpose and Need, and finally, recommends a shortlist of options and/or alternates to be further evaluated within the Phase 2 process of the USACE's Highway Methodology. It is important to note that the same design guidelines, aerial photography, GIS resource mapping, and plaza operation investigations were applied to the existing site upgrades and the alternate sites. This was done to ensure a fair and equitable comparison of the various options and alternates.

## **SECTION 3 - EXISTING CONDITIONS AT THE YORK TOLL PLAZA**

The following is a detailed analysis of the existing conditions at the York Toll Plaza based on the investigation and report titled, Response to LD534, presented to the Maine Legislature's Joint Standing Committee on Transportation in February 2008 along with updates to some of these conditions.

It is noteworthy to begin this discussion by reiterating the following: the location of the existing York Toll Plaza was not selected by HNTB or the MTA, nor was its location based upon engineering criteria or Best Practices. Its location was primarily determined by political negotiations between state and federal transportation officials surrounding the construction of the Piscataqua River Bridge and the new section of highway connecting the bridge with the Maine Turnpike. Both HNTB and the MTA opposed the decision at the time. Knowledge of this history and its long-term consequences, with which we are now addressing, serve as a reminder as to why engineering and environmental Best Practices should factor heavily into long-term transportation investment decisions. Fortunately, the strengthening of the environmental permitting process over the last 40 years, in particular the USACE Highway Methodology, combined with the recent development of FHWA guidelines for toll plazas, requires a more deliberative and accountable decision-making process for today's significant capital projects. From an engineering perspective, the plaza was built with approximately a 25 year design life. Now 40 years old, the plaza is not only failing functionally, but also structurally. The age of the plaza, the outmoded conditions of the existing tollbooths, canopy, and tunnel, and the poor soil conditions all contribute to the overall poor condition and performance of the plaza. The proximity to the Exit 7 Interchange and roadway geometry that does not meet engineering standards, compromise staff and motorist safety, and further renders the existing facility inadequate. A summary of these deficiencies is presented. Details on the Standards and Best Practices, against which these conditions and/or deficiencies are evaluated, are presented in Part 2 – Existing Site Evaluation in the section on Design Guidelines.

### 1. Proximity to Interchange

The proximity of the Chase's Pond Road Interchange (Exit 7), located immediately south of the York Toll Plaza, introduces potentially unsafe and undesirable operational conditions due to excessive and forced traffic weaving and poor sight distance (Figure 1.1). The FHWA"s recently published "State of the Practice and Recommendations on Traffic Control Strategies at Toll Plazas," recommends a one (1) mile separation between toll plazas and interchanges. The location of the existing toll plaza does not meet this recommendation. Exit 7"s southbound off-ramp is less than 1,000 feet from the plaza and the northbound on-ramp is less than 500 feet from the plaza. The close proximity of these interchange ramps to the plaza create unsafe traffic weaving, signing difficulty, and driver confusion for all travelers. Traffic weaving occurs, for example, when Exit 7 northbound traffic merges into mainline traffic and changes lanes to access one of the center lanes, while mainline truck traffic shifts to the right to access wide load and dedicated E-ZPass lanes. As described here, there is a very short distance for a driver to interpret conditions, make decisions, and take the appropriate action. Adding to driver confusion is the fact that typical highway sign spacing cannot be appropriately accommodated within this short distance. Sign spacing is therefore very compressed which requires drivers to interpret, decide, and act much quicker than is normally

required. As a result of these conditions there have been numerous crashes over the years. In fact, the MaineDOT has classified the York Toll Plaza as a High Crash Location (HCL) consistently for the past ten years. Recent data shows an excessive number of crashes occurring at the plaza in both the northbound and southbound directions. The most recent HCL data reporting period 2006-2008, continues to document this trend with an above average number of crashes occurring on the northbound length of the turnpike between Exit 7 and the plaza. Over the last four years the number of crashes occurring southbound between the plaza and Exit 7 has increased to a point where this length of turnpike is now also classified as a High Crash Location. The overall trend is that both the north and southbound lanes at the York Toll Plaza are High Crash Locations and continue to grow worse over time. More HCL data including MaineDOT"s Crash Summary Report"s can be found in Appendix G. Given these results, the existing plaza is in an undesirable location.



FIGURE 1.1 - Exit 7 Interchange Ramps South of York Toll Plaza

### 2. Sight Distance

Sight distance to the toll plaza is compromised by bridges, curves, and hills. The FHWA Guidelines recommend that toll plazas should be sited such that motorists will be able to see the plaza while driving at posted speeds with adequate stopping and decision sight distance. Bridges, curves, and hills negatively impact the sight distance. At the York Toll Plaza there are two crest vertical curves and a horizontal curve that limit decision sight distance to the plaza for southbound traffic; and the Chase's Pond Road Bridge limits these distances for northbound traffic. As noted earlier, limiting sight distance affects the decisions drivers make as well as forces them to make those decisions in a much quicker time. During high volume periods, less informed decisions can lead to poor operation and an increased risk of crashes. These and the following conditions make the current plaza location unsafe and undesirable.



FIGURE 1.2 - Northbound Sight Distance Bridge and horizontal curve negatively impact sight distance.

### 3. Proximity to Overhead Structures - Bridges

The proximity of the plaza to the Chase's Pond Road Bridge, being 2,200 feet from the existing plaza, limits the available sight distance as seen in Figure 1.2 above. Note, that the bridge pier, bridge deck (and beams) hide some of the plaza from view. Further, the bridge abutment hides the Exit 7 northbound on-ramp from view. The limited view caused by the bridge creates safety risks to motorists as well as operational concerns from hurried decision making. Desirably, there should be a 3,500 foot separation between the plaza and overhead structures such as bridges. A clear view of the toll plaza, including all available lanes allows drivers to make timely, informed decisions on speed and path. While the view of the toll plaza is only partially obstructed, the overall decision sight distance criteria is compromised from the blocked view of the interchange ramps (typically not an issue because they should be one mile away). The close proximity of this bridge is an undesirable characteristic of the existing toll plaza location.

### 4. <u>Horizontal Geometry</u>

The York Toll Plaza was built on a horizontal curve. The FHWA Guidelines state that a toll plaza should be located on a straight section of roadway and not on a horizontal curve. As detailed in the discussion of Sight Distance, the combination of the existing horizontal and vertical curves reduces the available sight distance to the plaza. Limiting sight distance in this way affects the lane choice decision a driver must make and forces the driver to make that decision in a much shorter period of time. This becomes critical in high volume periods when lane distribution plays a larger role in overall plaza capacity. The horizontal curve also reduces the ability of this location to support highway speed tolling. This will be discussed in more detail later in the report. The curved roadway also has an operational impact on the plaza, specifically in the southbound direction. Vehicles approaching southbound make a sweeping right turn approaching the plaza. This movement creates a tendency for southbound vehicles to

travel through toll lanes on the outside of the curve (interior of the plaza) and reduces utilization of the tollbooths on the inside of the curve. Traffic that is not uniformly distributed in the plaza reduces operational efficiency, with some lanes over-utilized and some underutilized. While a certain amount of non-uniform usage is common at plazas, the existing roadway curve exacerbates the skewed distribution and therefore results in an undesirable condition.

### 5. Vertical Geometry

The existing York Toll Plaza is located at the low point of a hill that begins just north of the plaza (Figure 1.3). The FHWA Guidelines recommend toll plazas be located at the crest of a vertical curve. Locating the plaza on a high point will increase sight distance and provide operational benefits, as the approach up-grade will aide in slowing vehicles and the departure down-grade will aide in accelerating vehicles. The existing vertical geometry presents undesirable conditions for traffic departing northbound and approaching southbound. The northbound impact is primarily operational in nature, since the roadway north of the plaza includes a significant grade of 4.72% that impacts acceleration for departing vehicles, particularly heavy acceleration and the associated noise from trucks. There is currently a truck climbing lane in this area to mitigate this condition. Noteworthy is the fact that this is the only climbing lane on the Maine Turnpike and is required because of the requirement of heavy vehicles to stop or slow at the base of the hill. The southbound approach represents a concern from a safety perspective since it is on the downgrade of 4.72%. This creates a condition where vehicles (especially trucks) must brake sooner to compensate for the downgrade in addition to the significant speed reduction required in the plaza area. While the Maine Turnpike has a rule prohibiting excessive noises, this condition also contributes to some truck drivers using noisy engine brakes to assist with the deceleration. An additional safety concern associated with this down grade is the potential for vehicles which have lost their brakes to strike the plaza. The existing plaza location, as it relates to hills, is in a poor location from an operational perspective as well as from a safety perspective.



FIGURE 1.3 - Southbound Sight Distance Horizontal curve and down gradient are not desirable due to safety and operational concerns.

### 6. Toll Booths and Safety Bumpers

The original tollbooth structures were designed in the 1960s and are considered deficient by today"s standards from a space, layout, protection, and systems perspective. The original design did not anticipate the need for additional equipment required by modern technology such as computers and ETC systems. The current booths have limited space for collector activities and become extremely crowded during peak periods when all lanes are open, requiring one booth to have two attendants serving both directions (Figure 1.4). Current toll islands are designed for these smaller booths and will not accommodate the larger, modern booths as installed at other locations on the Maine Turnpike. Existing heating systems are outdated, take-up more space than modern components, and only provide a minimum amount of comfort. Modern booths are assembled with the latest heating and ventilating systems to provide better comfort. For additional information on toll booths, safety bumpers, and other toll plaza components, refer to Appendix C, What is a Toll Plaza?



FIGURE 1.4 - York Toll Booth Note, tight quarters for toll attendants when booth operated in both directions.

Current standards for toll booths incorporate a double concrete bumper to provide safety for the toll collector and to redirect an errant vehicle into its lane. The present bumpers are nearly non-existent as shown in Figure 1.5 compared to a newer bumper shown in Figure 1.6 on the following page. This is due to poor soil conditions in the area which is allowing these bumpers to settle. Soil settlement is discussed in more detail in Subsection 9 (Soil Conditions) of this section of the report.



FIGURE 1.5 - York Toll Booth, Single Bumper and Settled Island



FIGURE 1.6 - New Gloucester Toll Booth, Double Bumper and Raised Median

### 7. Tunnel

A narrow tunnel is located under the York Toll Plaza to serve as the main passageway for employees to safely access the toll booths and as a utility corridor to and from the individual booths. The tunnel is in poor condition and in need of rehabilitation. The tunnel is located in an area of high groundwater and experiences significant water infiltration. The tunnel ceiling has numerous cracks and utility penetrations which also allow for the infiltration of surface water into the tunnel. From a safety perspective, having water in the tunnel is undesirable due to the electrical and communication utilities present, as well as for the turnpike employees using the tunnel. Figure 1.7 illustrates water seepage and staining of the concrete in addition to the significant corrosion to the utilities. The majority of these utilities were added to accommodate electronic tolling.

These additions have reduced the passage width as well as increased the leaks and safety concerns.



FIGURE 1.7 - Conduit Penetrations in York Tunnel

Figure 1.8 illustrates the numerous utilities in the tunnel along with staining of the floor due to leaks in the tunnel. Many repairs have been completed in the tunnel to mitigate the water infiltration but it remains an ongoing maintenance concern.



FIGURE 1.8 - Numerous Conduit Runs in York Tunnel

Figure 1.9 demonstrates the narrow width of York's tunnel compared to the wider tunnel at the New Gloucester Toll Plaza shown in Figure 1.10. The extensive costs associated with a comprehensive tunnel repair are comparable to the costs for a new tunnel.



FIGURE 1.9 - York Tunnel Note, leak stains and narrow passageway.



FIGURE 1.10 - New Gloucester Tunnel Note, wide passageway.

### 8. Canopy

A canopy is located over the toll lanes as seen in Figure 1.11. The structural supports for the existing canopy are at capacity due to the signage that has been placed on the structure over time. Typically the placement of electronic variable messages signs on the canopy allows staff to change messages such as "Any Vehicle", "E-ZPass", and "Lane Closed". However, the installation of these larger and heavier signs is not feasible due the condition of the existing canopy.



FIGURE 1.11 - Canopy and Signs at York Plaza

### 9. Soil Conditions

The existing plaza was built in an area with poor subsurface soil conditions, mainly consisting of compressible clay. With this site condition recognized in the design, the plaza tunnel, booths, and canopy were constructed on foundation piers to prevent settlement of the entire structure due to consolidation of the clay soils. However, the roadway approaches to the plaza were not pier-supported. As a result, the approaches have and continue to settle as the clay soil consolidates. In an effort to mitigate the ongoing settlement of the roadway approaches, pavement overlays and shim courses have been added routinely thus minimizing the steep approach grade to the plaza. Even with the pavement shimming work, the plaza has a noticeable slope approaching and leaving the plaza, with the roadways settling away from the pier-supported plaza. This can be seen in Figure 1.12 and depicted in Figure 1.13 Settlement Schematic. This approach settlement has created a range of adverse conditions, from low bed tractor trailers striking the concrete slab (See Figure 1.13) to excessive settlement of the approach slabs and concrete safety bumpers (shown in Figures 1.5 and 1.6). Low bed tractor-trailers that strike the concrete roadway slab with their trailer bottoms often get stuck and increase potential for vehicle accidents, and settlement of the approach slab and concrete bumpers reduces the ability of the bumpers to absorb vehicle collisions increasing risk to toll plaza staff and patrons. Both conditions result in safety concerns.



FIGURE 1.12 - Settlement of Approach Slab Note, abrupt rise at plaza.

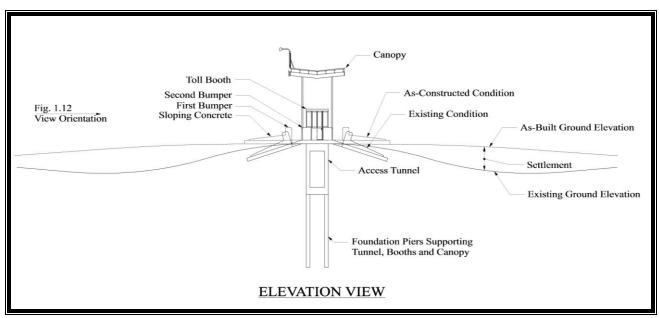


FIGURE 1.13 - Settlement Schematic



FIGURE 1.14 - Damaged Concrete Slab at Plaza

### 10. Summary of Existing Conditions

In summary, the existing plaza - considering both infrastructure and location - is functionally obsolete. The facility is nearly 40 years old and not conducive to safe operation with today"s traffic volumes and speeds. With respect to the FHWA"s current Design Guidelines and Best Practices, the plaza"s layout and location are non-conforming to many standards. The plaza is too close to an interchange; located on a curve, which does not provide adequate sight distance from the north or south; and, is too close to the Chase's Pond Road bridge, further limiting decision sight distance. In combination, these deficiencies lead to the plaza being rated as the 11<sup>th</sup> highest crash location in the state.

The plaza is at the bottom of a hill requiring heavy acceleration noise and engine braking. In addition, the plaza is located in an area where excessive differential soil settlement is causing some low-bed tractor trailers to strike the concrete slab while paying tolls and often getting stuck on the slab. With respect to the plaza, the toll booths and lanes are too narrow; the canopy is reaching its structural carrying capacity; and, the employee access tunnel is narrow, leaking, and unsafe. All of these current deficiencies (which are expected to continue to worsen with time) impact the safety of turnpike staff and patrons, as well as increases overall operation and maintenance costs. Additional information on maintenance costs can be found in Appendix H, Renewal & Replacement – Maintenance Program.

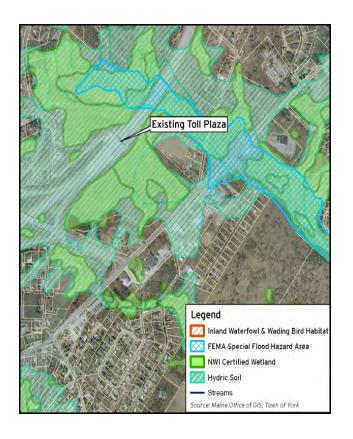
Based on these findings, it is prudent to complete an evaluation of the existing site in the form of upgrade options addressing the multitude of physical and operational deficiencies. These upgrade options are presented in Part 2 of this report, Existing Site Evaluation. Similar evaluations of potential new plaza locations are presented in Part 3, Alternate Site Evaluation.

# MAINE TURNPIKE SOUTHERN TOLL PLAZA

# **EXISTING SITE EVALUATION**







June 16, 2009

The Maine Turnpike Authority



Prepared by: HNTB Corporation



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#### **SECTION 1 – INTRODUCTION**

The Maine Turnpike Authority"s York Toll Plaza is situated seven miles north of the New Hampshire border and has served beyond its planned and structural life. It is processing more than three times the traffic it did when it first opened and is suffering from numerous operational and structural deficiencies and continues to be a safety concern. As a result of these factors, several years ago, the Maine Turnpike Authority (MTA) decided to curtail expending money on all non-critical repairs and to comprehensively evaluate the existing plaza issues and investigate how to most effectively move forward with a replacement that meets the Authority"s goal of operating a safe, efficient and modern southern toll plaza.

The Maine Turnpike Authority has since engaged this study and has released a number of findings, including a report at the beginning of 2008 titled Technical Report in Response to Maine LD534. (LD534, Resolve, Directing the Maine Turnpike Authority To Study the Relocation of the York Toll Booth, is a Legislative Document generated by the Joint Standing Committee on Transportation) The response report was essentially a compendium of existing conditions, deficiencies and other safety related findings to date that supported the need for the York plaza replacement. In fact, it detailed the finding that a new plaza in a new location would better meet the safety, capacity, design criteria, and modern toll technology goals than numerous options at the existing site. Following the presentation of the response report to the Transportation committee, the MTA held several meetings with the public and local officials to discuss these and other findings. At the urging of the York Board of Selectpersons, the MTA Board agreed to request that it "s Chief Consulting Engineering Firm, HNTB revisit the "existing site evaluation". As requested by the Selectpersons, the goal was to investigate out-of-the-box or "what it would take" alternatives that would meet design criteria, minimize impact to right-of-way and avoid taking homes.

The purpose of this Report is to document the evaluation of options for rehabilitating/reconstructing the York Toll Plaza at its existing site or in close proximity and to recommend any option(s) that warrant being carried forward for further consideration. This report will become Part One of the full Site Identification and Screening Report. The Site Identification and Screening Report will then evaluate the most reasonable existing site option(s) along with screened new sites in the identified corridor and ultimately make a recommendation for the replacement of the York Toll Plaza. Existing site evaluation along with alternative site analysis are requirements of the environmental permitting agencies prior to them issuing necessary permits.

A complete and thorough evaluation must include such alternatives that meet purpose and need, create the least amount of environmental and community impact and are practicable. Recommendations from this report shall reflect the following goals that MTA has for rehabilitating/reconstructing the York Toll Plaza:

- 1. Impacts to property and the environment shall be minimized.
- 2. The design shall be fiscally responsible considering both initial construction and long term maintenance costs weighed against benefits realized over the life of the design.
- 3. The plaza shall have safer operations for both Turnpike patrons and staff.
- 4. The plaza shall have adequate capacity for current and future traffic demands.
- 5. The plaza design shall meet industry design standards for layout and operations.
- 6. The plaza shall have the ability to implement a more modern and efficient Open Road Tolling (previously referred to as Highway Speed Tolling) technology as decided by the MTA Board. The Maine Turnpike Authority has made a decision to implement Open Road Tolling.

#### **SECTION 2 - DESIGN GUIDELINES FOR MAINLINE TOLL PLAZAS**

While the construction and expansion of the mainline of the Maine Turnpike (The Widening) benefited from established and updated highway design guidelines, such national and uniform guidelines were not available for toll plazas when the York Plaza was built in 1969. However, in 2006, responding to the needs of many tolling operations across the country, the Federal Highway Administration issued a report that documented the most current best practices and established new guidelines for the design and construction of toll plazas. These guidelines and best practices are focused primarily on the design and construction of toll booths and toll lanes and how these structures interface with mainline traffic operation

Design guidelines are assembled to provide planners and engineers with a set of current "best practices" to provide safe and efficient facilities. These guidelines are developed nationally from experience in a wide variety of specific discipline areas and conditions. Guidelines have been developed for the highway and roadway practice area, which apply to turnpikes and toll plazas. Following is a list of the national design guideline publications being used for evaluation of the York Toll Plaza to provide users with a safe, efficient and environmentally conscious facility.

- A Policy on Geometric Design of Highways and Streets, AASHTO 2004
- Manual on Uniform Traffic Control Devices (MUTCD), FHWA 2003
- Roadside Design Guide, AASHTO 2006
- State of the Practice and Recommendations on Traffic Control Strategies at Toll Plazas, FHWA 2006

Further discussion of the details of these design guideline publications follows.

### A. Purpose of National Design Guidelines

Excerpts from these various Guidelines, highlighting their purpose as well as the various basic design criteria mentioned, are contained in Appendix A.

### 1. A Policy on Geometric Design of Highways and Streets:

Excerpt from page xliv: "These guidelines are intended to provide operational efficiency, comfort, safety and convenience for the motorist. The design concepts presented herein were also developed with consideration for environmental quality. The effects of the various environmental impacts can and should be mitigated by thoughtful design process. This principle, coupled with that of aesthetic consistency with the surrounding terrain and urban setting, is intended to produce highways that are safe and efficient for users, acceptable for non-users, and in harmony with the environment."

### 2. Manual on Uniform Traffic Control Devices (MUTCD):

Excerpt from Section 1A.01 Purpose of Traffic Control Devices: ,<u>The purpose of traffic control devices</u>, as well as the principles for their use, is to promote <u>highway safety</u> and <u>efficiency by providing for the orderly movement of all road users on streets and highways throughout the Nation</u>."

Excerpt from Section 1A.06: "Uniformity of devices simplifies the task of the road user because it aids in recognition and understanding, thereby reducing perception/reaction time. Uniformity assists road users, law enforcement officers, and traffic courts by giving everyone the same interpretation. Uniformity assists public officials through efficiency in manufacture,

maintenance, and administration. Uniformity means treating similar situations in a similar way. The use of uniform traffic control devices does not, in itself, constitute uniformity. A standard device used where it is not appropriate is as objectionable as a nonstandard device; in fact, this might be worse, because such misuse might result in disrespect at those locations where the device is needed and appropriate."

### 3. Roadside Design Guide:

Excerpt from Preface page vii: "The *Roadside Design Guide* is developed and maintained by AASHTO subcommittee on Design, Technical Committee for Roadside Safety. The guide presents a synthesis of current information and operating practices related to roadside safety ..."

A second noteworthy point is that this document is a guide. It is not a standard, nor a design policy. It is intended for use as a resource document from which individual highway agencies can develop standards and policies. While much of the material in the guide can be considered universal in its application, there are several recommendations that are subjective in nature and may need modification to fit local conditions. However, it is important that significant deviations from the guide be based on operational experience and objective analysis."

## 4. <u>State of the Practice and Recommendations on Traffic Control Strategies at Toll Plazas:</u>

Excerpt from page 1: "The goal is to achieve a consistent strategy for handling potential points of conflict, controlling flow of various vehicle types and conveying information at toll plazas so that <u>safety</u> and <u>operations</u> are enhanced, better <u>efficiency</u> and economy of design are achieved, and motorist recognition and comprehension are improved."

Excerpt from page 2: "Further trends show toll roads facing greater commuter and recreational demands, resulting in cash paying and ETC users familiar with the toll road mixed with unfamiliar cash paying users. Without the use of good design practice, including effective deployment of various traffic control devices, this mix can result in unsafe and inefficient operations. ETC users now expect non-stop, high speed travel through toll plazas without incurring any delays. Development of national guidelines that address the implications of electronic toll collection on plaza operations has therefore become much more critical.

The common theme among these guidelines, as it relates to their purpose, is that uniformity of design practices and procedures is a key factor in the safety of travelers on our Nation's highways. In addition, operational efficiency of our roadway network can be improved through the use of these national guidelines and best practices. Another important result of applying these guidelines is the efficient use of resources while minimizing environmental impacts. Evaluation of the existing toll plaza will be based on these design manuals to develop a fair and reasonable summary of findings; setting the stage for rehabilitation strategies that are safe, efficient, economical, and environmentally sensitive.

HNTB will then utilize these national guidelines to develop, analyze and compare plaza alternatives resulting in final engineering recommendations that meet acceptable design practice. Ultimately, it is HNTB's goal to utilize these national guidelines, along with professional judgment, to maximize the safety of the traveling public and to the MTA toll staff while also providing the best value to the Maine Turnpike toll-payers. The development of a toll plaza design that ignores industry standards, acceptable design practice, and nationally published design guidelines increases the safety risks to drivers and toll staff alike, is not supported by HNTB and should not be considered by the MTA.

### B. Basic Design Criteria for Toll Plazas

The next portion of the analysis is to detail the guidelines to be used for the design, location and implementation of traffic control strategies for toll plazas as well as to be used in the evaluation of an existing toll plaza. The following guideline criteria are documented in the Federal Highway Administration State of the Practice unless otherwise noted:

- Provide one mile (5,280 ft) minimum separation between toll plaza and interchanges. A one mile separation affords drivers with adequate time to interpret signs, maneuver accordingly and minimizes other decisions and distractions. A toll plaza placed near an interchange increases traffic weaving issues, signing difficulty, a wide range of vehicle speeds and general driver confusion.
- Provide adequate decision sight distance (DSD) in advance of the toll plaza. DSD, as defined by AASHTO, is the distance needed for a driver 1) to detect an unexpected or otherwise difficult to perceive information source or condition in the roadway environment that may be visually cluttered, 2) recognize the condition or its potential threat, 3) select an appropriate speed and path, and 4) initiate and complete the maneuver safely and efficiently. For open road (highway speed) tolling, the DSD requirement is composed of two sight distances: 1) 1,500 ft before the split point between open road and conventional cash lanes and 2) 1,800 ft between the split point and the plaza. At a point 3,300 ft prior to the plaza (total of these two values), the driver shall be able to see the split point as well as the plaza so that the driver can maneuver as necessary. This 1,500 ft DSD assumes vehicles are traveling at 70 mph and advance signing is provided in accordance with FHWA Guidelines. The second distance of 1,800 ft between the split and the plaza is based on the geometrics of the plaza. At the split point 1,800 ft prior to the plaza, the driver should also be able to clearly see the toll plaza.
- Resulting from the above DSD recommendation Provide 3,300 ft separation between toll plaza and overhead structures. This distance is based on previously described DSD criteria. The driver should have unobstructed views of the split point and plaza, thereby improving facility safety. This requirement will also reduce or eliminate potential impacts to existing overhead structures. Overhead structures and bridges have two components that can restrict sight, one being the bridge itself and the other being the abutments and piers. These components can block view of signs, impact depth perception and in some cases require guardrail further blocking views of conditions existing on the far side of the bridge.
- Locate toll plaza on a horizontal tangent (straight section) with no curves. Locating a toll plaza on a tangent (straight section of roadway) improves sight distance, driver awareness, and facility safety when compared to a location on a horizontal curve. Placing a toll plaza on a curve: reduces driver sight distance, causes additional distractions to drivers thereby increasing potential for crashes, reduces plaza operational efficiency as some booth lanes will be over utilized and some underutilized, and may create engineering challenges relating to roadway cross slopes and super elevation needs.
- Locate the toll plaza on a roadway high point. Placing a toll plaza at the crest of a hill will provide sight distance advantages for all traffic and plaza operational benefits to cash patrons as the approach upgrade will aide in slowing vehicles down while the departure downgrade will aide in accelerating vehicles. This reduces the amount of engine braking and heavy acceleration noises often associated with the plaza. FHWA Studies have been done to

determine acceptable levels of grade approaching and departing a toll plaza. Grades 3.0% and steeper have an adverse affect on the performance of commercial vehicles and grades less than 0.5% create drainage problems and possible icy conditions in the winter. Therefore, grades approaching and departing the toll plaza should be within the range of 0.5% to 2.0%.

The following table further describes key issues addressed by the above guidelines as well as describing their impact on safety, operations and the environment.

## Figure 1 Design Guideline Summary Matrix Maine Turnpike Southern Toll Plaza

Design Criteria	Safety			Operations			Environment			
	Most Safe	Least Safe	Explanation	Best operationally	Worst Operationally	/ Explanation	Least Environmental Impacts	Most Environmental Impacts	Explanation	Summary
1. Separation from interchange										
Weaving of Traffic	Toll plaza separated from interchange by at least 1 mile	Toll plaza and interchange located at same location	Interchanges - Mainline driver in the left and middle lanes planning to exit at an interchange move into the right lane prior to approaching an off ramp. Mainline drivers in the right lane not using the interchange, often move into the middle lane to avoid decelerating and accelerating vehicles in the right lane. Toll Plazas - Mainline drivers approaching a toll plaza typically change lanes in advance of a toll plaza. Providing a minimum of a 1 mile separation between an interchange and a toll plaza distributes the weaving vehicles (vehicles changing lanes) over a larger area thus reducing the concentration of weaving vehicles. A lower concentration of weaving vehicles typically equates to a lower number of collisions. Therefore, a 1 mile separation between an interchange and a toll plaza is likely to result in less collisions.	Toll plaza separated from interchange by at least 1 mile	Toll plaza and interchange located at same location	Interchanges - Mainline driver in the left and middle lanes planning to exit at an interchange move into the right lane prior to approaching an off ramp. Mainline drivers in the right lane not using the interchange, often move into the middle lane to avoid decelerating and accelerating vehicles in the right lane. Toll Plazas - Mainline drivers approaching a toll plaza typically change lanes in advance of a toll plaza. Providing a minimum of a 1 mile separation between an interchange and a toll plaza distributes the weaving vehicles (vehicles changing lanes) over a larger area thus reducing the concentration of weaving vehicles. A lower concentration of weaving vehicles typically equates to a higher capacity. Therefore, a 1 mile separation between an interchange and a toll plaza should is likely to result in higher capacity.	Toll plaza separated from interchange by at least 1 mile	Toll plaza and interchange located at same location or in close proximity	Additional mainline travel lanes could be constructed to decrease the concentration of weaving vehicles resulting in an increase in safety and capacity. Additional lanes would likely impact wetland and streams	Toll plazas and interchanges separated by at least 1 mile results in the highest safety, the best operations, and the least environmental impacts
Highway signing	Toll plaza separated from interchange by at least 1 mile	Toll plaza and interchange located at same location	Highway guide signs are suggested to guide motorist to their intended destination. National guidelines suggests that the same basic message be repeated multiple times starting 2 miles in advance. This allows adequate time for a driver to read, understand, and react to a message. (Note that vehicles traveling at the posted speed of 65 mph (95 feet/sec) will travel hundreds of feet while drivers see a sign, read and understand the message, decide on an action, and then implement the action.) Signs should be consistent and easily understood. Signing for both the toll plaza and the interchange within the 2 mile corridor requires multiple signs with separate and distinct messages which can create confusion for the driver. A confused driver is more likely to be involved in a collision than a non-confused	Toll plaza separated from interchange by at least 1 mile	Toll plaza and interchange located at same location	Multiple signs create confusion and may lead to drivers not choosing their correct course of action (For example - May result in driver missing an exit). This condition results in substandard operations.				
2. Horizontal Alignment	Toll Plaza located on a straight section of roadway	Toll Plaza located on a curve	Toll plazas located on a straight section of road are more visible to the driver than a toll plaza located on a horizontal curve. This allows for adequate decision sight distance (DSD). DSD is the distance required for a vehicle traveling at 70 mph to detect an unexpected condition, recognize it's potential threat, select an action, and implement the action. High visibility leads to increased safety as a driver can see the toll plaza and start to make decision such as decreasing speed and changing lanes well in advance of the toll plaza. This provides for increased safety as the concentration of weaving vehicles is decreased.	Toll Plaza located on a straight section of roadway	Toll Plaza located on a curve	Drivers tend to stay on outside of curve. This results in the booths on the outside of the curve being heavily utilized while booths on the inside of the curve are underutilized. This condition decreases the overall capacity of the toll plaza which results in congestion when the demand exceeds the capacity. Congestion results in poor operations.	Not Applicable	Not applicable		Toll plazas located on a straight section of roadway results in the highest safety and the best operations
3. Vertical Alignment (profile)										
Grades	Up grade entering toll plaza and down grade leaving toll plaza	Down grade entering toll plaza and up grade leaving toll plaza	Gravity (downhill pull) positively influences vehicles ability to decelerate when vehicle is traveling uphill. A vehicle approaching a toll plaza climbing a steep hill will decelerate without the use of brakes. Therefore, steep upgrades to toll plaza minimizes the potential of serious collisions since gravity helps to decelerate vehicle which reduces the speed. Specific concerns include vehicles with faulty breaks and non attentive drivers.	Up grade entering toll plaza and down grade leaving toll plaza	Down grade entering toll plaza and up grade leaving toll plaza	Gravity (down hill pull) positively influences a vehicles ability to accelerate when traveling downhill. A vehicle leaviing a toll plaza on a down grade can move forward without the use of the engine. Upon leaving the toll plaza, a downgrade will facilitate the acceleration of the vehicle.	upgrade entering plaza and downgrade exiting toll plaza	down grade entering plaza and up grade exiting toll plaza	The use of gravity to assist with vehicle deceleration (entering plaza) and vehicle acceleration (departing plaza) minimizes fuel consumption, noise associated with braking,	Toll plazas on a high point at the end of a long tangent with a 2% up grade entering the plaza and a 2% down
Vertical Curves	Toll plaza located at end of long straight (tangent) section of roadway	Toll plaza located just beyond crest of hill	Toll plazas located at the end of a straight section of road are more visible to the driver than a toll plaza located just beyond the crest of a hill. This allows for adequate decision sight distance (DSD). DSD is the distance required for a vehicle traveling at 70 mph to detect an unexpected condition, recognize it's potential threat, select an action, and implement the action. High visibility leads to increased safety as a driver can see the toll plaza and start to make decision such as decreasing speed and changing lanes well in advance of the toll plaza. This provides for increased safety as the concentration of weaving vehicles is decreased.	Toll plaza located at end of long straight (tangent) section of roadway	Toll plaza located just beyond crest of hill	Toll plazas located at the end of a straight section of road are more visible to the driver than a toll plaza located just beyond the crest of a hill. High visibility allows the driver adequate time to see the toll plaza and start to make decision such as decreasing speed and changing lanes well in advance of the toll plaza. This decreases the concentration of the weaving and results in higher capacity	Not applicable	Not applicable	Not applicable	piaza and a 2% own grade leaving the plaza conform to the accepted national guidelines. This guideline reflects a balance of the safety, operational, and environmental concerns
4. Proximity to Bridges	Toll plaza located over 3500' feet from overhead bridge structure		Toll plazas located at least 3500' from an overhead bridge are more visible to the driver than a toll plaza located just beyond an overhead bridge. This distance allows for adequate decision sight distance (DSD). DSD is the distance required for a vehicle traveling at 70 mph to detect an unexpected condition, recognize it's potential threat, select an action, and implement the action. High visibility leads to increased safety as a driver can see the toll plaza and start to make decision such as decreasing speed and changing lanes well in advance of the toll plaza. This provides for increased safety as the concentration of weaving vehicles is decreased. An overhead bridge within the plaza area may require intermediate piers. The piers, as well as their protection (Guardrail, impact attenuator, etc.) are a hazard and would likely results in more collisions.	Toll plaza located over 3500' feet from overhead bridge structure	Bridge structure located in close proximity to toll plaza	Toll plazas located at least 3500' from an overhead bridge are more visible to the driver than a toll plaza located just beyond an overhead bridge. High visibility allows the driver adequate time to see the toll plaza and start to make decision such as decreasing speed and changing lanes well in advance of the toll plaza. This decreases the concentration of the weaving and results in higher capacity	Not applicable	Not applicable	Not applicable	Toll plazas located at least 3500' from overhead bridges providee the highest safety and the best operations.
5. Toll Plaza Capacity	Toll plaza can process peak traffic without congestion in the mainline section. Delays are minimized	Toll plaza can not process average traffic and congestion extends into mainline section	Congestion on the mainline (3 lane section of roadway - outside of plaza area) has high potential for serious collision as mainline drivers traveling at 65 mph are not expecting stopped traffic on the mainline.	Toll plaza can process peak traffic without congestion in the mainline section. Delays are minimized		Congestion in the mainline has high potential for vehicles to divert to alternate routes to avoid congestion	Minimal number of toll lanes	Large number of toll lanes	large number of toll lanes likely to have larger wetland and stream impact than minimal number of toll lanes	Toll Plaza should have adequate capacity to process traffic such that traffic does not become congested in the mainline section

Alternate Location

Existing Location

Not applicable

## **SECTION 3 - PROJECT PURPOSE & NEED**

### A. Project Purpose

The purpose of the York Toll Plaza Replacement Study is to 1) identify structural, operational and safety deficiencies at the (York) toll plaza, and 2) propose a course of action that will ultimately result in a toll plaza that is considered safe, efficient, economical and satisfies the MTA"s goal of incorporating open road tolling. HNTB"s final project recommendation will take into consideration Turnpike operational parameters, engineering design criteria, capital and operational costs, and physical features including natural resources, cultural resources, and community resources. The final project recommendations should accommodate current and future traffic needs safely and efficiently, utilize nationally recognized design guidelines, provide the best value, and meet the requirements of the environmental permitting agencies. The basic project Purpose and Need, as proposed to the U.S. Army Corps of Engineers (USACOE) and subsequently approved/accepted by USACOE, is contained in Appendix B of this report. In addition, and to assist in understanding the various components of a toll plaza, please refer to Appendix C - What Is a Toll Plaza? The appendix contains a brief description of these components and an accompanying diagram.

#### **B.** Project Need

The need for the project can be separated into two areas, physical and operational. First, the physical needs are due to the poor and failing condition of the physical infrastructure itself including booths, canopy, access tunnel, the space limitations of the existing tollbooths, the absence of adequate toll staff protection, and the poor soil conditions. Second, the operational needs are demonstrated by the design deficiencies of the existing York Toll Plaza; a plaza and approach area that restricts operational efficiencies and meets none of the recently published FHWA design guidelines for toll plazas. Proximity to an interchange, poor or non-existent sight distance and poor alignment have led to a high number of crashes resulting in the plaza being classified as the 11<sup>th</sup> highest crash rate location in the State out of over 900 such locations. Historically, near capacity operations along with unsafe vehicle weaving maneuvers further render the existing facility inadequate to perform safely into the future. Initial consideration of these issues, appeared to make upgrading the existing facility along with installation of open road tolling technology, infeasible. Details of these inadequacies and their consequences are described in greater detail later in the report.

### C. Summary

As stated in the Maine Turnpike Authority's enabling legislation,38M.R.S.A. §1961, the Legislature made the following findings of fact: "The economic and social well-being of the citizens of the State requires that the transportation system be developed in a comprehensive manner and depends upon the safety, efficiency and modern functional state of the turnpike."

Based on the York Toll Plaza's crash rate history and operational performance, it is clear that the present day plaza can not deliver, today or in the future, a "safe, efficient and modern operation", as required of the Turnpike. The York Toll Plaza is not in conformance with current best practices and design guidelines and is in need of major rehabilitation or replacement to improve operations and overall safety. Current deficiencies impact the safety of both Turnpike staff and the traveling public and increase overall operation and maintenance costs. Capacity improvements are also needed to more efficiently and safely process the traffic volumes at a reasonable level of service today and in the future. While the addition of tolling lanes and ETC have improved the plaza's capacity, additional ETC toll lanes or open road toll lanes are needed to efficiently meet the future traffic volumes.

Similarly, infrastructure upgrades including maintenance paving, safety bumpers, island rehabilitation, signage improvements, etc., have improved the overall operation for both patrons and employees. However, these upgrades have only been considered short-term improvements and have met only a portion of the total need.

The MTA decided in 2001 that the future needs of the entire plaza should be addressed and further short to mid term fixes or improvements would be curtailed. A more comprehensive evaluation was deemed necessary to determine immediate and future needs, including what type of modifications would be required to bring the plaza layout up to current design standards and best practices, and to determine what structural or infrastructure improvements would be required to provide proper safety for staff and travelers at and near the plaza itself.

This report documents the guidelines and standards by which toll plazas should be designed and operated and compares and contrasts various levels of rehabilitation and reconstruction that address some or all of these deficiencies. As part of improving the plaza operations, the report also documents benefits and shortcomings of various tolling strategies including conventional toll booths, electronic toll collection and open road tolling.

### **SECTION 4 - TOLL COLLECTION STRATEGIES**

Two types of toll collection systems are generally used in the industry today. One is the "ticket system" where motorists receive a ticket upon entering the system and then surrender the ticket and a cash toll upon exiting the system. The other is the "barrier system" where a set cash toll is charged based on a vehicle"s number of axles. The Maine Turnpike currently operates a barrier toll system with electronic toll collection (ETC) capabilities in all toll lanes. The Maine Turnpike also recognized the benefits to the traveling public of standardizing its toll collection with neighboring States and other states in the Northeast U.S. and therefore has adopted the E-ZPass system.

At all Maine Turnpike plazas, electronic tolls can be collected in a traditional stop-and-go cash toll lane as well as through a dedicated slow speed ETC lane. ETC in both stop—and-go cash lanes and dedicated ETC lanes requires patrons to slow to a maximum speed of 10 mph while passing through the plaza to ensure the safety of Maine Turnpike staff as well as their own. With the development of more sophisticated transponders and receivers, another ETC method, Open Road Tolling (ORT) allows ETC patrons to travel at highway speeds (55-65 mph) while paying their toll. For safe operations, these ORT facilities physically separate the ETC patron from the cash paying patrons. ETC patrons remain on the mainline of the highway and cash paying patrons exit to the right to a conventional toll plaza.

### A. Split Toll Plaza (Layout)

While not a tolling technology, split plazas are a tolling strategy and are frequently reviewed for potential benefits. Both a split toll plaza and a single toll plaza configuration have been considered as part of this study. A single plaza is a toll plaza where the northbound and southbound conventional plazas are built in the same location, whereas a split plaza has the northbound and southbound toll plazas in different locations. A split plaza could, in concept, reduce the mainline project footprint at any single location by dividing the total footprint between two locations, thus potentially reducing overall impact at any one location while creating plazas in two locations. However, a split toll plaza might result in greater overall project impacts and costs due to duplications of some facilities and additional earth disturbance required, e.g. from a second utility building, tunnel entrance, parking lot. A split plaza might have been appropriate if a single location, without major constraints, could not be found

The existing location of the York Toll Plaza was reviewed to determine whether or not this site could be used in one direction or the other. Conceptual plaza layouts were developed and analyzed, and the following conclusions were reached:

- 1. Critical FHWA design guidelines would be violated. These include:
  - Criteria related to proximity of adjacent interchanges
  - Criteria related to horizontal geometry decision sight distance
  - Criteria related to vertical geometry decision sight distance
- 2. The support infrastructure, i.e. building, parking and access, already exists on the southbound side. The existing plaza would have to serve SB traffic to utilize this infrastructure. However, to do this, all SB traffic (cash and ETC) destined for Chases Pond Road would likely be separated from the thru traffic to address the merge and weave issue. The ramp traffic, both cash and ETC might then be routed through booths dedicated specifically to the ramp to again minimize weaving maneuvers. This could be confusing and potentially dangerous for the ramp traffic that is not expecting to exit so far ahead of, and out of sight of, the Chases Pond Road crossing. The NB plaza would be located elsewhere on the mainline.

3. The costs required to address the existing physical deficiencies of the existing plaza, including the adverse soil conditions and failing tunnel, would be substantial and would nearly approach costs of an entirely new two-way plaza. Expenditure of substantial funds to rehabilitate the existing deficiencies would not be prudent when considering the fact that the resulting design features would be substandard and another toll plaza would need to be built for the other direction of traffic.

Since it would not be feasible to provide one direction of a split plaza at the existing York Toll Plaza location, there is no operational advantage to a split plaza. In fact there are several operational disadvantages to a split plaza:

- A split plaza could double the required number of supervisors;
- A split plaza would increase the number of toll attendants because they would no longer be able to switch between the northbound and southbound directions to accommodate peak traffic flows;
- A split plaza would require two sets of utilities;
- A split plaza would require two fully equipped support buildings;
- A split plaza would require up to four turnarounds for winter maintenance, whereas a single plaza would require up to two; and
- In addition to the operations and maintenance disadvantages, construction of a split toll plaza at two locations would cost more than a single plaza.

Therefore, further consideration of a split plaza at the existing or a new location would only occur if there were no suitable locations that would accommodate a single plaza.

### **B.** One Way Tolling

The Maine Turnpike Authority studied the concept of collecting tolls at York in only one direction in 2005. One-way tolling charges twice the one-way fare in one direction, while making the other direction toll-free. Typically, the concept of one-way tolling is used at bridges and tunnels to capture the high traffic volumes associated with peak commuting hours. The concept of one-way tolling in this area came to the forefront in August 2003, when New Hampshire's Governor authorized the New Hampshire DOT to conduct a one-way tolling experiment at the I-95 Hampton Toll Plaza. One-way tolling trials were conducted in the late summer/fall of 2003 and again during the summer of 2004. However, New Hampshire has discontinued these trials and has no plans to convert Hampton Toll Plaza to one-way tolling.

The complete One-Way Tolling Feasibility Study can be found in Appendix D. The Maine Turnpike Authority voted to cease further consideration of a one-way toll at the York Plaza based on the following findings.

- Local Diversion/Traffic Impacts. The average rate of diversion resulting from implementing one-way tolling is anticipated to be 11.7% or roughly 5,400 vehicles for an average day in 2007 shifting to local roads. (Present diversion rate is 2% 3%, as documented in the recent 2007 York Toll Diversion Study.)
- Loss in Revenue. Implementation of one-way tolling is anticipated to result in a net revenue loss of approximately \$2.0 million dollars per year.
- *Toll Opportunity*. Doubling the toll at York in one direction may limit the ability to effectively increase toll rates in the future. In addition to doubling the toll in one direction,

any future toll increase would also need to be doubled and added to that toll. For example, a 25¢ increase in each direction would be more acceptable than a 50¢ increase in one direction. Traffic diverting the plaza in one direction to avoid the 50¢ increase could be more appealing than diverting the plaza in both directions to avoid the 25¢ increase for each direction. Similarly, no tolls in one direction may cause an "attraction" to some vehicles for that direction of travel. A downside to this is these vehicles are not paying for their share of the upkeep.

### C. All Electronic Tolling

In 2006, the Maine Turnpike Authority voted and approved the concept that the replacement York Toll Plaza would be built incorporating highway speed toll lanes, also known as Open Road Tolling (ORT) for E-ZPass customers at the new plaza. ORT would allow E-ZPass users to pay their tolls electronically while traveling at normal highway speed (55-65 mph) by simply passing beneath sensors on the mainline of the highway. Cash paying customers would briefly exit the mainline of the highway to pay their tolls at a more traditional plaza. This decision was made after consideration of the potential benefits of ORT such as: improved safety, congestion relief, customer convenience, and capital cost savings, all weighed against some of the business costs associated with probable revenue leakage.

As part of the alternatives analysis related to the York Toll Plaza project, HNTB was commissioned to review the potential for All-Electronic Tolling (AET), also known as cashless and previously referred to as full Open Road Tolling. AET would eliminate all cash toll payments at the toll plaza. With AET, E-ZPass customers would continue to pay their tolls electronically, but at normal highway speeds. Tolls would be collected from non-E-ZPass users by capturing their license plates on video, using their license plate number to either match pre-paid license plate accounts or discover their mailing address and sending them a bill.

Since 2006, a small number of agencies have begun conversion or have set policies that state future installations will incorporate AET. A few more agencies have initiated extensive formal studies to evaluate the applicability of AET. Many other agencies are mainly waiting to see the results of these agencies" activities before conducting extensive assessments. It should be noted that although some agencies have committed to convert to AET, at the time of this review, no existing cash based agency has completed a total conversion to AET and therefore there is little to no available comparable information to assist other agencies with forecasting the applicability of AET for their own roadways. Furthermore, there is very little standardization of reporting of the business impacts of AET and much reluctance on the part of those agencies involved in AET to release documented and audited results of the business impacts. Considering the lack of information plus the broad range of local factors and the unique characteristics of each facility, a decision regarding use of AET cannot be based solely on what other agencies may be doing, but must consider the individual agency case in order to appropriately determine feasibility.

While the potential benefits of AET can be documented, the significant risk associated with the uncertainty behind the business costs of AET make the option of AET for the York Toll Plaza replacement unfeasible. The following points elaborate on this risk:

1. The traffic mix of the Maine Turnpike is such that a significant number of patrons are non E-ZPass users and from out of state or out of country. The extent to which these customers would not migrate to E-ZPass and/or pre-paid video products is uncertain and these factors greatly influence business costs such as operating costs and revenue losses. Current AET facilities

- typically have a high percentage of E-ZPass or similar accounts and have a high percentage of commuters and frequent resident users.
- 2. The current lack of industry data for similar roadways already implementing AET limits the ability to compare potential MTA outcomes and makes forecasting difficult to calibrate.
- 3. The uncertainty relative to how customers will respond to the changes in payment methods and the uncertainty relative to revenue recovery potential for violations pose too broad a range of potential outcomes. These include potentially significant risks to net revenue required to operate the roadway.
- 4. Difficulties attributed to the duplicate license plate numbering system and the ability of video systems to recognize the myriad of different plate types present minor operational challenges.
- 5. The resulting toll and fee structure for an AET system could result in actual or perceived unfair distribution of payments between Maine and out of state customers. This results when out of state violators do not pay because there is no significant enforcement capability. The structure is then set up or perceived to be set up to offset these losses by in-state paying patrons further compelled to pay because of threat of vehicle registration hold.
- 6. The ability to recover toll revenue from as much as 26 percent of the total traffic at York due to the lack of interstate legislation that would compel payment from out of state patrons weighs significantly in this risk. While in-state collection is backed by laws and enforcement opportunity, out-of-state and out-of-country collection lacks this enforcement and has perplexed toll agencies for over 10 years; and we believe that this issue will not be cured in the next 20 years.
- 7. Revenue risk also may result in non-compliance with bond covenants and debt service requirements.
- 8. The MTA may be limited in its ability to allow for certain types of post payment options typical for AET systems. For example, post payments of video tolls by customers are considered an extension of credit and any restrictions on how the MTA operates under these situations would need to be considered.
- 9. The cost of producing and mailing a bill for say a \$2 dollar toll will also need to be considered. Collection of this toll would include for example, computer processing of a license plate number, generation of license plate reports by State, request for registration name and address from State, generation of an invoice, envelope labeling, postage, mail opening, documentation of toll being paid, removal of open invoice from records, etc. This does not include any time or effort to respond to emails or phone calls explaining the invoice or any follow-up invoice.

Greater certainty around the potential impacts to toll operating costs and revenue impacts resulting from AET would be necessary to determine if the range of risks can potentially be mitigated to an acceptable level or if the risks are insurmountable. Based on the cost analyses conducted, the range of risk to the MTA resulting from uncertainties related to AET over 20 years could be as high as \$400 million. Therefore, given the revenue risk associated with the stated uncertainties, HNTB does not recommend AET for the York Toll Plaza at this point in time.

The complete All Electronic Tolling Report can be seen in Appendix E.

#### D. Open Road Tolling

Following is a brief summary of highway speed tolling, now known as open road tolling. To keep this summary consistent with the full report contained as an appendix, the phrase highway speed tolling or highway speed dedicated ETC lanes will be used instead of the currently recognized term of open road tolling or open road lanes. Following this summary, the remainder of the report will utilize the term open road tolling. The Maine Turnpike Authority has studied various means of collecting tolls

including two modes of electronic toll collection: (a) purely slow speed dedicated electronic toll collection (ETC) lanes, or (b) highway speed dedicated ETC lanes. The current York plaza, as well as many other MTA toll plazas, utilizes slow speed (10 mph) dedicated ETC lanes. The industry trend in the design of many new or replacement toll plazas incorporates highway speed (65 mph or similar) dedicated ETC lanes into the plaza design to take advantage of significant benefits associated with these designs. One factor in evaluating highway speed dedicated ETC lanes is the makeup of the vehicle stream. The southern portion of the Turnpike currently has a high enough percentage of E-ZPass customers, including a high percentage of heavy truck traffic, to be conducive to this tolling technology.

The benefits associated with the highway speed dedicated lanes specifically include:

- A highway speed toll plaza offers safety improvements due to the separation of non-stop from stopping traffic and reduction of the workers" exposure to fast moving traffic in the plaza area.
- Highway speed configurations can help to relieve congestion. Operational efficiencies from highway speed lanes present opportunity to more cost effectively manage traffic congestion at tolling points.
- Customer convenience increases with highway speed options. All ETC customers have the opportunity to travel at the posted highway speed through the plaza rather than the current 10 mph speed limit.
- Highway speed lanes have the potential to attract ETC customers through the expanded benefits offered by the new option. A high ETC customer base leads to a larger population of users making the most of the benefits of ETC and improves operations for the road operator.
- The benefits of highway speed lanes have the potential to attract cars from local roadways.
- Highway speed toll plaza configurations are potentially more cost effective. Preliminary cost estimates show that the cost of more complex toll equipment and infrastructure for a highway speed plaza is more than offset by the savings of not building additional manual toll lanes to handle the same throughput capacity as the highway speed toll lanes.
- The trend in the industry is to construct highway speed facilities. It is more cost effective and less disruptive to customers to build a new plaza with highway speed toll lanes than to renovate a plaza in the future to accommodate highway speed toll collection lanes.

However, in making the decision to incorporate highway speed lanes at future toll plazas, the Maine Turnpike Authority considered the following potential increases to business costs:

- Highway speed lanes will increase operational costs for back office and the customer service center due to initial and ongoing customer education, additional post processing of transactions and increased violation processing.
- Non-payment events at the plaza will likely increase due to patron unfamiliarity with the system and increased scofflaws. Other toll agencies who have installed highway speed lanes have experienced increases after conversion that lessens over time as a result of familiarization and enforcement.

In summary, the projected benefits outweigh the modest increase in business costs associated with highway speed tolling. The full Dedicated Electronic Toll Collection Lane Design Recommendations report can be found in Appendix F.

In light of these potential costs and benefits, and in comparison to other tolling technologies and strategies, the Maine Turnpike Authority made the decision to incorporate dedicated highway speed ETC lanes into the design of the future mainline toll plazas.

#### SECTION 5 - EXISTING YORK TOLL PLAZA SAFETY AND CAPACITY

This section documents the existing safety and capacity of the York Toll Plaza. This section also seeks to correlate the existing safety and capacity levels to overall plaza efficiency and operation and the fact that the existing York Toll Plaza does not meet several criteria relative to plaza design and layout. It is important to recognize that the existing York Toll Plaza was built with an expected life of 10-12 years. At thirty years beyond this intended life, the plaza faces major problems in terms of safety, efficiency and cost.

#### A. Safety

MaineDOT"s Crash Records Section summarizes all reported crashes in which there is property damage in excess of \$1000, or in which there has been personal injury. In order to summarize this information, the MaineDOT has established a Node and Link System. This system assigns a four-digit node number to each intersection, major bridge, railroad crossing, and crossing of town, county or urban compact lines. The segments of road that connect the nodes are referred to as links. As crash reports are received by MaineDOT, the information is assigned to the corresponding link or node at which they occurred. Appendix G provides crash data for the vicinity of the York Toll Plaza.

If a particular link or node meets certain criteria, then the MaineDOT classifies it as a High-Crash Location (HCL). These criteria are:

o The link or node must have eight or more reported crashes over the past three years <u>and</u> the link or node must have a "critical rate factor" (CRF) over 1.00. (The critical rate factor is a ratio of the crash rate at a particular link or node divided by the statewide crash rate average for a similar type of facility. The term "rate" is calculated by number of crashes divided by the number of millions of annual entering vehicles).

HNTB gathered recent MaineDOT crash data at and in proximity to the existing York Toll Plaza. Data was gathered for two, three-year time periods. The first was January 2003 through December 2005. The second was January 2004 through December 2006. Two sets of crash data were reviewed as the more recent crash data (04-06) became available during the course of preparing this report.

The following table provides a summary of MaineDOT crash data at the York Toll Plaza.

**Critical Rate** High Crash State Direction Years Location Factor (CRF) Location (Y/N) Ranking Approach 4.45 Yes 11 At Toll Plaza <1.0 2003-2005 No NA Departure <1.0 No NA Northbound Approach 3.53 Yes 17 At Toll Plaza <1.0 2004-2006 No NA Departure <1.0 NA No Approach <1.0 No NA At Toll Plaza <1.0 2003-2005 No NA Departure <1.0 NA No Southbound <1.0 Approach NA No At Toll Plaza <1.0 2004-2006 No NA Departure 1.28 Yes 320

Table 1 Crash Data at York Toll Plaza

Summary of the crash data reveals that the northbound approach to the York Toll Plaza is currently a HCL. The close proximity of the NB on-ramp for Chases Pond Road to the plaza contributes to unsafe merging of two streams of traffic as they are approaching a toll plaza. In fact, MaineDOT has ranked this NB approach as the 11<sup>th</sup> and 17<sup>th</sup> highest locations for the periods 2003-2005 and 2004-2006 respectively out of over 900 locations Statewide. It is worth noting that the toll plaza is not equipped with safety bumpers on the departing side of the toll lanes. This is particularly concerning since the middle lanes can be used in either direction and there is no guardrail or other physical separation to prevent errant vehicles from crossing into the opposite toll lanes and striking a toll booth from this unprotected side. Additionally, a HCL exists at the southbound departure where weaving occurs for traffic either taking the SB off-ramp to Chases Pond Road or continuing on the mainline. The locations can be seen on the aerial photo in Figure 2.

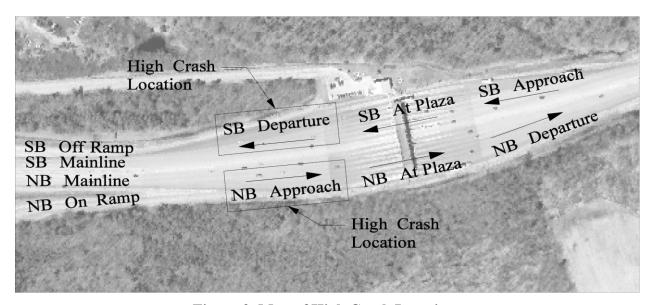


Figure 2 Map of High Crash Locations

A review of these HCL crash records reveals the majority of the crashes occurring were sideswipes/rear end. This is consistent with expectations given the close proximity of both the NB on and SB off-ramps to the York Toll Plaza and the inherent weaving and lane changing. Remedy for sideswipe type crashes would be to either separate ramp traffic from toll plaza/mainline traffic or to relocate the toll plaza farther away from the interchange. It is also worthy to note that as the E-ZPass customer base increases there will be an increase in the weaving and lane changes as these customers access the dedicated E-ZPass lanes. Along with this increase in weaving and lane change maneuvers comes an increased risk of additional and more serious crashes.

#### B. Capacity

The operations of the existing York toll plaza from 2009 to the design year of 2030 have been evaluated by comparing both the projected absolute peak hour and the projected 30<sup>th</sup> highest peak hour traffic volumes by direction with the capacity of the lane configuration. Capacity of the toll plaza varies based on number of lanes, mixture of cash and E-ZPass patrons, and processing rates during peak hour operations. The evaluation below uses an updated lane processing rate and cash/E-ZPass

patron mix based on a review of 2008 lane data as compared to previous analyses<sup>1</sup> done using more historic data. See Section 6 for more details on the processing rates. Northbound and southbound were analyzed separately.

## 1. Northbound Analysis

Experience has shown that queuing can be significant when a plaza exceeds 90% of its capacity. Based on the updated analysis, the northbound plaza does not exceed the 90% capacity level throughout the design horizon of the plaza for both absolute peak and 30<sup>th</sup> highest peak hours. This is shown in Table 2 and Table 3. Therefore, the NB plaza as currently configured is not likely to experience significant design hour queuing. However, even moderate queuing may at times restrict access to certain lanes and impact overall toll plaza operation. This has been observed in E-ZPass lanes where cash lane queues may block access to these lanes during peak periods due to existing plaza approach geometry.

In order to remain below capacity thresholds, it is critical to periodically alter the configuration of the plaza to reflect increasing traffic volumes overall. Between 2009 and 2030, it is anticipated the volume of E-ZPass customers will more than double while the volume of cash-paying volumes will decline by about 30%. Therefore, over time, cash lanes need to be converted to E-ZPass lanes in order to adequately serve the growing volume of E-ZPass patrons. This conversion is noted in Table 2.

In reviewing the data in Table 2, it is important to understand the following assumptions about the manner in which the table was developed:

- o The table assumes that 9 lanes are available to serve peak-hour traffic.
- o All E-ZPass lanes are slow-speed lanes (posted speed of 10 mph) with a capacity of about 1,100 vehicles per hour (vph).
- Cash lanes, while allowing E-ZPass transactions, operate with the following average capacities:
  - o Prior to 2013, while the cash toll is \$2.00, the capacity is estimated at **388 vph**.
  - From 2013 onward (after an assumed toll increase), the capacity is reduced to approximately **320 vph**.
- o The analysis does not identify times in which lanes could be eliminated. Rather, it identifies times in which lanes may be converted from cash to E-ZPass.
- O A new lane is converted from cash to E-ZPass as soon as the existing E-ZPass lanes are filled to capacity. For example, once the E-ZPass volumes exceed 2,200 vph, a 3<sup>rd</sup> E-ZPass lane is added, since two dedicated E-ZPass lanes can handle a maximum of 2,200 vph (assuming a per-lane capacity of 1,100 vph). Similarly, a 4<sup>th</sup> E-ZPass lane is added (and a cash lane removed) once the E-ZPass volumes exceed 3,300 vph. One caveat: the lanes are only converted **if** the remaining number of cash lanes is sufficient to meet the demand for cash-paying patrons.
- The table illustrates how the capacity of the plaza varies, based on (a) total volumes, (b) the mix of traffic (i.e. cash vs. E-ZPass), and (c) the configuration of the plaza (i.e. number of cash and E-ZPass lanes). It does not necessarily reflect how the plaza was

<sup>&</sup>lt;sup>1</sup> As compared to previous analyses conducted in the York Toll Replacement Technical Report In Response to Maine LD534 by HNTB, February 2008

operated in the past, and it is not necessarily a prescription for how the plaza should be operated in the future.

Table 2 Northbound Capacity of Existing Plaza, 2009-2030 – Absolute Peak Hour

Friday - Northbound													
Year		te Peak ume	Lane	% Capacity									
	Cash	E-ZPass	Cash	Tandems	E-ZPass	Toll Plaza							
2009	1,686	2,066	7	0	2	62.2%							
2010	1,654	2,162	7	0	2	61.0%							
2011	1,622	2,259	6	0	3	69.8%							
2012	1,591	2,356	6	0	3	68.4%							
2013	1,559	2,455	6	0	3	81.2%							
2014	1,528	2,554	6	0	3	79.6%							
2015	1,497	2,654	6	0	3	78.0%							
2016	1,467	2,754	6	0	3	76.5%							
2017	1,438	2,856	6	0	3	74.9%							
2018	1,409	2,958	6	0	3	73.4%							
2019	1,382	3,059	6	0	3	72.0%							
2020	1,353	3,163	6	0	3	70.5%							
2021	1,327	3,266	6	0	3	69.1%							
2022	1,301	3,370	5	0	4	81.4%							
2023	1,276	3,475	5	0	4	79.8%							
2024	1,252	3,579	5	0	4	78.3%							
2025	1,229	3,685	5	0	4	76.8%							
2026	1,205	3,792	5	0	4	75.3%							
2027	1,179	3,903	5	0	4	73.7%							
2028	1,153	4,016	5	0	4	72.0%							
2029	1,131	4,125	5	0	4	70.7%							
2030	1,107	4,238	5	0	4	69.2%							

Table 3 provides the same analysis at Table 2, but it is based on the volumes from the  $30^{th}$  highest hour. As the table indicates, in the NB direction, the plaza typically operates at 55-65% of its capacity during the  $30^{th}$  highest hour.

Table 3 Northbound Capacity of Existing Plaza - 30th Highest Peak Hour

Friday - Northbound												
Voor	30th High	h Volume	Lane	e Configura	ation	% Capacity						
Year	Cash	E-ZPass	Cash	Tandems	E-ZPass	Toll Plaza						
2009	1,531	1,876	7	0	2	56.5%						
2010	1,502	1,964	7	0	2	55.4%						
2011	1,473	2,052	7	0	2	54.3%						
2012	1,445	2,140	7	0	2	53.3%						
2013	1,416	2,230	6	0	3	73.8%						
2014	1,388	2,319	6	0	3	72.3%						
2015	1,360	2,411	6	0	3	70.9%						
2016	1,333	2,502	6	0	3	69.4%						
2017	1,306	2,594	6	0	3	68.0%						
2018	1,280	2,687	6	0	3	66.6%						
2019	1,255	2,779	6	0	3	65.4%						
2020	1,229	2,873	6	0	3	64.0%						
2021	1,205	2,966	6	0	3	62.8%						
2022	1,182	3,061	6	0	3	61.6%						
2023	1,159	3,157	6	0	3	60.4%						
2024	1,137	3,251	6	0	3	59.2%						
2025	1,116	3,347	5	0	4	69.8%						
2026	1,094	3,444	5	0	4	68.4%						
2027	1,071	3,545	5	0	4	66.9%						
2028	1,047	3,647	5	0	4	65.4%						
2029	1,027	3,747	5	0	4	64.2%						
2030	1,006	3,850	5	0	4	62.9%						

#### 2. Southbound Analysis

The updated analysis of the southbound plaza indicates that, during the absolute peak hour, the plaza will operate in excess of the 90% capacity level for every year from 2013 through 2030. As a result, significant queues are likely to occur in this direction during these hours. This is a critical point due to the existing geometry approaching the toll plaza. Queues from the manual lanes may block vehicles from accessing the right most lanes of the toll plaza and impact overall toll plaza operation.

During the 30<sup>th</sup> highest hour, the southbound plaza only occasionally reaches the 90% capacity level. At no point after 2009 does the capacity exceed 92%. Results of this analysis are shown in Table 4 and Table 5. The assumptions noted for Table 2 apply to these tables also.

In order to remain below capacity thresholds, it is critical to periodically alter the configuration of the plaza. Between 2007 and 2018, it is anticipated the E-ZPass volumes will increase by 125%, while cash-paying volumes decline by about 25%. Therefore, over time, cash lanes need to be converted to E-ZPass lanes in order to adequately serve the rapidly growing volume of E-ZPass patrons. This conversion is noted in the table below.

Table 4 Southbound Capacity of Existing Plaza - Absolute Peak Hour

	Sunday - Southbound													
Year		te Peak ume	Lan	% Capacity										
	Cash	E-ZPass	Cash	Tandems	E-ZPass									
2009	2,183	1,873	7	0	2	80.5%								
2010	2,151	1,973	7	0	2	79.3%								
2011	2,119	2,076	7	0	2	78.2%								
2012	2,087	2,179	7	0	2	77.0%								
2013	2,055	2,283	7	0	2	92.8%								
2014	2,024	2,388	7	0	2	92.6%								
2015	1,993	2,494	7	0	2	92.6%								
2016	1,962	2,601	7	0	2	92.4%								
2017	1,933	2,708	7	0	2	92.4%								
2018	1,903	2,816	6	0	3	99.2%								
2019	1,875	2,925	6	0	3	97.7%								
2020	1,846	3,035	6	0	3	96.3%								
2021	1,819	3,145	6	0	3	94.8%								
2022	1,793	3,256	6	0	3	93.4%								
2023	1,767	3,368	6	0	3	93.0%								
2024	1,743	3,479	6	0	3	93.4%								
2025	1,719	3,591	6	0	3	93.7%								
2026	1,694	3,707	6	0	3	94.1%								
2027	1,668	3,825	6	0	3	94.4%								
2028	1,641	3,945	6	0	3	94.7%								
2029	1,612	4,069	6	0	3	95.1%								
2030	1,582	4,196	5	0	4	98.9%								

Table 5 Southbound Capacity of Existing Plaza - 30th Highest Peak Hour

Sunday - Southbound												
Year		lighest ume	Lane	% Capacity								
	Cash	E-ZPass	Cash	Tandems	E-ZPass							
2009	1,922	1,649	7	0	2	94.7%						
2010	1,894	1,738	7	0	2	91.2%						
2011	1,866	1,828	7	0	2	88.1%						
2012	1,838	1,919	7	0	2	85.3%						
2013	1,810	2,010	7	0	2	82.7%						
2014	1,782	2,102	7	0	2	80.3%						
2015	1,755	2,196	7	7 0		78.4%						
2016	1,728	2,290	6	6 0 3		90.0%						
2017	1,702	2,384	6	0	3	88.7%						
2018	1,676	2,480	6	0	3	87.3%						
2019	1,651	2,576	6	0	3	86.1%						
2020	1,626	2,673	6	0	3	84.7%						
2021	1,602	2,770	6	0	3	83.5%						
2022	1,579	2,867	6	0	3	82.2%						
2023	1,556	2,966	6	0	3	81.1%						
2024	1,535	3,063	6	0	3	80.0%						
2025	1,514	3,162	6	0	3	78.8%						
2026	1,492	3,264	6	0	3	77.7%						
2027	1,469	3,368	5	0	4	91.8%						
2028	1,445	3,475	5	0	4	90.4%						
2029	1,419	3,583	5	0	4	88.7%						
2030	1,393	3,695	5	0	4	87.1%						

#### 3. Evaluation of Existing Measures to Improve Operation and Increase Capacity

Given the historic capacity and operational constraints of the existing York Toll Plaza, changing directional demand, and varying processing rates due to adjusted toll rates, the three middle lanes have been made reversible; i.e., the lanes can be operated for either northbound or southbound traffic depending on need. (Note: these lanes are always on the left for approaching traffic.) This introduces safety concerns and creates a situation that is contrary to the industry standard of locating dedicated ETC lanes on the far left side of available toll lanes; e.g., on the Maine Turnpike, currently one or more (reversible) cash lanes may be to the left of a dedicated ETC lane. As a result, in certain reversible lane configurations, slow speed ETC patrons now must travel between stopped traffic on both sides of them.

In order to marginally increase the capacity of the plaza, the Authority (since 2001) has implemented tandem booths during peak periods in the summer. This was intended to be a temporary measure as this is confusing for the Turnpike patron due to their unfamiliarity with the practice and only results in an additional capacity of 30%, or approximately 100 vehicles per hour. The use of tandem booths requires a flagger to direct drivers into the lane and two toll collectors per lane. In addition, their use presents accountability concerns relative to toll collector audits as

temporary booths do not contain standard lane computers for accounting and payment recording. Therefore, due to safety concerns of the flagger operating in the toll lanes, patron confusion, and accountability concerns, the extensive long-term use of tandem booths to address capacity needs is not desirable.

In summary; the need for reversible lanes and tandem booths, as presently utilized, will likely decrease over time due to the growth in E-ZPass usage and subsequent decrease in cash paying customers. Regardless, HNTB recommends the elimination of reversible lanes as they create safety concerns for both driver and toll staff. With respect to tandem booth, HNTB also recommends the elimination of their usage as they too create safety concerns for both driver and toll staff and provide little additional capacity.

#### SECTION 6 – PROPOSED TOLL PLAZA SIZING

Given the public interest in this study, the plaza sizing task has progressed well beyond the conceptual planning level of the rest of the report. As the York Toll Plaza Study has developed, there have been numerous conditions and sets of data that have shaped intermediate findings. Not the least of which is fluctuating and recently declining traffic volumes and a more critical look at toll plaza processing rates. Earlier planning level results of plaza sizing have therefore been updated to reflect these conditions. Following are details and a summary of the plaza sizing exercise.

A toll plaza should have adequate capacity to safely and effectively process the anticipated traffic without excessive queues and delays. However, unlike roadways and intersections which have national standards addressing capacity, no such standards exist for toll plazas. Each toll agency typically has its own goal as to adequate capacity. Historically, the Maine Turnpike Authority's goal has been to have a toll plaza meet two objectives throughout its design horizon of 20 years. One objective is to keep average delays during the absolute peak hours to approximately one minute or less. Another objective is to keep average queue lengths during the peak hours to 300 feet or less. These goals, which are intended to maximize patron convenience and safety, can also result in conservatively designed toll plazas, i.e. one with too long of a storage area or too many lanes.

HNTB recommends that the size of a proposed toll plaza, whether a conventional or open road design, be based on the 30<sup>th</sup> highest hour traffic volumes in each direction, i.e. the volume of traffic present in a single hour that is exceeded only 29 times in a typical year. This recommendation is based upon HNTB"s experience with toll plaza design and sizing in other locations around the country and balancing the operational and safety requirements as expressed by the Maine Turnpike Authority. Any toll plaza should be adequately sized to provide a reasonable level of operation (moderate queues and delays) for patrons, but at the same time account for real-life circumstances such as lane equipment failures and vehicle incidents which may block or close toll lanes for an extended period of time. Toll plaza sizing and layout should also take into consideration absolute peak volume operating conditions such that vehicle queues do not impact mainline traffic and create an undesirable safety situation. By using the 30<sup>th</sup> highest hour traffic volumes by direction, an appropriately sized plaza that best balances the needs of both patron convenience and Maine Turnpike operation can be achieved. While using the 30<sup>th</sup> high hour as the standard, HNTB also recommends analyzing traffic conditions during the absolute peak in order to ensure that toll plaza backups do not create an unsafe condition (such as backing up to the mainline).

The process of developing an appropriately-sized toll plaza for the Maine Turnpike is described below:

<u>Step 1</u> – Develop Design-Hour Volumes (DHV\*\*s). HNTB utilized the 30<sup>th</sup> highest hour traffic volumes by direction to determine the size of this mainline toll plaza. However, analysis was conducted for the absolute peak hour conditions to ensure that traffic will not back onto the mainline and create a safety issue or cause unreasonable delays.

<u>Step 2</u> – Develop traffic projections. In order to evaluate toll plaza operations throughout the design horizon of the toll plaza, it is necessary to estimate the extent to which design-hour traffic will grow over time. At the York Toll Plaza, historical data suggests that design-hour traffic growth will average approximately **1.66**% per year over the design life of the facility. Over the past two years, peak-hour traffic at the York toll plaza has actually declined. However, over a design horizon of a project such as this, a 1.66% annual growth rate provides a reasonable estimate of long-term growth.

Step 3 – Identify payment types. In order to properly analyze a toll plaza, it is critical to understand the peak-hour split between cash-paying patrons and E-ZPass patrons. Generally speaking, the efficiency of a given toll plaza increases as the percentage of E-ZPass patrons increases. In 2008, during peak summer weekends, approximately 45% of the peak-hour patrons at the York Toll Plaza had an E-ZPass<sup>2</sup>. It is also necessary to project how the share of E-ZPass patrons will change over time. Historic data and current industry trends suggests that the share of E-ZPass patrons will grow by approximately 3% annually in the next few years and thereafter the growth will slow over time to about 1% per year. At the York Toll Plaza, peak-hour usage of electronic toll collection has grown from about 10% in 1997 to roughly 45% in 2008.

The end result of Steps 1 through 3 is an estimate of the number of peak-hour patrons (both cash and E-ZPass) passing through the toll plaza during each year of the toll plaza"s design horizon. These volumes (for both 30<sup>th</sup> high hour and the absolute hour) were summarized earlier in Table 2 through Table 5.

<u>Step 4</u> – Perform initial plaza sizing and configuration. Based on the volumes, projection and payment types developed in Steps 1, 2, 3 it is possible to develop an initial estimate of the appropriate toll plaza size. At the York Toll Plaza, the following operating standards were used to determine plaza size:

- Patrons with an E-ZPass proceed through a conventional toll lane at a rate of 1,100 vehicles per hour (vph).
- Patrons with an E-ZPass proceed through a open road toll lane at a rate of 1,800 vph.
- The processing rate for patrons paying cash depends on (a) the toll charge itself, and (b) whether the lane is operating as a conventional lane or a tandem lane.
  - o \$2.00 Toll Conventional = 388 vph; Tandem = 500 vph
  - Other Toll Conventional = 320 vph; Tandem =  $415 \text{ vph}^3$ .

The end result of this step is to identify the total number of lanes (both cash and dedicated E-ZPass) required to handle the peak-hour volumes

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<sup>&</sup>lt;sup>2</sup> The actual share of E-ZPass varied by day and by direction. Friday traffic in the NB direction exhibited the highest share of E-ZPass usage (52%). By comparison, Sunday traffic in the SB direction registered about 43% E-ZPass usage. In general, time periods that serve commuting patrons (such as Friday afternoons) have a higher share of E-ZPass usage.

<sup>3</sup> Previous analysis has indicated a conventional toll lane processing rate of 289 vph. The rates cited in the "Other Toll" category are derived from observations at the York Toll Plaza during the time in which a \$1.75 toll was charged.

<u>Step 5</u> – Test via simulation. After estimating the appropriate size of the toll plaza, the performance of the proposed size is simulated via use of the VISSIM computer model. VISSIM is a driver behavior-based simulation program that is used to simulate a wide variety of traffic operations, from urban arterials to freeway interchanges to complex toll facilities. The simulation serves two important purposes:

- Provides a visual illustration of the performance of the plaza, providing qualitative feedback concerning the performance of the plaza; and,
- Provides information on queues and delays at the plaza, providing quantitative feedback as well

The following table summarizes the required lane configuration for plaza sizing for each of the nine (9) options that are considered in Section 7 Rehabilitate/Reconstruct Feasibility Analysis. A complete traffic forecast and model was developed for each option including optimizing the way each lane operates. Traffic forecasting and model creation was completed according to the above-described procedure. The exceptions are the No Build and Infrastructure Upgrade scenarios (Options 1 and 2) which both continue to operate with the same number of lanes as they do today. Each option was evaluated and optimized for existing, intermediate and design year conditions, including volumes, ETC usage and heavy vehicle parameters. The operational results of modeling are contained in Table 7 below. Expected queues and vehicle delays for the existing plaza configuration as well as for the various options being considered are listed for comparison.

It is important to understand what these values represent. Traffic queues reported for the existing condition are a result of all cash and E-ZPass customers mixed at a cash plaza that has only slow speed E-ZPass lanes which sometimes become blocked due to long cash lane queues. This queue occupies the approach area and the mainline. Traffic queues reported for open road alternatives are a result of essentially only cash customers in cash only lanes. Cash only lane operation is much more predictable than mixed cash and E-ZPass and so plaza sizing can be set more precisely. Alternatives with cash only lanes have been sized to minimize the number of lanes and resulting impacts, while accepting sometimes longer queues than a mixed cash and E-ZPass alternative. It is also important to note that a queue in a cash only lane will not be allowed to form back into mainline near free flowing traffic.

Table 6 – Toll Plaza Sizing

Opt#	Location	Description	Year	NB Conventional <sup>2</sup>	NB Ramp	NB ORT	Reversible <sup>1</sup>	SB Conventional <sup>2</sup>	SB Ramp	SB ORT	Total Lanes	Total Width (ft) <sup>3</sup>
	Existing		2013	7	0	0	3	7	0	0		
1	Site	No Build (Maintenance Only)	2030	7	0	0	3	7	0	0	17	295
2	Existing	Infrastructure Upgrade Only	2013	7	0	0	3	7	0	0	17	295
	Site	initiastructure opgrade omy	2030	7	0	0	3	7	0	0	17	293
3	Existing	Upgrade w/ Conventional Tolling	2013	6	2	0	2	7	2	0	19	399
	Site	opp. add ii, conventional ronning	2030	6	2	0	2	7	2	0		333
4a	Existing	Upgrade w/ ORT and ramp tolls	2013	5	2	2	0	6	2	2	19	439
	Site		2030	4	2	3	0	5	2	3		
4b	Existing	Upgrade w/ ORT (no ramp tolls)	2013	5	0	2	0	6	0	2	15	335
	Site		2030	4	0	3	0	5	0	3		
6	Existing	Upgrade Existing Site w/ ORT, East Side Mainline Realignment, and	2013	5	0	2	0	6	0	2	15	335
	Site	Relocated Interchange	2030	4	0	3	0	5	0	3		
_	Existing	Relocate Plaza to West w/ ORT,	2013	5	0	2	0	6	0	2		
7	Site	West Side Mainline Realignment, and Relocated Interchange	2030	4	0	3	0	5	0	3	15	335
0	Existing	Relocated Plaza to South w/ ORT	2013	5	2	2	0	6	2	2	40	382
8	Site	and Reconfigured Interchange (with ramp tolls)	2030	4	2	3	0	5	2	3	19	4
9	Existing	Relocated Plaza to South w/ ORT and Relocated Interchange (with	2013	5	2	2	0	6	2	2	10	435
9	Site	ramp tolls)	2030	4	2	3	0	5	2	3	19	435
6, & 7	Same as 6,	Same config. as 6, & 7, except that conventional plaza has been	2013	4 <sup>5</sup>	0	2	0	5	0	2	13	297
(alt)	& 7	reduced by 1 lane in each direction	2030	3 <sup>5</sup>	0	3	0	4	0	3	13	237
8 & 9	Same as 8	Same config. as 8 & 9, except that conventional plaza has been	2013	4 <sup>5</sup>	2	2	0	5	2	2	17	344-
(alt)	& 9	reduced by 1 lane in each direction	2030	3 <sup>5</sup>	2	3	0	4	2	3	1/	397

Reversible lanes are capable of being operated as either northbound or southbound.
 Conventional lane allows cash and slow speed electronic toll

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collection (E-ZPass)

<sup>&</sup>lt;sup>3</sup> Total width is pavement width at center of plaza.

<sup>&</sup>lt;sup>4</sup> Does not include separate 58' wide plaza for NB on ramp

<sup>&</sup>lt;sup>5</sup> The reduction of one conventional lane is achieved by operating 3 tandem lanes

 $Table\ 7-Traffic\ Queue\ and\ Delay\ Summary-30^{th}\ Highest\ Hour$ 

			Opt 1&2		Opt 3		Opt 4a		Opt 4b		Opt 6-7		Opt 6-7 (alt)		Opt 8-9		Opt 8-9 (alt)	
		Existing Site - No-Build / Infrastructure Upgrade		Existing Site Upgrade with Conventional Tolling		Existing Site Upgrade with Highway Speed Tolling & Separate 2-In Ramp Plazas		Existing Site Upgrade with Highway Speed Tolling (no neparate ramp plazas)		Various Locations with ORT		Various Locations with ORT and Reduced Plaza Size		Various Locations with ORT and Ramp Toll Plazas		Various Locations with ORT, Ramp Toll Plazas, and Reduced Plaza Size		
	Year	2013	2030	2013	2030	2013	2030	2013	2030	2013	2030	2013	2030	2013	2030	2013	2030	
'n	NB Queue (ft)																	
ع €	average	221	119	183	145	146	106	171	141	125	127	176	136	109	116	174	119	
High Ny P	max	563	333	373	334	202	144	237	251	187	174	254	217	155	163	244	175	
NB 30th High Hour (Friday PM)	NB Delay (sec)																	
3 30 F	cash	35.3	36.2	35.5	27.9	32.1	21.8	33.4	33.4	29.3	25.9	26.6	15.8	25.2	30.9	26.4	14.0	
ž	E-Zpass	17.5	15.1	12.8	17.9	5.0	8.5	4.3	8.0	3.2	6.3	3.7	8.4	2.8	6.6	3.0	3.5	
'n	SB Queue (ft)																	
₹ €	average	130	189	72	102	208	172	534	471	239	318	350	345	150	163	340	127	
ligh ay P	max	386	457	284	191	293	255	651	671	301	449	417	555	86	248	535	195	
SB 30th High Hour (Sunday PM)	SB Delay (sec)																	
330 (Su	cash	29.7	50.5	27.9	45.0	50.9	45.3	125.0	133.3	57.5	80.0	65.5	94.5	34.9	51.2	60.6	25.3	
SE	E-Zpass	10.3	27.0	12.9	24.2	1.7	4.2	5.7	9.9	2.7	5.9	3.8	7.3	1.8	3.8	3.9	3.4	
	NB Queue (ft)																	
one	average	178	186	167	135	150	120	186	358	125	221	180	370	108	139	184	140	
ا <del>د</del>	max	301	277	362	196	205	185	259	620	181	353	272	662	170	202	265	213	
H E	NB Delay (sec)																	
g Gt	cash	37.3	46.5	34.7	33.0	34.2	27.2	46.4	104.5	31.1	62.5	29.0	73.0	25.7	36.0	28.8	19.9	
d 3	E-Zpass	11.4	11.5	9.6	8.7	3.8	6.4	4.1	8.8	2.5	6.4	2.8	9.0	2.3	5.4	2.7	2.9	
ombined 30th (Saturday AM)	SB Queue (ft)																	
Sat	average	152	131	122	123	163	141	455	325	196	239	275	182	128	137	265	131	
] ö )	max	255	338	253	305	203	179	550	420	268	311	342	283	156	204	347	203	
NB & SB Combined 30th High Hour (Saturday AM)	SB Delay (sec)																	
N N N	cash	36.0	36.4	33.2	42.6	40.3	33.6	112.8	83.8	45.5	64.9	49.5	35.0	27.1	34.1	47.8	20.8	
	E-Zpass	11.1	11.4	8.5	10.3	1.3	2.4	5.3	6.1	2.2	3.7	3.0	3.2	1.3	2.2	2.9	2.2	

The following points of explanation are critical to properly interpreting Table 7:

- While this table provides a comparison of vehicle queues and delays for the various options, it is of utmost importance to understand each option's physical characteristics and the differences between some of the options. As an example, for 2013 NB, Option 4a and 4b have very similar average queues, 146" and 171" respectively and cash delays, 32.1 sec 33.4 sec respectively. However, there physical layout is quite different, Option 4a has 19 lanes and occupies a footprint of 439" allowing for dedicated lanes to serve the York Interchange while Option 4b has 15 lanes and occupies 335" at the plaza and requires the E-ZPass users utilizing the York Interchange to utilize the cash lanes. As any operational comparison is made, the reader should also consider the physical characteristics of the options being compared.
- VISSIM traffic simulations were run for the years 2013 and 2030 to validate traffic operation projections. It is estimated that, at some point between 2025 and 2029, a cash lane in each direction will need to be converted into an Open Road Lane.
- All options are based on a cash processing rate of 320 vehicles per hour.
- Options with Open Road Tolling assume that 3% of E-ZPass patrons will use the conventional lanes and experience delays similar to the cash patrons. The 97% of E-ZPass patrons using the Open Road lanes will experience virtually no delay. The E-ZPass delay in the table presents a weighted average of the two E-ZPass streams of traffic.
- All options should be compared in light of the characteristics highlighted in Table 6. The primary differences between the options include the following:
  - o **Plaza type.** Options 1 through 3 involve conventional toll plazas with reversible lanes. All other options involve ORT facilities with no reversible lanes.
  - o **Ramp tolls.** Options 3, 4a, 8 and 9 each include two 2-lane ramp toll plazas. All other options have no ramp toll plazas.
  - Tandem lanes. Some options involve reducing the overall cross-section by two lanes.
     This is accomplished by operating with 3 tandem lanes in each direction during peak periods.
  - Mix of Cash and ORT lanes. For all ORT scenarios, the mix of cash and ORT lanes changes over time. In order to handle the projected surge in E-ZPass usage over time, one cash lane in each direction will need to be converted to an ORT lane.

The following conclusions may be drawn from the results in Table 7:

- Based on an analysis of traffic conditions during the 30<sup>th</sup> highest hour, all options are feasible. All options maintain a good level of service for E-ZPass patrons, preserve modest delays for cash patrons, and yield minimal queuing. Even Option 4b, which had the highest delays, maintained an average peak-hour queue of less than 500" (or less than 1/10<sup>th</sup> of a mile).
- The existing 17-lane plaza provides adequate peak-hour capacity throughout the study period. Therefore, the motivation for improving the toll plaza is not *primarily* operational.
- The foremost operational benefit of ORT is a significant reduction in delays for E-ZPass patrons. The near free-flow conditions afforded to E-ZPass patrons in an ORT environment represents a significant improvement in their level of service.
- The column labeled Opt8-9 (with a 15-lane cross-section) reflects similar queues and delays as the column labeled Opt8-9(alt) (with a 13-lane cross-section). This suggests that the use of tandem lanes during peak periods is a feasible means of maintaining service levels while reducing the footprint of the plaza. This benefit should be weighed against the safety- and accountability-related disadvantages of tandem lanes, as noted in Section 5.

• Option 4b is perhaps the least-desirable option of all. This option forces E-ZPass patrons traveling to or from Chases Pond Rd. to intermingle with cash patrons that are continuing on the mainline. As a result, the volumes at the "conventional" portion of the plaza in Option 4b are higher than all other options. This yields greater queuing and delays relative to all other options.

In light of the above-noted observations, HNTB draws the following conclusions:

- 1. Open Road Tolling does not necessarily provide an opportunity to reduce the cross-sectional area of the toll facility. However, it offers safety benefits by reducing the number of stops and starts and by separating slow-moving cash traffic from faster-moving E-ZPass traffic, and it significantly improves the level of service for E-ZPass patrons.
- 2. In the opening year, the facility will need to have 2 ORT lanes in each direction.
- 3. In addition to the ORT lanes, the Authority will need to construct conventional lanes to serve cash-paying patrons.
  - a. If the Authority wishes to avoid the use of tandem lanes, then it should construct 5 conventional lanes in the NB direction and 6 conventional lanes in the SB direction.
  - b. If the Authority wishes to minimize the footprint of the plaza, then it should construct 4 conventional lanes in the NB direction and 5 conventional lanes in the SB direction. During peak periods, 3 of the conventional lanes in each direction will need to be operated as tandem lanes.
  - c. As noted earlier, all results in Table 7 are based on traffic conditions during the 30<sup>th</sup> highest hour. Occasionally, actual traffic volumes will exceed this level. During those times, ORT options that do **not** include tandem lanes provide more flexibility to respond. In other words, options which do **not** include tandems could periodically incorporate tandems in order to respond to occasional surges. By contrast, options which already include tandems have little ability to augment their capacity in order to respond to surges which exceed the 30<sup>th</sup> highest hour.
- 4. At some point prior to the end of the design life of the facility, one cash lane in each direction will need to be converted to an ORT lane.
- 5. If the plaza is separated from the interchange, then the Authority can avoid constructing ramp toll plazas. However, if the plaza is constructed in the immediate vicinity of the interchange, then separate ramp toll plazas may be needed to improve operations and enhance safety.
- 6. The analysis has been based on an assumption of fairly modest growth in the share of E-ZPass usage. From 2010 through 2030, it is assumed that the share of E-ZPass usage will grow by about 1.0-1.5% per year, reaching a share of approximately 75% in 2030. If E-ZPass usage grows faster than expected, then the operational forecasts will change as well. In general, greater E-ZPass usage will yield improved performance of the toll facility in any configuration but more so in the ORT configurations.

#### SECTION 7 – REHABILITATE/RECONSTRUCT FEASIBILITY ANALYSIS

The nine options investigated for the York Toll Plaza replacement have been developed based on infrastructure need, tolling strategies, and traffic demand. Mindful of developing a complete range of existing site alternatives, the following options vary from do-nothing or No-Build to a newly constructed plaza with the latest in tolling technology. Following are summaries of the analysis completed for each option, including some preliminary conclusions of each alternative seasibility of meeting the project purpose and need. The goal of this existing site analysis is to identify those options that appear feasible and recommend them to be carried into the next phase of analysis. Further refinement of the recommended option(s) and their respective design will be necessary, however at the conceptual design stage the following considerations are used to compare and contrast the various options:

- safety;
- capacity;
- operational and physical conditions of the plaza;
- adherence to the previously stated basic engineering guidelines;
- property and natural resource impacts, and
- cost.

Presented below is a discussion of each option"s construction elements, the deficiencies and adequacies of design and operations, property and natural resource impacts and costs reported in 2010 dollars. Layout graphics for each of the Options as well as a table that compares the various elements of the options follow the discussion; see Figure 3 to Figure 11 Option 9: Relocate Plaza to South with Open Road Tolling and Relocated Interchange and Table 8 at the end of this section.

Option 1: No-Build (Maintenance Only)

Option 2: Infrastructure Upgrade

Option 3: Upgrade Existing Site with Conventional Tolling and Separate Ramp Lanes
Option 4A: Upgrade Existing Site with Open Road Tolling and Separate Ramp Lanes
Option 4B: Upgrade Existing Site with Open Road Tolling without Separate Ramp Lanes
Option 5: Relocate Plaza to Alternate Location with Open Road Tolling (not part of this

evaluation but a placeholder for consistency with previously developed

documents)

Option 6: Upgrade Existing Site with Open Road Tolling, East Side Mainline

Realignment, and Relocate Interchange

Option 7: Relocate Plaza to West with Open Road Tolling, West Side Mainline

Realignment, and Relocate Interchange

Option 8: Relocate Plaza to South with Open Road Tolling and Reconfigure Interchange Option 9: Relocate Plaza to South with Open Road Tolling and Relocate Interchange

## **Option 1: No-Build (Maintenance Only)**

For baseline and comparison purposes, and as required by environmental permitting agencies, a No-Build option is introduced and discussed. This option would not invest in a full scale replacement of the facility or mainline realignment; instead it consists of renovation of the failing components. As it exists, this plaza is not in conformance with the current engineering practices. According to recent

crash records, this plaza is considered a High Crash Location. Section 5 summarizes this crash data. Deficiencies include the plaza is too close to an interchange, is located on a curve, is too close to an overhead bridge and is at the bottom of a hill. The Chases Pond Road Interchange (Exit 7) is within 1,000 ft of the plaza exacerbating crash potential especially for the Northbound on ramp and Toll Plaza merge area. The Southbound off ramp is also very close to the Plaza and requires unsafe weaving maneuvers to access the ramp. Sight distance criteria are not met for either direction of travel. Due to subsurface conditions, the bumpers that are supposed to protect staff in the toll booths by redirecting errant vehicles are sinking and creating additional safety concern.

The physical infrastructure, booths, tunnel, and canopy are all in urgent need of major rehabilitation. This alternative will only address the most serious of these issues as part of a long-term maintenance or renewal and replacement program. Identified deficiencies not addressed under Option 1 include the sinking roadway, deteriorating and undersized tunnel and proximity to the interchange.

From an operational perspective, there are currently vehicle queue (backup) problems during the busiest periods that would not be addressed by this option. Currently, during these peak periods the two dedicated ETC lanes in each direction have limited access due to inadequate visibility and the vehicle queues that extend back. Once able to maneuver into one of the two dedicated ETC lanes for each direction, patrons are limited to a 10 mph speed limit which slows processing time. Another concern with the ETC lanes is that this moving traffic is typically sandwiched between stop-and-go traffic of the single-direction cash lanes and the reversible cash lanes. This occurs due to the need of operating the three middle lanes as reversible depending on the direction of greatest demand. As the ETC traffic increases, the need for these reversible lanes may decrease allowing for a reassignment of these lanes to dedicated ETC lanes. See Sections 5 and 6 for details on the traffic analysis for this option.

Construction costs associated with this option are defined as the long term maintenance cost less the costs of maintaining a similar new toll plaza. The condition of the existing infrastructure, such as the leaking tunnel, sinking approach slabs and safety bumpers and deteriorating canopy require renewal costs above and beyond that of brand new components. These maintenance costs are categorized by either Annual Maintenance costs or Renewal and Replacement costs.

Annual maintenance costs consist of the following components:

- Toll equipment operation and replacement based on the current tolling structure
- Plaza maintenance based on the current physical layout and condition of the plaza
- Building maintenance based on the current building infrastructure in place at the plaza
- Seasonal tandem toll booth installation and removal

The Turnpike has developed a Renewal and Replacement (R&R) maintenance program for prolonging the life of the plaza another 20 years (2010–2030). It also shows where the Authority could anticipate and plan for the larger expenditures. Major elements of the anticipated R&R maintenance costs consist of the following components:

- Asphalt pavement
  - o Pavement crack sealing
  - Mill and fill overlays to address the settlement of the roadway and accelerated pavement wear and tear due to poor soil subsurface conditions
- Sealing of the concrete slabs and other concrete surfaces
- Canopy painting and roof sealing

- Concrete bumper rehabilitation to maintain integrity and improve safety
- Tunnel rehabilitation

Other elements of toll plaza operation and maintenance such as staffing, guardrail, drainage, and other routine maintenance activities were not evaluated as these elements would be common to all other replacement design options considered to date.

There are no associated property or wetland impacts for this option.

This option, when compared to a purely no-build maintenance only option highlights the deficiencies at the existing site. When simply annualized over the 20 year period of 2010-2030, the Authority could expect to expend an average of \$615,000 on a yearly basis for these extraordinary renewal and replacement activities. Given the condition of infrastructure there would need to be a substantial expenditure in the first few years. A total cost of more than \$12.3 million would be expended above and beyond normal maintenance activities. Additional details can be found in Appendix H.

The No-Build option for the York Toll Plaza does not meet the Maine Turnpike Authority"s objective of: having a southern toll plaza that is overall safe, efficient and economical, that is user-friendly and that implements open road tolling. This option does not address the current physical and safety deficiencies which will grow worse with time. The York Toll Plaza will continue to have capacity and operational issues. A total cost of approximately \$12.3 million for this Option is not prudent.

#### **Option 2: Infrastructure Upgrade**

This option would build a new plaza 200 feet north of the existing toll plaza. The current number of lanes would be built along with maintaining the reversible lane capability. The proportion of cash versus dedicated slow speed ETC lanes would continue to be monitored and adjusted to maintain the best possible efficiency, i.e. as E-ZPass user numbers grow so too will the number of dedicated slow-speed E-ZPass lanes. The infrastructure to be replaced would include: toll booths and bumpers, canopy, tunnel, approach slabs and toll equipment. The upgrade would not include: altering the vertical or horizontal alignment, or improving access to Exit 7 On/Off ramps. The layout of this option can be seen in Figure 4.

Option 2 will continue to prolong the use of a plaza facility that does not meet basic engineering criteria. The plaza is too close to an interchange, is not on a tangent, is not far enough away from the overhead bridge and is not at the crest of a small hill. The Chases Pond Road Interchange (Exit 7) is within 1,000 feet of the existing toll plaza exacerbating two high crash locations due to the merge/weave area between the northbound on ramp and northbound plaza approach, and the merge/weave area from southbound plaza departure to the southbound off ramp. Sight distance design criteria are not met for either travel direction. This option assumes that the upgraded toll plaza would be located approximately 200 feet north of the existing facility. Moving the plaza 200 feet north allows for construction phasing and minimizes interruptions to toll plaza operations however it moves the plaza closer to a hill and further into a curve. Along with moving the plaza north, the approach and departure transition zones will be extended to meet the acceptable transition lengths of today"s guidelines. Replacement of the tunnel and approach slabs would be done with consideration of poor soil conditions and projected settlement. However, the settlement of the roadway beyond the immediate plaza approaches would not be addressed here due to the poor soil conditions extending up to 1,000 feet in each direction.

From an operational perspective, there are currently vehicle queue (backup) problems during the busiest periods that would not be addressed under this option. Currently, during these peak periods, the dedicated ETC lanes have limited access due to inadequate visibility and the vehicle queues that extend back into the three-lane mainline section. Once able to maneuver into one of the two dedicated ETC lanes for each direction, patrons are limited to a 10 mph speed limit which slows processing time. Another concern with the ETC lanes is that this moving traffic is typically sandwiched between stop-and-go traffic of the single-direction cash lanes and the reversible cash lanes. This occurs due to the need of operating the three middle lanes as reversible depending on the direction of greatest demand. As the ETC traffic increases, the need for these reversible lanes may decrease allowing for a reassignment of these lanes to dedicated ETC lanes. See Sections 5 and 6 for details on the traffic analysis for this option.

The Infrastructure Upgrade option does not meet the Maine Turnpike Authority"s objectives of open road tolling, the basic project purpose or the goals for safety, operation or maintenance. Furthermore, this option does not meet the basic engineering criteria. The majority of current infrastructure deficiencies will be addressed but many safety deficiencies will still exist since the basic engineering criteria are not met. The York Toll Plaza will continue to have operational issues that will worsen with time. The layout carries anticipated impacts of 0 home displacements, 1.5 acres of right-of-way, and 11 acres of wetlands and an approximate total cost of \$23 million. A total cost of approximately \$23 million for this Option is not prudent.

#### Option 3: Upgrade Existing Site with Conventional Tolling and Separate Ramp Lanes

This option would upgrade the infrastructure, as noted in Option 2, along with more efficient conventional tolling by separating the interchange ramps with their own toll booths. Several layouts were investigated during the design process altering the horizontal alignment to avoid the existing utility building and separating ramp traffic from mainline traffic. The chosen layout, seen in Figure 5, consists of 19 tolling lanes: six (6) Northbound, seven (7) Southbound, and two (2) reversible mainline toll lanes with two (2) dedicated ramp toll lanes for Exit 7 in each direction for a total of 19 lanes. A number of dedicated ETC lanes would be implemented in each direction on mainline. The proportion of cash versus dedicated slow speed ETC lanes would continue to be monitored and adjusted to maintain the best possible efficiency, as it is done today. This design minimizes the weaving conflicts of ramp and mainline traffic since ramp traffic is physically separated from mainline traffic. This layout assumes that the upgraded toll plaza would be located approximately 200 feet north of the existing facility. Replacement of the tunnel and approach slabs would be done with consideration of poor soil conditions and projected settlement. This layout can be seen in Figure 5.

Option 3 will continue to prolong the use of a facility that does not meet the objective of open road tolling, the basic engineering criteria and does little to address the major safety concerns. The plaza is not on a tangent, is not far enough away from the overhead bridge and is not at the crest of a small hill. While dedicated ramp booths and lanes minimize weaving conflicts by physically separating mainline traffic from ramp traffic at the plaza, the dedicated ramps only shift the decision point a short distance away from the plaza. The result is a plaza that is still too close to an interchange. Dedicated ramp lanes for Exit 7 will require advance signing that must be intermingled with the Cash vs. E-ZPass signing. It will likely be complicated and potentially confusing to the public. Sight distance design criteria are not met for either travel direction.

With this layout, vehicle processing time improves but ETC users are still limited to slow vehicle speeds. This plaza would accommodate the heaviest traffic volumes with minimal queuing. See Sections 5 and 6 for details on the traffic analysis for this option.

The layout carries anticipated impacts of 0 home displacements, 6.3 acres of right-of-way, and 17.6 acres of wetlands and an approximate total cost of \$40.9 million. This Option does not meet the Maine Turnpike Authority"s objective, the basic project purpose or all the goals for safety, operation and maintenance, including the implementation of open road tolling. Although traffic capacity will be improved, the total project cost of approximately \$40.9 million for this Option is not prudent.

## Option 4A: Upgrade Existing Site with Open Road Tolling and Separate Ramp Lanes

This option would upgrade the existing facility with open road tolling. Layouts investigated during the design process included altering the horizontal alignment to avoid the existing Administration Building, reconfiguring the Exit 7 Interchange, and separating ramp traffic from mainline traffic. The final layout accepted impacts to the Administration Building in exchange for an improved horizontal alignment and minimized environmental impacts. Given the continued increase in electronic toll collection, the decrease in cash toll collection and the fluctuation in overall traffic growth, two separate plaza layouts were developed to process this mix of traffic as efficiently as possible. For the opening year, layout consists of five NB and six SB cash toll lanes, two open road toll lanes in each direction and two dedicated ramp toll lanes in each direction. Growth in E-ZPass usage, and corresponding decline in cash tolls, will dictate that by 2019 one cash lane in each direction can be converted to an open road toll lane to maintain efficient use of both lane types and to minimize overall plaza sizing. The attached graphic shows the future layout, i.e. three (3) open road toll lanes in each direction, four (4) NB and five (5) SB cash toll lanes, and two (2) dedicated ramp toll lanes in each direction. Dedicated ramp booths were introduced to separate interchange traffic from toll traffic. This layout assumes that the upgraded toll plaza would be located approximately 200 ft north of the existing facility. This option assumes the replacement of the tunnel to facilitate safe access for the tolling staff. Replacement of the tunnel and approach slabs would be done with consideration of projected settlement and poor soil conditions. This layout can be seen in Figure 6.

Option 4A will continue to prolong the use of a facility that does not meet the full benefits of open road tolling, the basic engineering criteria and does little to address the major safety concerns. The plaza is not on a tangent, is not far enough away from the overhead bridge and is not at the crest of a small hill. While dedicated ramp booths and lanes minimize weaving conflicts by physically separating mainline traffic from ramp traffic at the plaza, the dedicated ramps only shift the decision point a short distance away from the plaza. The result is a plaza that only marginally meets the proximity to an interchange. Dedicated ramp lanes for Exit 7 will require advance signing that must be intermingled with the Cash vs. E-ZPass signing. It will likely be complicated and potentially confusing to the public. Sight distance design criteria are not met for either travel direction.

With this layout, vehicle processing time improves upon opening due to the physical separation of ETC and cash patrons, and will continue to improve as ETC usage increases. However, the geometrics of the mainline and ORT lanes and proximity to interchange will likely require lower mainline speed. Therefore, ETC patrons will not fully benefit from the implementation of open road tolling. This plaza would accommodate the heaviest traffic volumes with some queuing for cash patrons. Toll plaza personnel will benefit from interacting only with stop and go cash traffic and not with intermittent free

flowing ETC traffic; resulting in improved safety at the toll plaza area. See Table 7 for details on the traffic analysis for this option.

This option carries anticipated impacts of 0 home displacements, 8.1 acres of right-of-way, and 28 acres of wetlands and an approximate total cost of \$56.3 million. Although this option does not address all of the safety and geometric deficiencies, and does not realize the full benefit of open road tolling, this option does partially meet one of the more critical design criteria and has comparatively fewer impacts than other existing site alternatives.

## Option 4B: Upgrade Existing Site with Open Road Tolling without Separate Ramp Lanes

This option would upgrade the existing facility with open road tolling. The layout for this option is essentially the same as Option 4A but does not have the dedicated ramp toll lanes. Reiterating from Option 4A, the final layout accepted impacts to the Administration Building in exchange for an improved horizontal alignment and minimized environmental impacts. Given the continued increase in electronic toll collection, the decrease in cash toll collection and the fluctuation in overall traffic growth, two separate plaza layouts were developed to process this mix of traffic as efficiently as possible. For the opening year, layout consists of five NB and six SB cash toll lanes and two open road toll lane in each direction without the use of dedicated ramp toll booths. Growth in E-ZPass usage, and corresponding decline in cash tolls, will dictate that by 2019 one cash lane in each direction can be converted to an open road toll lane to maintain efficient use of both lane types and to minimize overall plaza sizing. The attached graphic shows the future layout, i.e. three (3) open road toll lanes in each direction, four (4) NB and five (5) SB cash toll lanes. This layout assumes the upgraded toll plaza would be located approximately 200 ft north of the existing facility. This option includes the replacement of the tunnel to facilitate safe access for the tolling staff. Replacement of the tunnel and approach slabs would be done with consideration of projected settlement and poor soil conditions. This layout can be seen in Figure 7.

Option 4B will continue to prolong the use of a facility location that will not allow the MTA to meet basic engineering criteria and will not realize the full benefits of open road tolling. This layout will create a confusing traffic pattern by requiring all southbound Exit 7 traffic, cash and E-ZPass patrons, to travel through the cash only lanes. This results in a continued vehicle weave condition south of the plaza. For northbound patrons, Exit 7 on-ramp traffic will also continue with a weave situation approaching the plaza as E-ZPass patrons shift left and heavy trucks shift right to utilize the truck climbing lane following the plaza. Both of these confluence points have been recognized as High Crash Locations and this Option will not remove the root cause of this designation. This option provides a separation of slow or stopped cash patrons from open road patrons through the use of a physical barrier. Minimizing right-of-way and wetland impacts dictates this barrier be a minimum length which coincides with the deceleration length required for the cash lanes. The result for southbound traffic is 1) the end of this barrier and corresponding lane change does not become visible to the approaching driver until approximately 1650 feet away, only 200 feet more than the minimum required, 2) the barrier is on the inside of a curve requiring cash and Exit 7 traffic to steer across its location further to the inside of curve, and 3) is situated such that approach signing for Cash tolls and Exit 7 off ramp traffic must occupy the same space, creating multiple decisions to be made at the same time. For northbound traffic 1) the end of barrier and corresponding lane change will not be visible to the approaching driver until 1800 feet away, only 350 feet more than the minimum required, and 2) it requires traffic signage to be in very close proximity to Exit 7 off ramp signing. The combination of horizontal geometry, vertical geometry and complex signing make this layout a safety concern. In

addition, the plaza is still too close to an interchange, is not on a tangent, is not far enough away from the overhead bridge and is not at the crest of a small hill. Sight distance design criteria are not met for either travel direction.

With this option, vehicle processing time improves at opening due to the separation of ETC and cash patrons, and will continue to improve as ETC usage increases. However, the geometrics of the mainline and ORT lanes and proximity to interchange will likely require lower mainline speed and therefore ETC patrons will not fully benefit from the implementation of open road tolling. Also, in the future year this option requires the use of tandem cash toll lanes during peak hour flow. This option would accommodate the heaviest traffic volumes with some queuing for cash patrons. Toll plaza personnel will not see the same benefit as in Option 4A from complete separation of Exit and mainline traffic, i.e. there will be E-ZPass patrons within the Exit 7 ramp traffic that will be required to utilize the cash lanes. See Table 7 for details on the traffic analysis for these options.

This option carries anticipated impacts of 0 home displacements, 3.3 acres of right-of-way, and 22.2 acres of wetlands with an approximate cost of \$43 million. This option does not address the safety and geometric deficiencies; in fact it potentially increases safety concerns, and does not realize the full benefit of open road tolling. This option does have comparatively fewer impacts than other existing site alternatives.

## Option 5: Relocate Plaza to Alternate Location with Open Road Tolling

Investigation of alternative locations was suspended in order to focus the comprehensive evaluation on the existing toll plaza area. Option 5 is being listed here only to maintain numerical consistency with previously developed documents.

# Option 6: Upgrade Existing Site with Open Road Tolling, East Side Mainline Realignment, and Relocate Interchange

Option 6 was developed as one possibility to answer the question, "What would it take to replace the plaza in York?" While this option was thought to be, and ultimately deemed to be, impractical, it was researched and is being offered as part of a fully comprehensive response to the York Selectman. This option proposes upgrading the existing plaza with open road tolling and an eastern realignment of the mainline between the Turnpike and Route 1. The Exit 7 interchange at Chases Pond Road will be replaced with an interchange just south at Route 91. Local roadway work will include: 1) upgrading Route 91/Cider Hill Road between the Route 1 and Bog Road intersections, 2) rerouting a portion of Chases Pond Road north of the Turnpike to intersect Bog Road and 3) realigning Bog Road to accommodate the SB off ramp. Structural work will include the removal of the Chases Pond Road Bridge and lengthening of the Route 91 Bridge/Cider Hill Road Bridge. Given the continued increase in electronic toll collection, the decrease in cash toll collection and the fluctuation in overall traffic growth, two separate plaza layouts were developed to process this mix of traffic as efficiently as possible. For the opening year, layout was developed with five NB and six SB cash toll lanes and two open road lanes in each direction. Growth in E-ZPass usage, and corresponding decline in cash tolls, will dictate that by 2019 one cash lane in each direction can be converted to an open road toll lane to maintain efficient use of both lane types and to minimize overall plaza sizing. The attached graphic shows the future layout, i.e. three (3) open road toll lanes in each direction, four (4) NB and five (5) SB cash toll lanes. This can be seen in Figure 8 Option 6: Upgrade Existing Site with Open Road Tolling, East Side Mainline Realignment, and Relocate Interchange

This design generally meets basic engineering criteria identified in Section 3. The Turnpike is realigned so that the plaza is on a tangent segment of highway. The separation of the plaza and the interchange falls short of the 1 mile criteria by approximately 1,000 feet and is therefore categorized as marginally meeting standard. The advance signing for the new Route 91 Interchange, in concert with signing for open road tolling that must be incorporated with the toll plaza signing, will likely be complicated and potentially confusing for the public. The third criterion, proper separation from a bridge so sight distance is not jeopardized, is satisfied. Adjusting the profile to create a high point will satisfy the fourth criterion. The horizontal alignment north of the plaza contains s-curves that are one degree (5750" radius) so that the alignment can get back on track with the mainline. Though this alignment technically meet design standards, potential safety issues are likely to occur with high speed traffic making the s-curve maneuver. The soils at this location are poor and are likely to add to the overall cost and complexity of this option.

With this layout, vehicle processing time improves with the incorporation of open road lanes and will continue to operate well as ETC usage increases. This plaza would accommodate the heaviest traffic volumes with minimal queuing for cash patrons. Toll plaza personnel will benefit from interacting only with stop and go cash traffic and not with intermittent free flowing ETC traffic; resulting in improved safety at the toll plaza area. See Table 7 for details on the traffic analysis for this option.

This option marginally meets the basic design criteria; however falls short of the overall project purpose, in that it is not an environmentally conscious solution and is not cost effective. This option carries anticipated impacts of 89 home displacements, 202 acres of right-of-way, and 57 acres of wetlands and an approximate total cost of \$155 million. Given the community and environmental impacts alone makes this Option not prudent; cost adds yet another reason to dismiss this option.

## Option 7: Relocate Plaza to West with Open Road Tolling, West Side Mainline Realignment, and Relocate Interchange

Option 7 was developed as one possibility to answer the question, "What would it take to replace the plaza in York?" While this option was thought to be, and ultimately deemed to be, impractical, it was researched and is being offered as part of a fully comprehensive response to the York Selectman. This option proposes upgrading the existing plaza with open road tolling and a realignment of the mainline to the west between the Turnpike and Chases Pond Road. The Exit 7 interchange at Chases Pond Road will be replaced with an interchange to the south at Route 91. Local roadway work will include: 1) upgrading Route 91/Cider Hill Road between the Route 1 and Bog Road intersections, 2) rerouting a portion of Chases Pond Road north of the Turnpike to intersect Bog Road and 3) realigning Bog Road to accommodate the SB off ramp. Structural work will include the removal of the Chases Pond Road Bridge and lengthening of the Route 91/Cider Hill Road Bridge. Given the continued increase in electronic toll collection, the decrease in cash toll collection and the fluctuation in overall traffic growth, two separate plaza layouts were developed to process this mix of traffic as efficiently as possible. For the opening year, layout was developed with five NB and six SB cash toll lanes and two open road lanes in each direction. Growth in E-ZPass usage, and corresponding decline in cash tolls, will dictate that by 2019 one cash lane in each direction can be converted to an open road toll lane to maintain efficient use of both lane types and to minimize overall plaza sizing. The attached graphic shows the future layout, i.e. three (3) open road toll lanes in each direction, four (4) NB and five (5) SB cash toll lanes. This can be seen in Figure 9 Option 7: Relocate Plaza to West with Open Road Tolling, West Side Mainline Realignment, and Relocate Interchange

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This design generally meets basic engineering criteria identified in Section 3. The Turnpike is realigned so that the plaza is on a tangent segment of highway. The plaza and the Exit 7 interchange meet the one mile separation criteria. The advance signing for the new Route 91 Interchange, in concert with signing for open road tolling that must be incorporated with the toll plaza signing, will likely be complicated and potentially confusing the public. The third criterion, proper separation from a bridge so sight distance is not jeopardized, is satisfied. Adjusting the profile to create a high point will satisfy the fourth criterion.

With this layout, vehicle processing time improves with the incorporation of open road lanes and will continue to operate well as ETC usage increases. This plaza would accommodate the heaviest traffic volumes with minimal queuing for ETC patrons. Toll plaza personnel will benefit from interacting only with stop and go cash traffic and not with intermittent free flowing ETC traffic; resulting in improved safety at the toll plaza area. See Section 8 for details on the traffic analysis for this option.

The existing site is surrounded by wetlands with approximately 61 acres of wetland to be impacted. Mitigation costs for these impacts are approximately \$24.6 million assuming a 4:1 replacement ratio. The relocation of the Chases Pond Road interchange and the realignment of the Turnpike to the west would potentially displace 22 homes/buildings and an additional 106 acres of right-of way would be acquired.

This option essentially meets the basic design criteria; however, it falls short on the overall project purpose in that it does not offer a cost effective and environmentally conscious solution. This option, carrying anticipated impacts of up to 21 home displacements, 106 acres of right-of-way, and 62 acres of wetlands and an approximate total cost of \$106 million, is simply not prudent.

## Option 8: Relocate Plaza to South with Open Road Tolling and Reconfigure Interchange

Option 8 was developed as one possibility to answer the question, "What would it take to replace the plaza in York?" While this option was thought to be, and ultimately deemed to be, impractical, it was researched and is being offered as part of a fully comprehensive response to the York Selectman. Furthermore, Option 8 will likely require U.S. Congressional action before proceeding into any formal design due to the fact that the Maine Turnpike Authority does not have jurisdiction to toll the Interstate south of the existing plaza. However, for purposes of discussing all possibilities this option is detailed here. Option 8 would locate the plaza underneath a new Chases Pond Road Bridge with a combination of open road tolling and conventional cash tolls. To address the NB weigh station located south of Cider Hill Road and achieve the required one mile separation from an interchange, a collector distributor road for NB traffic is developed to separate the weigh station along with the exiting ramp traffic from the mainline traffic. The collector – distributor road allows traffic to exit onto Chases Pond Road or continue to the toll plaza to go thru the cash toll lanes and merge with the mainline north of the toll plaza. Separate ramp toll plazas, each with 2 cash lanes, will be constructed for NB traffic entering the Turnpike and SB traffic exiting the Turnpike. The Exit 7 SB on ramps will be reconstructed and extended to meet appropriate spacing with the merging cash and open road tolling lanes. Local road work would be approximately 800" of realigning Chases Pond Road. Structural work would include reconstructing both Route 91/Cider Hill Road and Chases Pond Road bridges with longer spans. Given the continued increase in electronic toll collection, the decrease in cash toll collection and the fluctuation in overall traffic growth, two separate plaza layouts were developed to

process this mix of traffic as efficiently as possible. For the opening year, layout was developed with five NB and six SB cash toll lanes, two open road lanes in each direction, and two dedicated ramp toll lanes in each direction. Growth in E-ZPass usage, and corresponding decline in cash tolls, will dictate that by 2019 one cash lane in each direction can be converted to an open road toll lane to maintain efficient use of both lane types and to minimize overall plaza sizing. The attached graphic shows the future layout, i.e. three (3) open road toll lanes in each direction, four (4) NB and five (5) SB cash toll lanes, and two (2) dedicated ramp toll lanes in each direction. This can be seen in Figure 10 Option 8: Relocate Plaza to South with Open Road Tolling and Reconfigure Interchange

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This design generally meets the four basic engineering criteria identified in Section 3. A one mile separation of the interchange ramps and the toll plaza is met along with standard spacing for merging and diverging traffic streams being satisfied due to longer than normal on/off ramps. However, with the interchange bridge at the plaza, traffic has to make a decision to exit the Turnpike a mile or more before the Chases Pond Road Interchange which is sooner than expected. The advance signing for the Exit 7 Interchange and dedicated ramp lanes, in concert with signing to direct open road and cash tolling traffic, will likely be complicated and potentially confusing to the public. Other basic design criteria of locating a plaza on a tangent and a high point will be met marginally. A horizontal curve begins on the mainline approximately 1,000 feet north of the plaza, however adequate sight distance is available, and a high point generated from a profile adjustment will be local considering the proximity to the existing hill north of Chases Pond Road. The fourth criterion of separation from a bridge is met.

With this layout, vehicle processing time improves with the incorporation of open road lanes and will continue to operate well as ETC usage increases. This plaza would accommodate the heaviest traffic volumes with minimal queuing for cash patrons. Toll plaza personnel will benefit from interacting only with stop and go cash traffic and not with intermittent free flowing ETC traffic; resulting in improved safety at the toll plaza area. See Table 7 for details on the traffic analysis for this option.

This option essentially meets the basic design criteria; however falls short on the overall project purpose in that it does not offer a cost effective and environmentally conscious solution. This option carrying anticipated impacts of up to 7 home displacements, 17.7 acres of right-of-way and 52 acres of wetlands and an approximate total cost of \$118 million, while not completely addressing the safety and geometric deficiencies, is simply not prudent.

### Option 9: Relocate Plaza to South with Open Road Tolling and Relocate Interchange

Option 9 was developed as one possibility to answer the question, "What would it take to replace the plaza in York?" While this option was thought to be, and ultimately deemed to be, impractical, it was researched and is being offered as part of a fully comprehensive response to the York Selectman. Furthermore, Option 9 will likely require U.S. Congressional action before proceeding into any formal design due to the fact that the Maine Turnpike Authority does not have jurisdiction to toll the Interstate south of the existing plaza. However, for purposes of discussing all possibilities this option is detailed here. Option 9 would locate the plaza directly below a new Chases Pond Road Bridge with a combination of open road tolling and conventional cash tolls. The Exit 7 interchange at Chases Pond Road will be replaced with an interchange to the south at Route 91. A collector – distributor road for NB approaching traffic will separate NB weigh station and NB exiting and entering ramp traffic from the mainline traffic. NB entering traffic and weigh station traffic will be required to go thru dedicated ramp cash toll lanes that are separated from the main plaza. After the plaza, all NB traffic passing

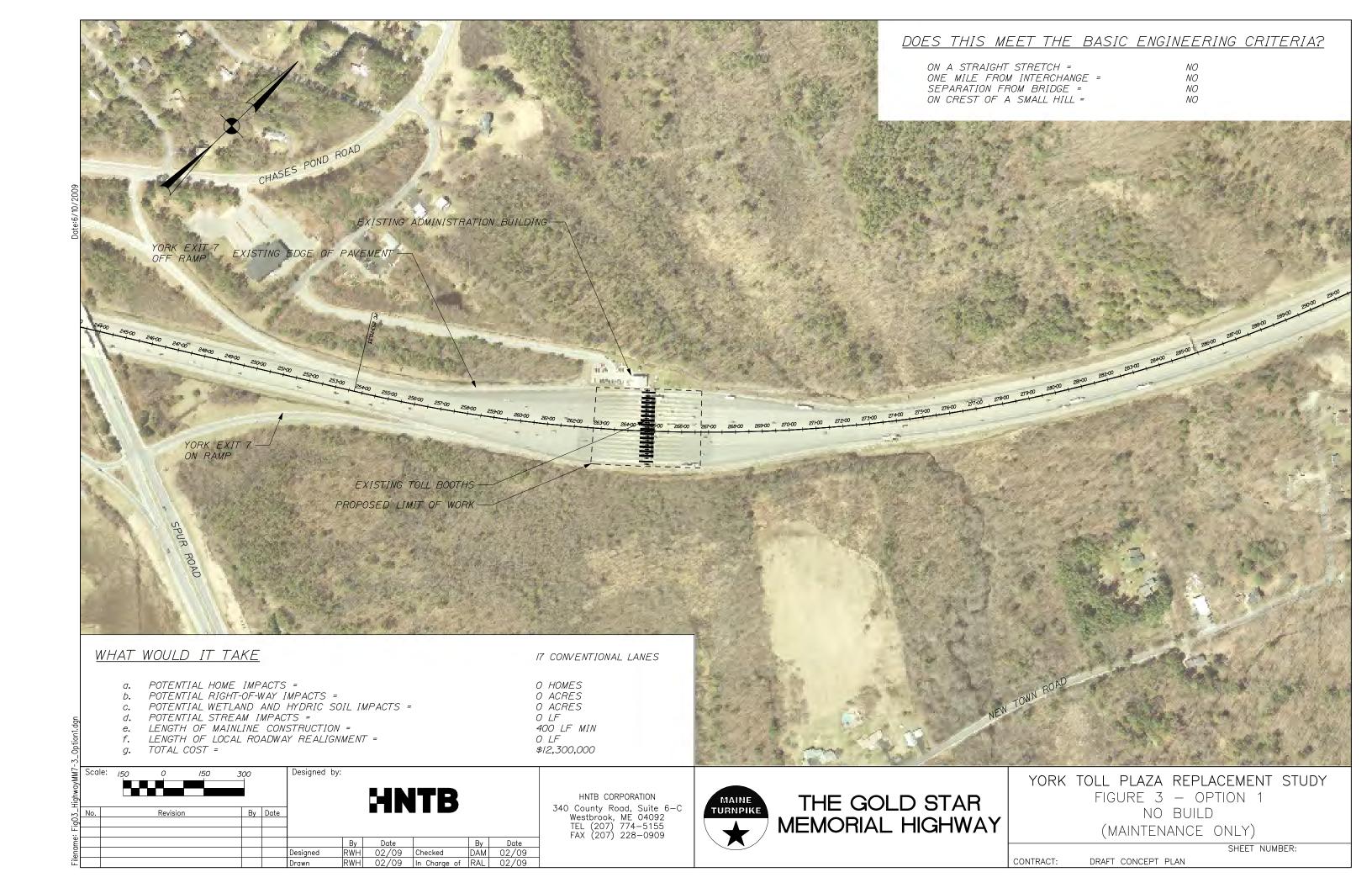
through the cash lanes will merge prior to merging with the ORT mainline traffic. SB motorists destined for Route 91 will exit prior to the exiting cash traffic and proceed through a longer than normal ramp and cash toll lanes that are separated from the main plaza. This traffic will then continue to Route 91. Local roadway work will include: 1) upgrading Route 91/Cider Hill Road between the Route 1 and Bog Road intersections, and 2) realigning Bog Road to accommodate the SB off ramp. Structural work would include reconstructing both Route 91/Cider Hill Road and Chases Pond Road bridges with longer spans. Given the continued increase in electronic toll collection, the decrease in cash toll collection and the fluctuation in overall traffic growth, two separate plaza layouts were developed to process this mix of traffic as efficiently as possible. For the opening year, layout was developed with five NB and six SB cash toll lanes, two open road lanes in each direction, and two dedicated ramp toll lanes in each direction. Growth in E-ZPass usage, and corresponding decline in cash tolls, will dictate that by 2019 one cash lane in each direction can be converted to an open road toll lane to maintain efficient use of both lane types and to minimize overall plaza sizing. The attached graphic shows the future layout, i.e. three (3) open road toll lanes in each direction, four (4) NB and five (5) SB cash toll lanes and two (2) dedicated ramp toll lanes in each direction. This can be seen in Figure 11 Option 9: Relocate Plaza to South with Open Road Tolling and Relocated Interchange

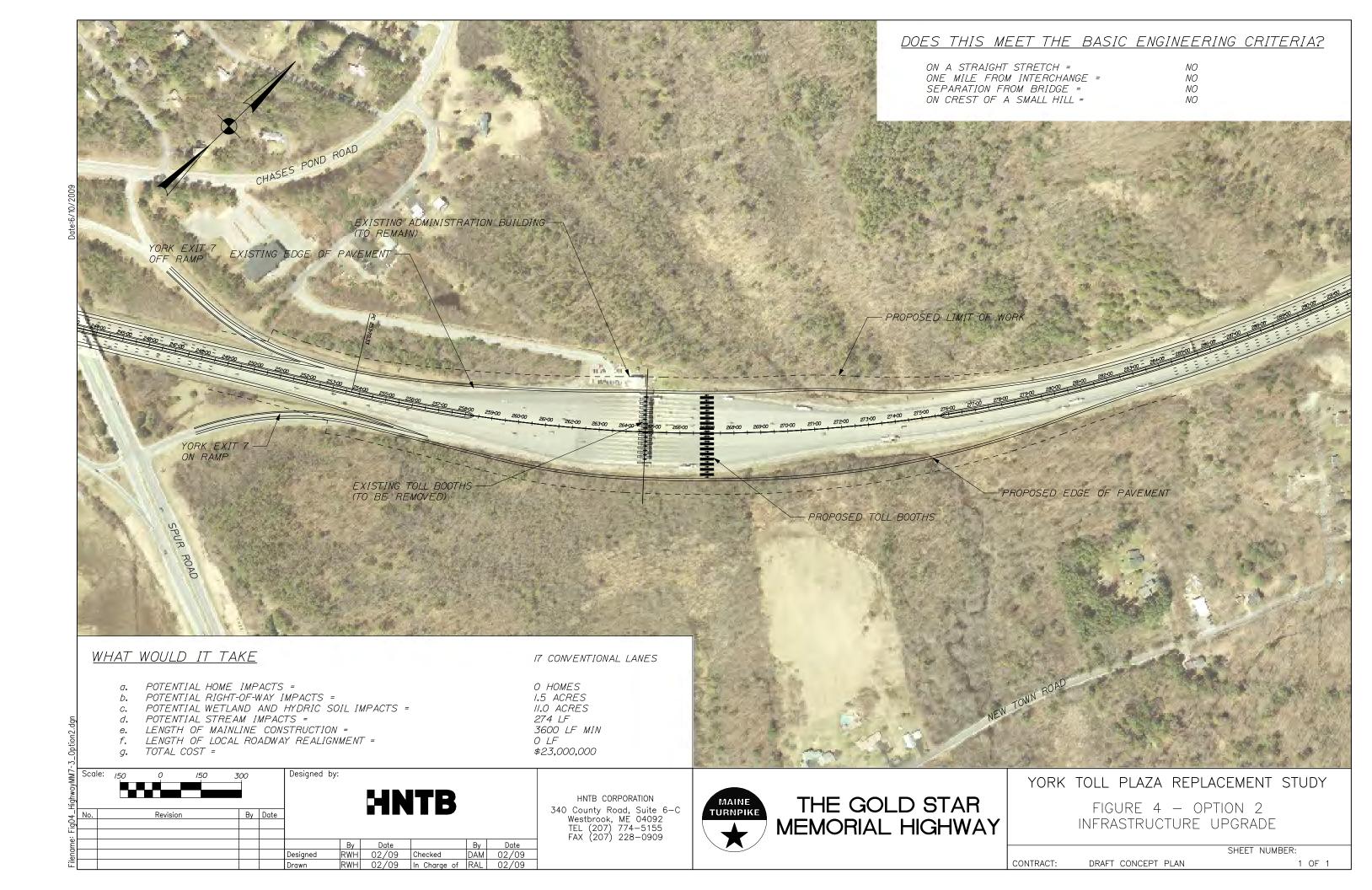
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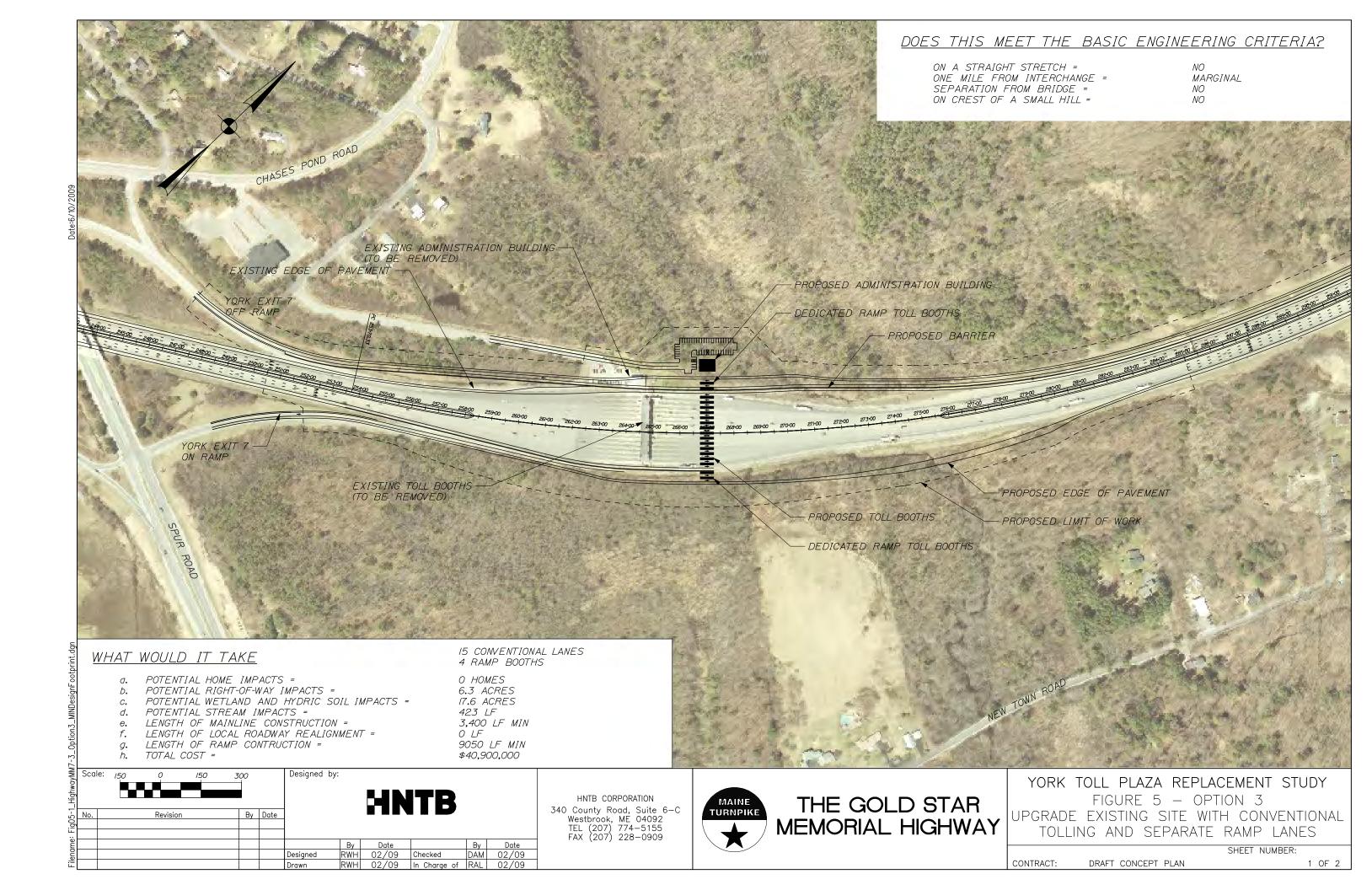
This design generally meets the four basic engineering criteria identified in Section 3. A one mile separation of the interchange ramps and the toll plaza along with standard spacing for merging and diverging traffic streams is satisfied. However, with the interchange bridge at the plaza, traffic has to make a decision to exit the Turnpike a mile or more before Chases Pond Road which could be sooner than expected. The advance signing for the Exit 7 Interchange and dedicated ramp lanes, in concert with signing for open road and cash tolling, will likely be complicated and potentially confusing to the public. Other basic design criteria, locating a plaza on a tangent segment of highway and on a high point, will be met marginally. A horizontal curve begins on the mainline approximately 1,000 feet north of the plaza, however adequate sight distance is available, and a high point generated from a profile adjustment will be local considering the proximity to the existing hill north of Chases Pond Road. The fourth criterion of separation from a bridge is met.

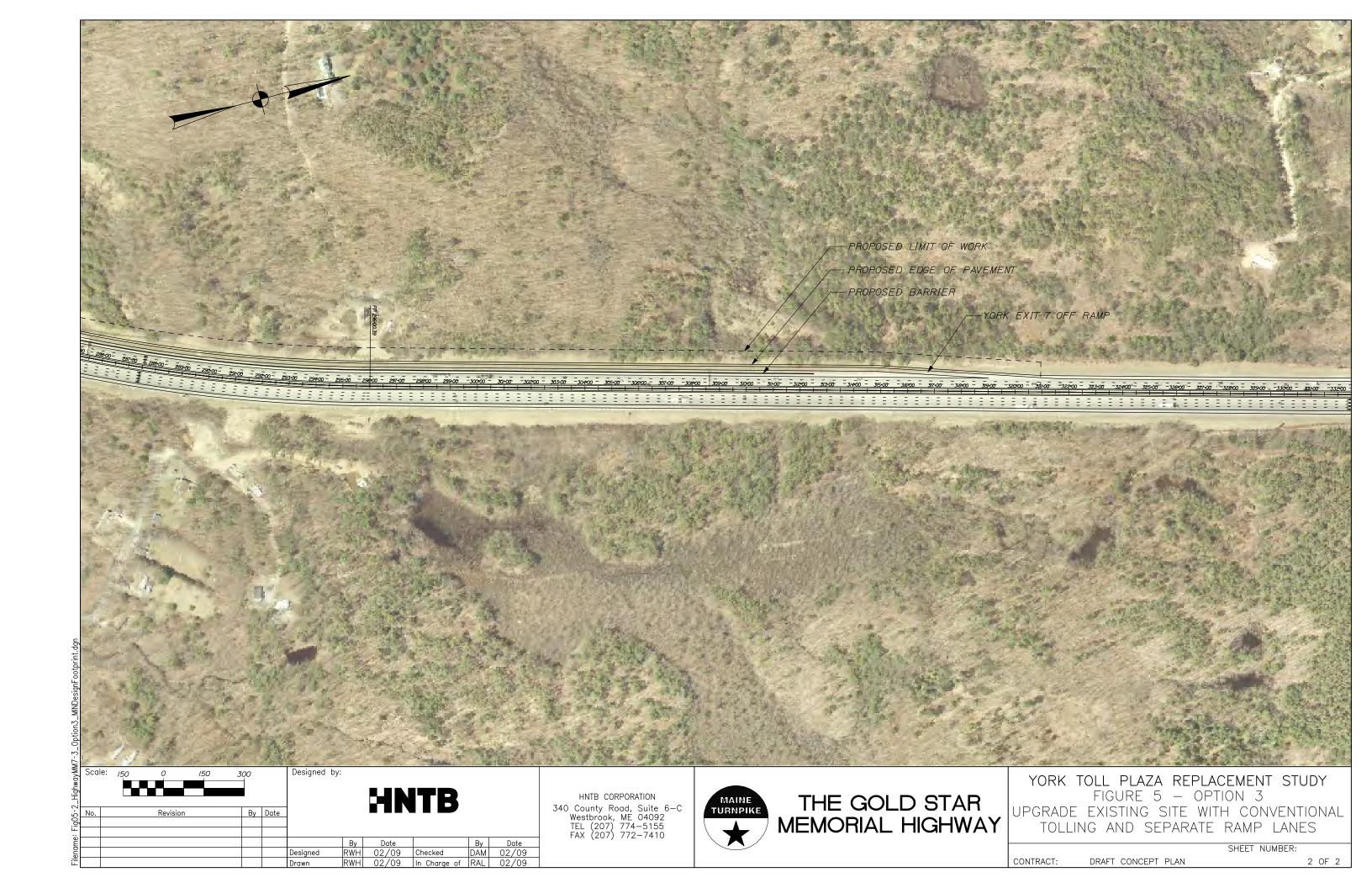
With this layout, vehicle processing time improves with the incorporation of open road lanes and will continue to operate well as ETC usage increases. This plaza would accommodate the heaviest traffic volumes with minimal queuing for cash patrons. Toll plaza personnel will benefit from interacting only with stop and go cash traffic and not with intermittent free flowing ETC traffic; resulting in improved safety at the toll plaza area. See Table 7 for details on the traffic analysis for this option.

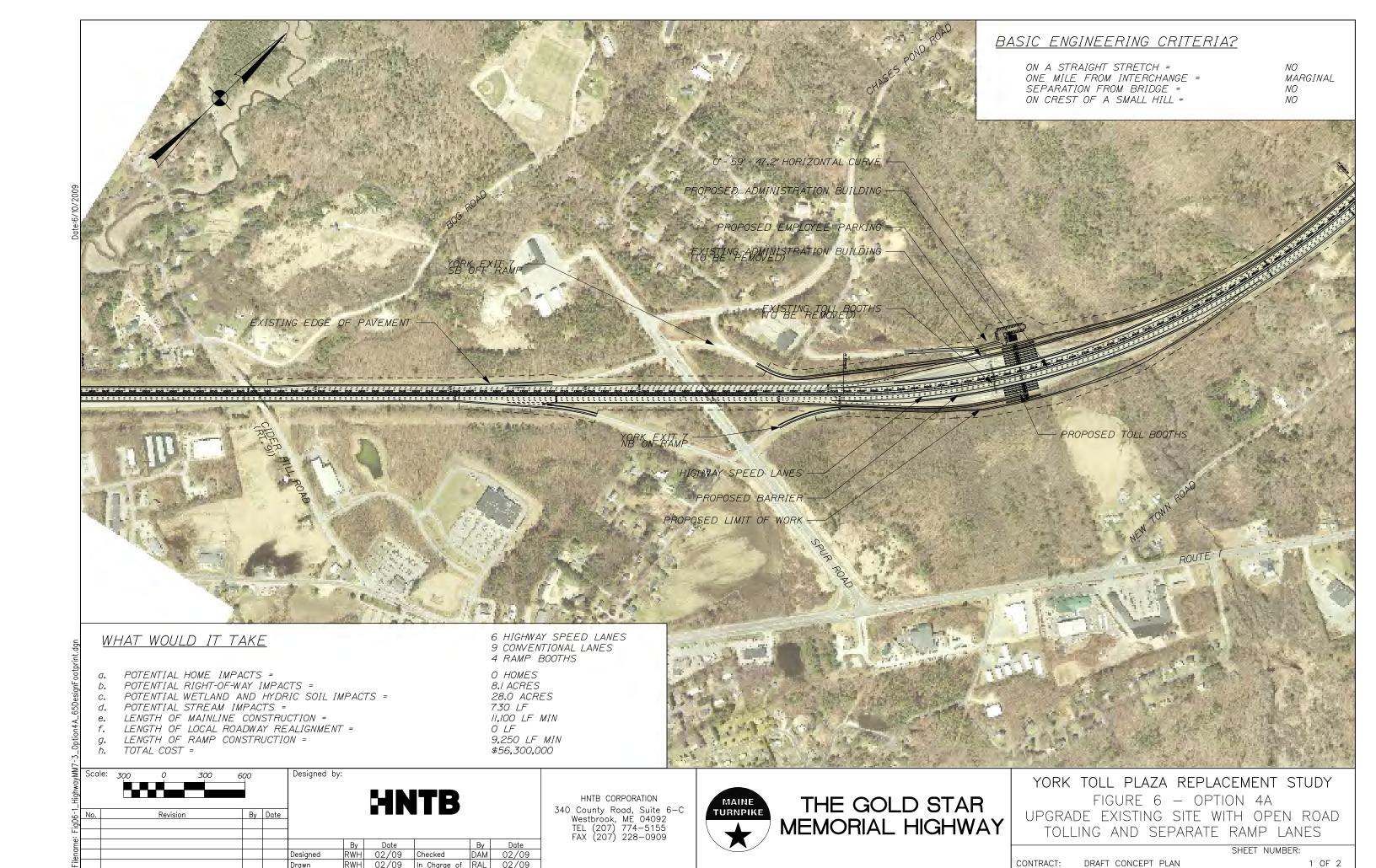
This option essentially meets the basic design criteria; however falls short on the overall project purpose, which is to find a cost effective and environmentally conscious solution. This option, carrying anticipated impacts of up to 7 home displacements, 19.7 acres of right-of-way, and 43.7 acres of wetlands and an approximate total cost of \$94.5 million, while not completely addressing the safety and geometric deficiencies, is simply not prudent.







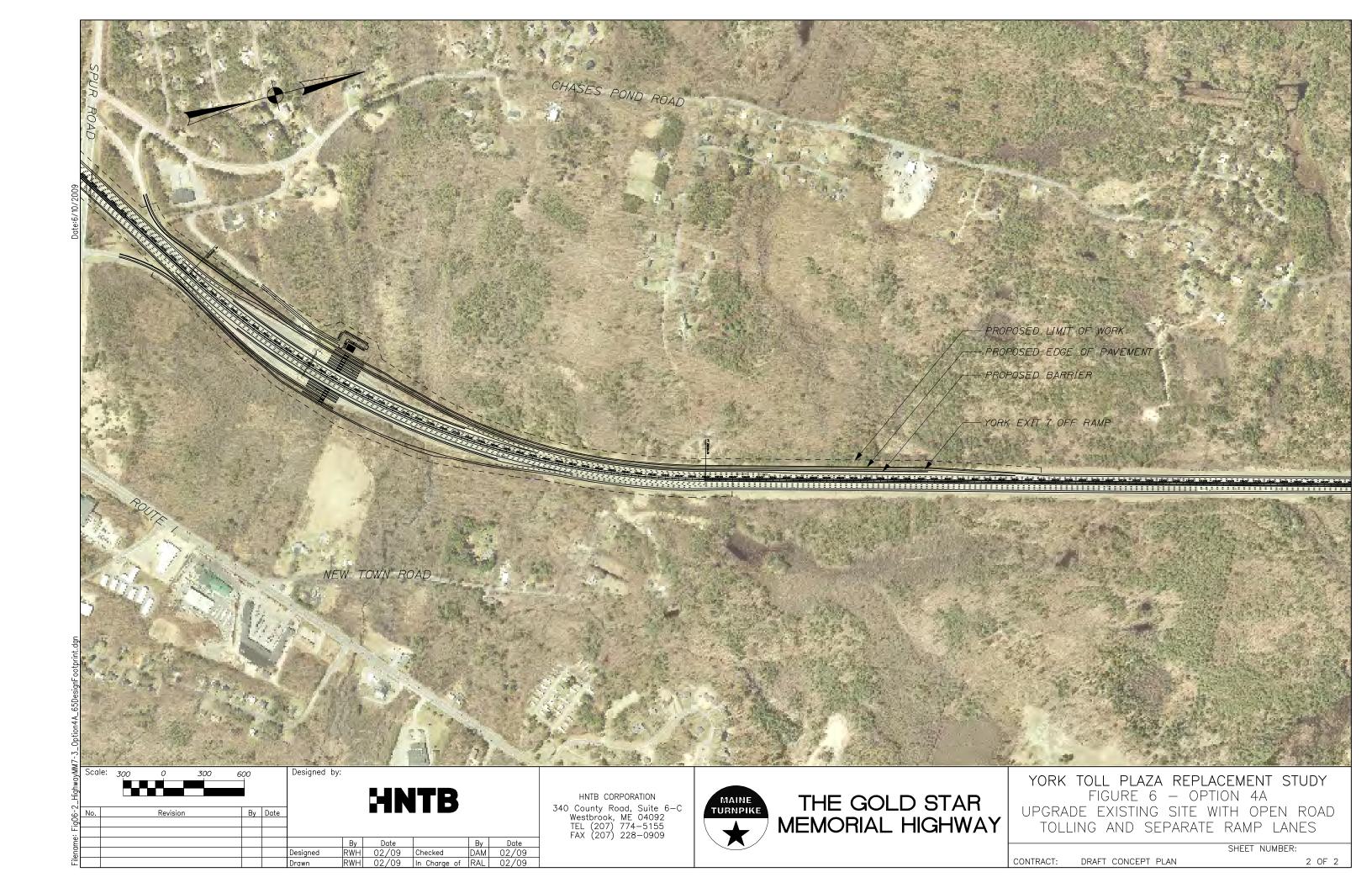


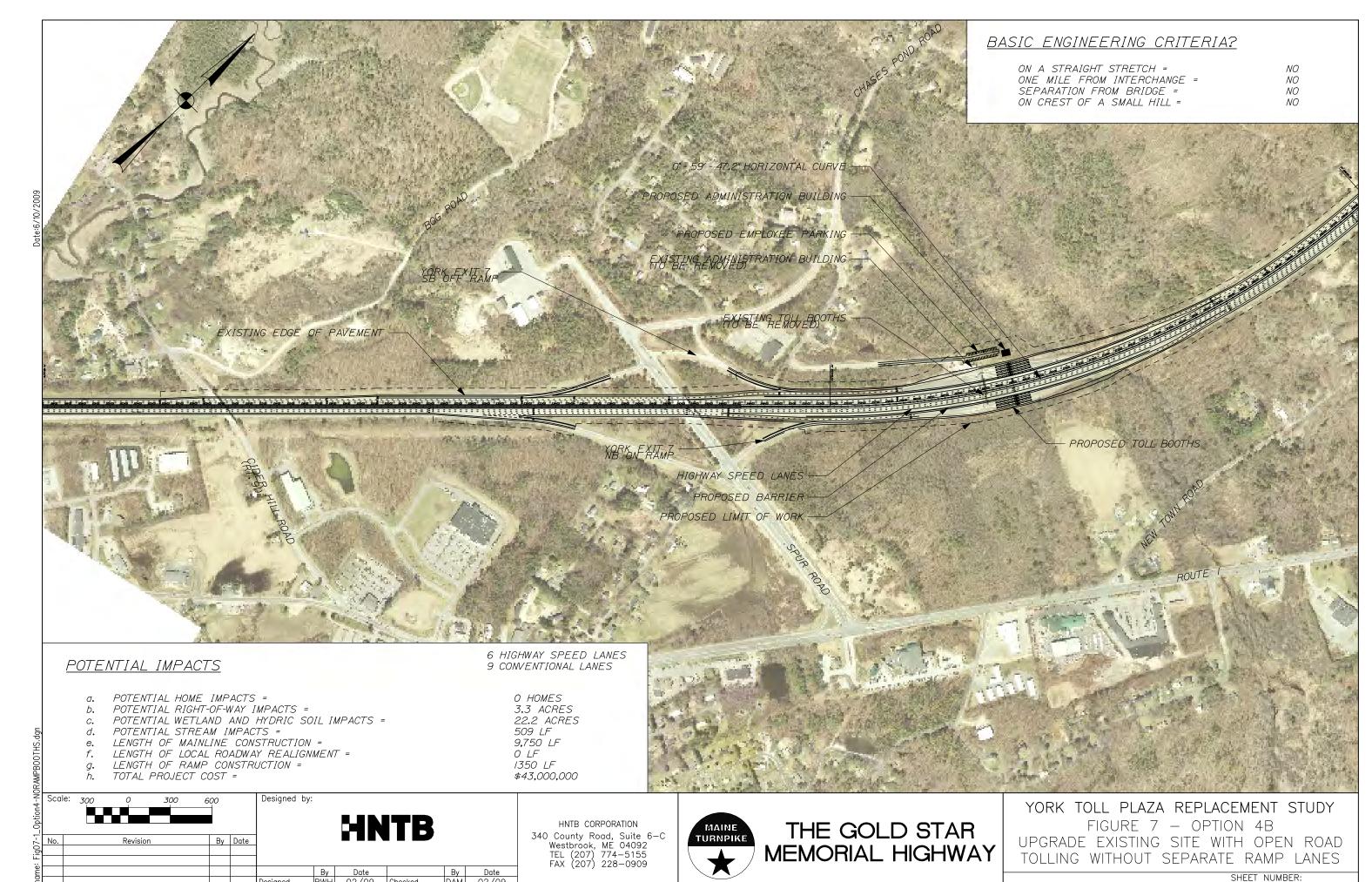


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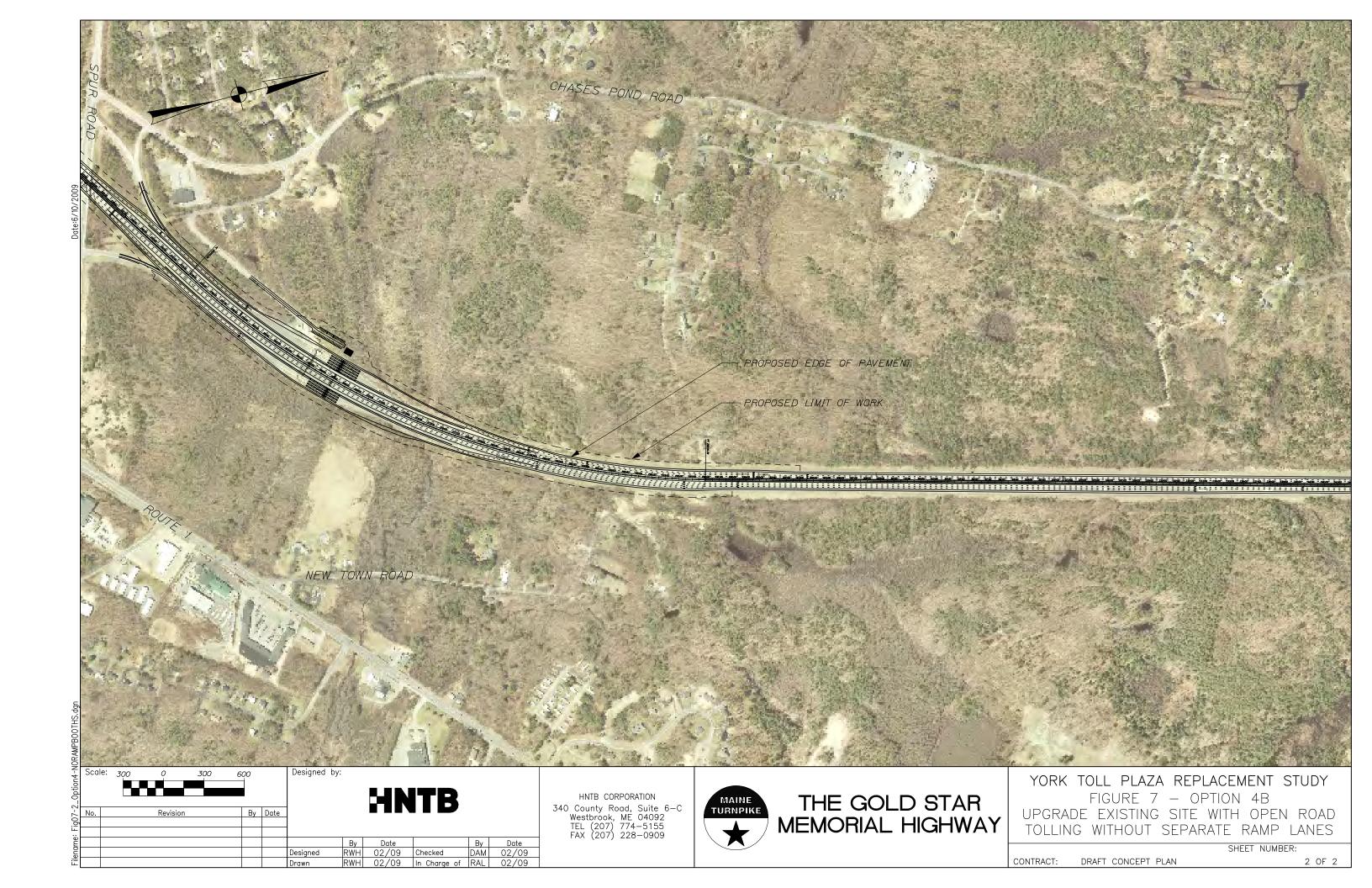


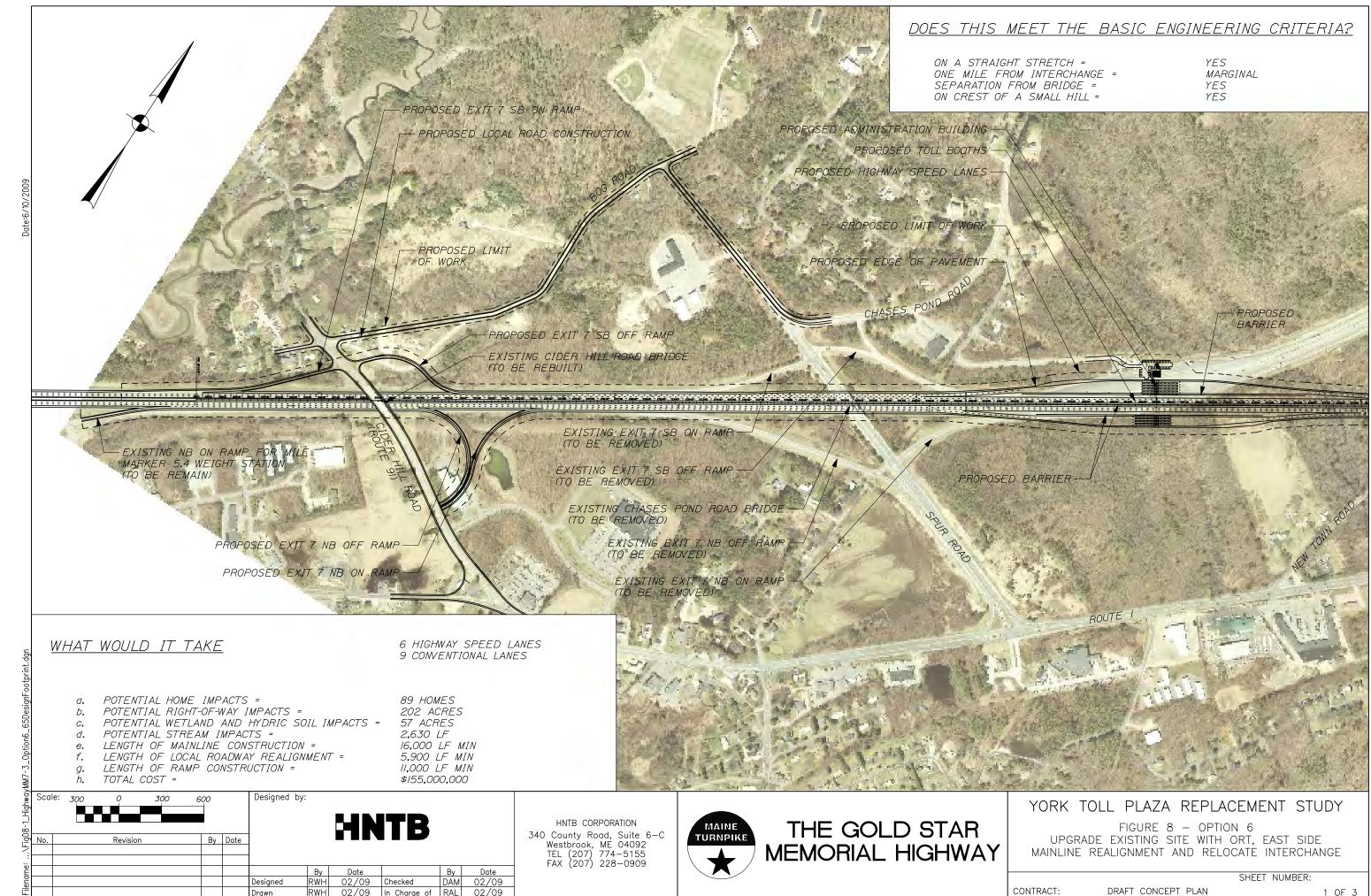
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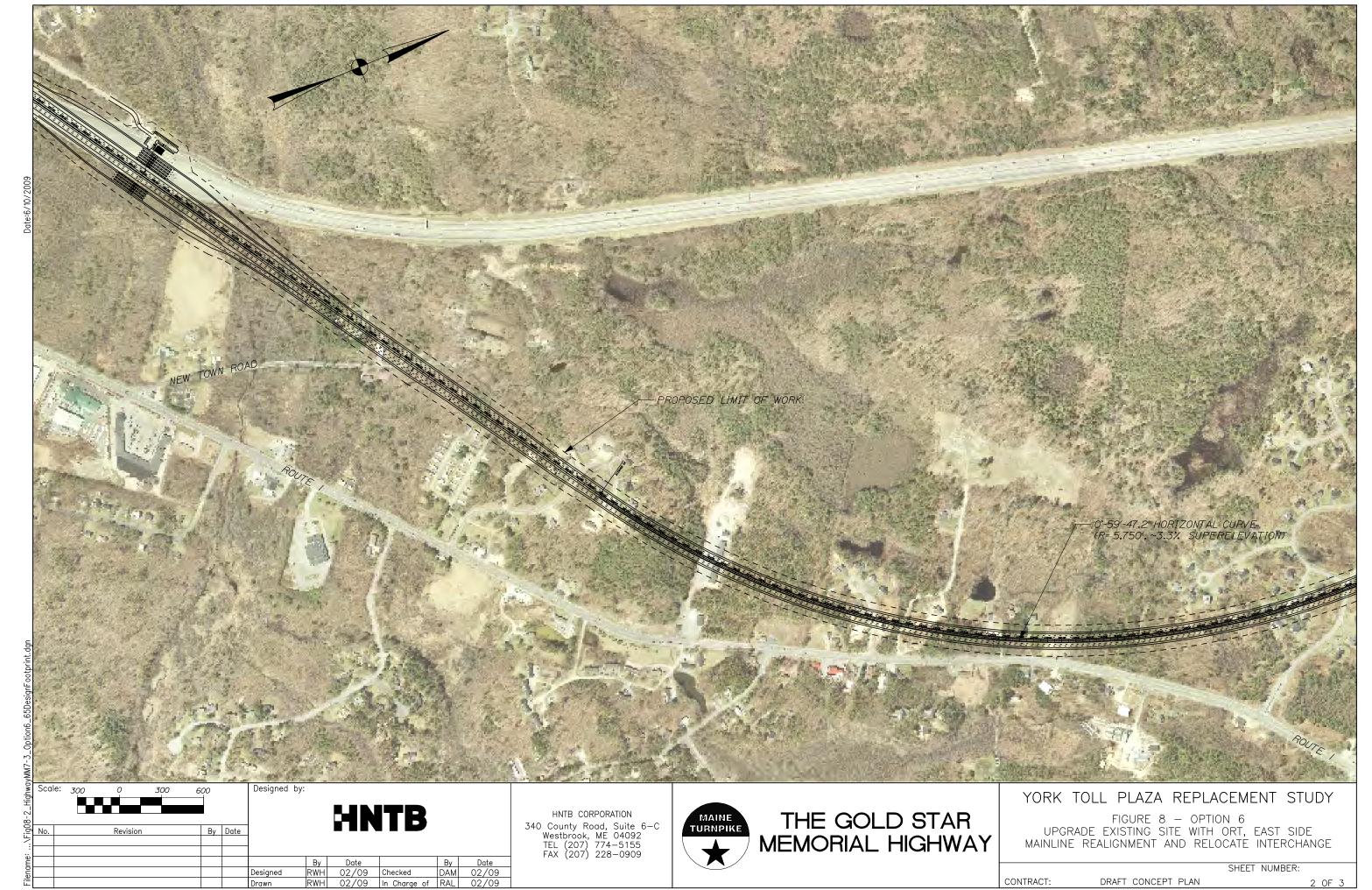
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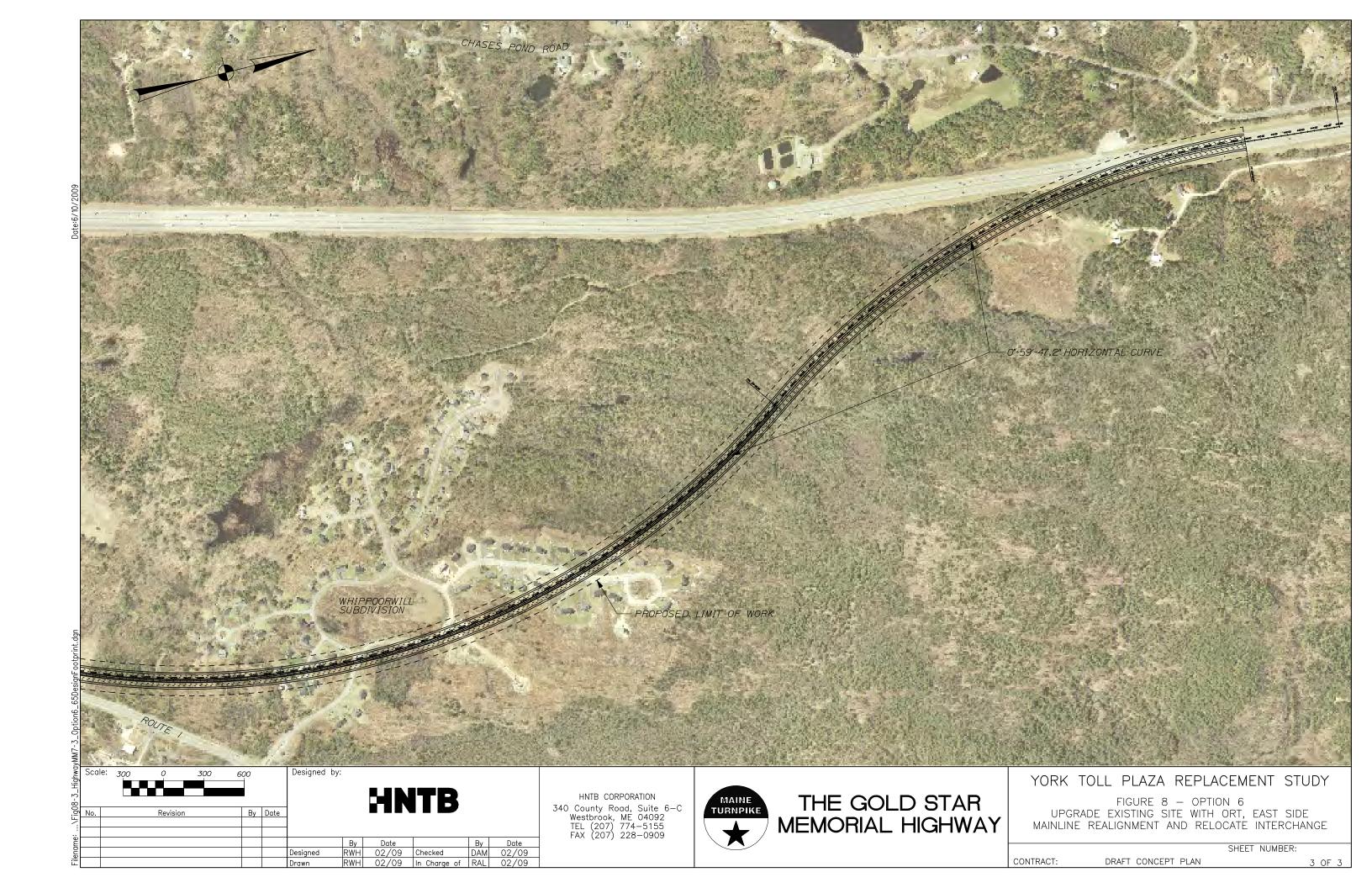


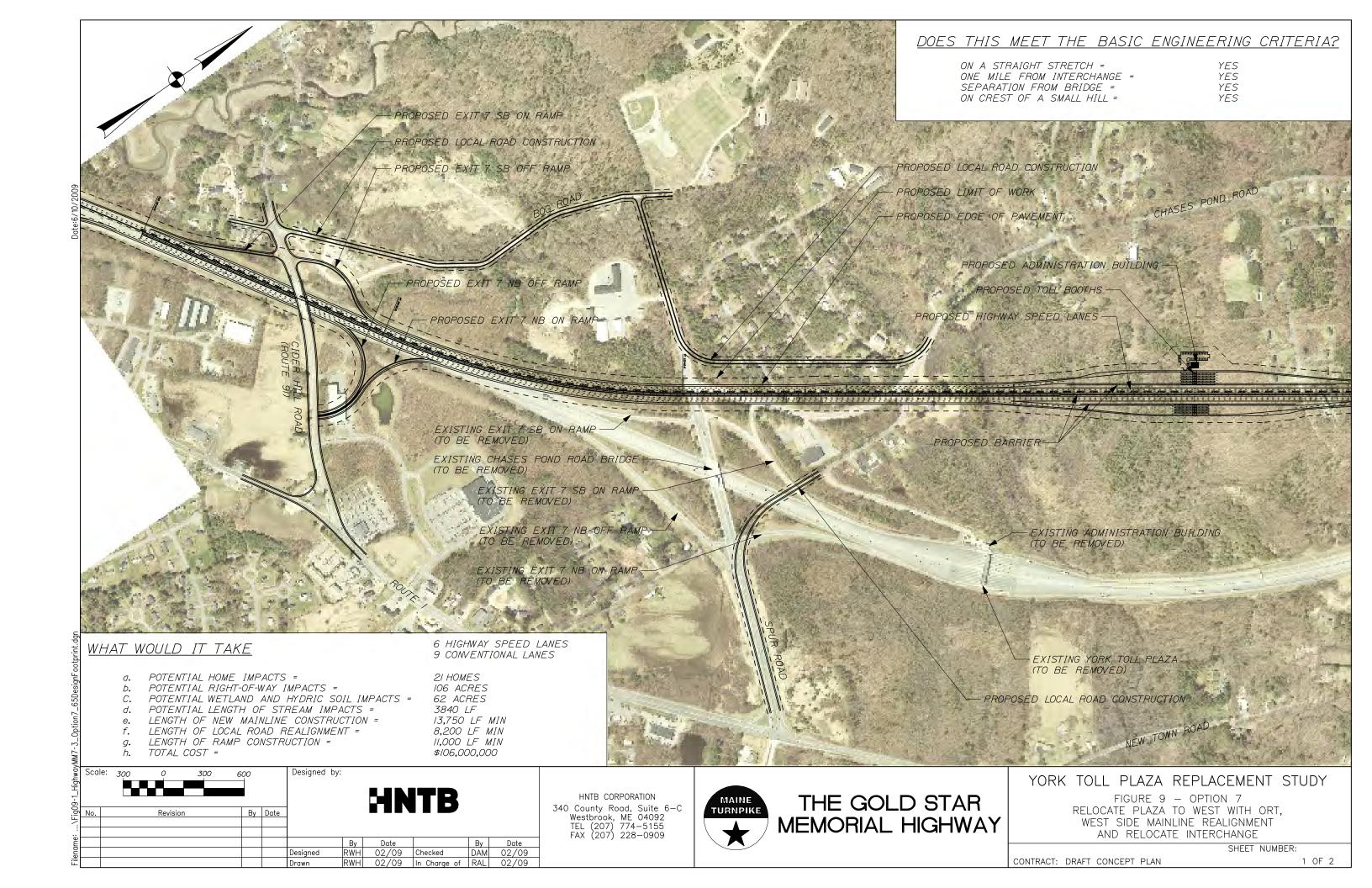


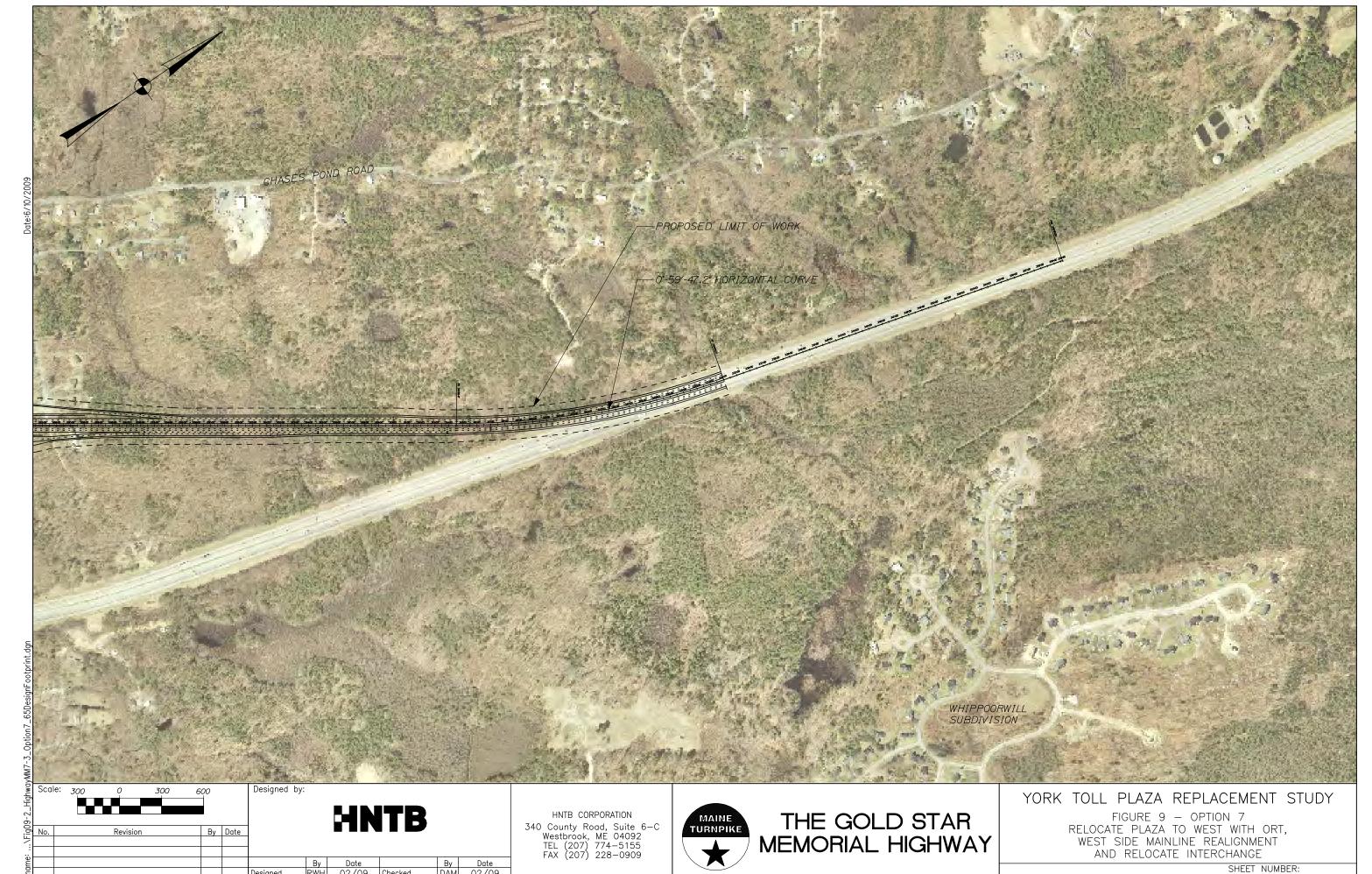
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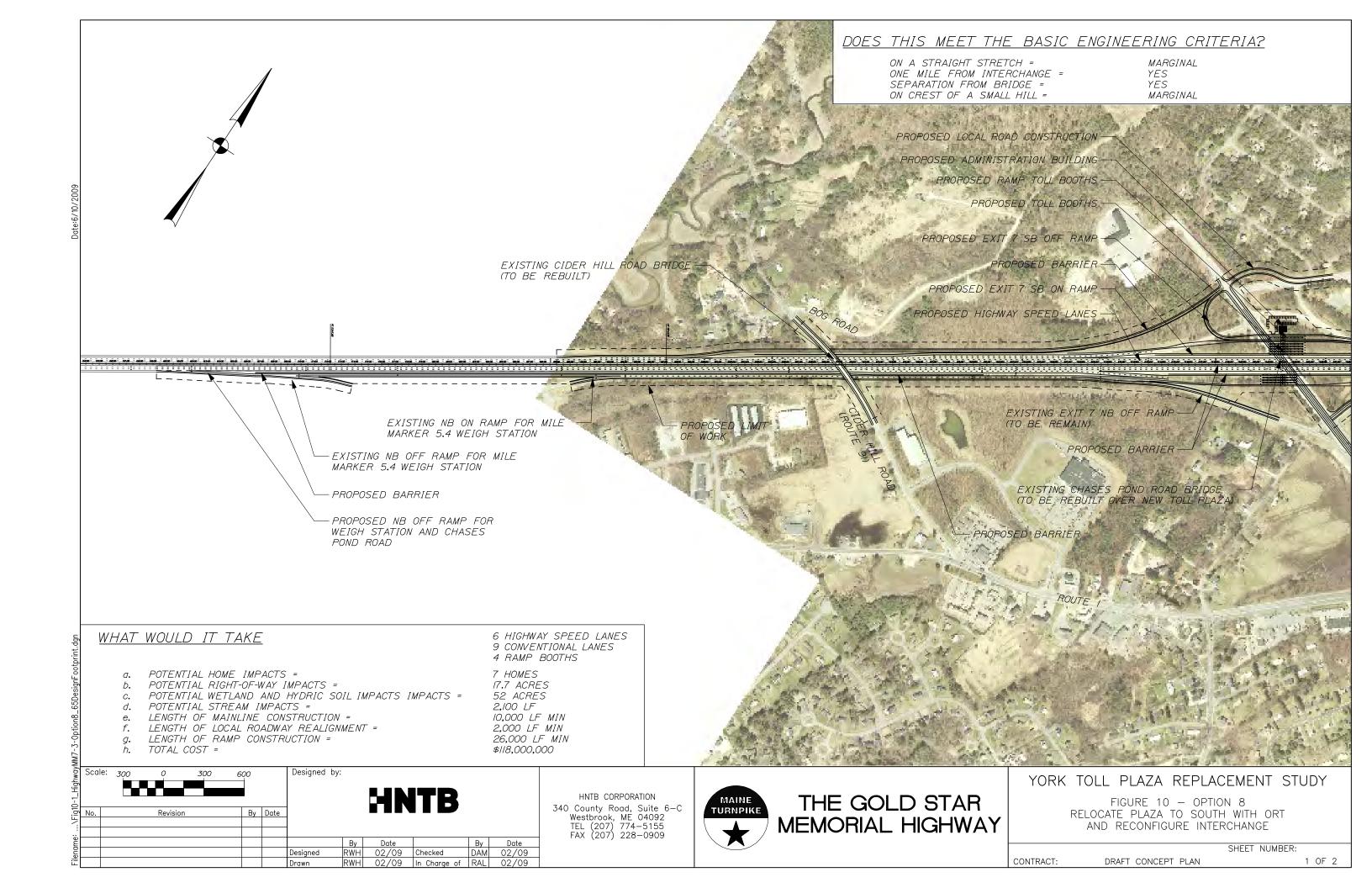


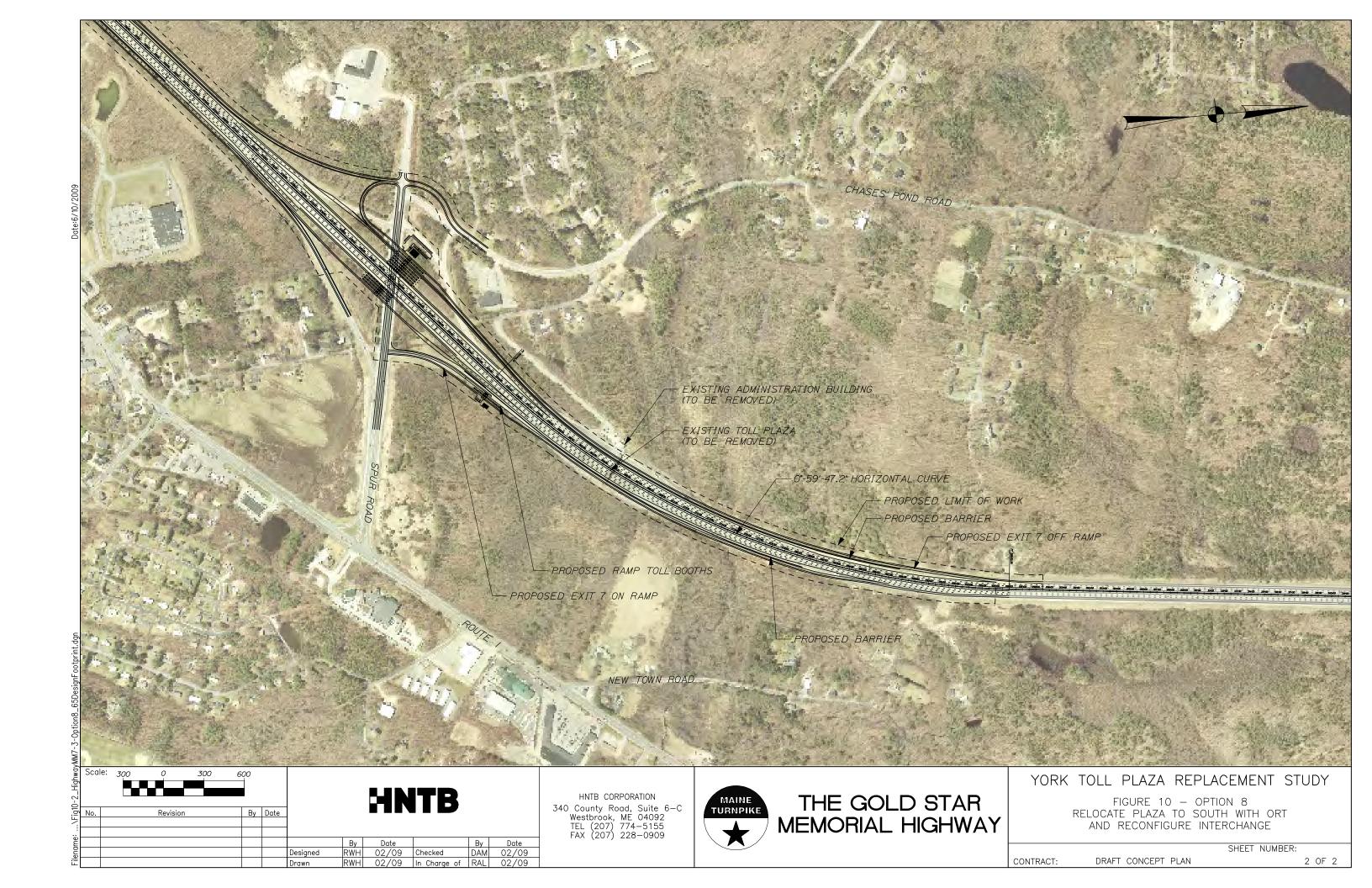


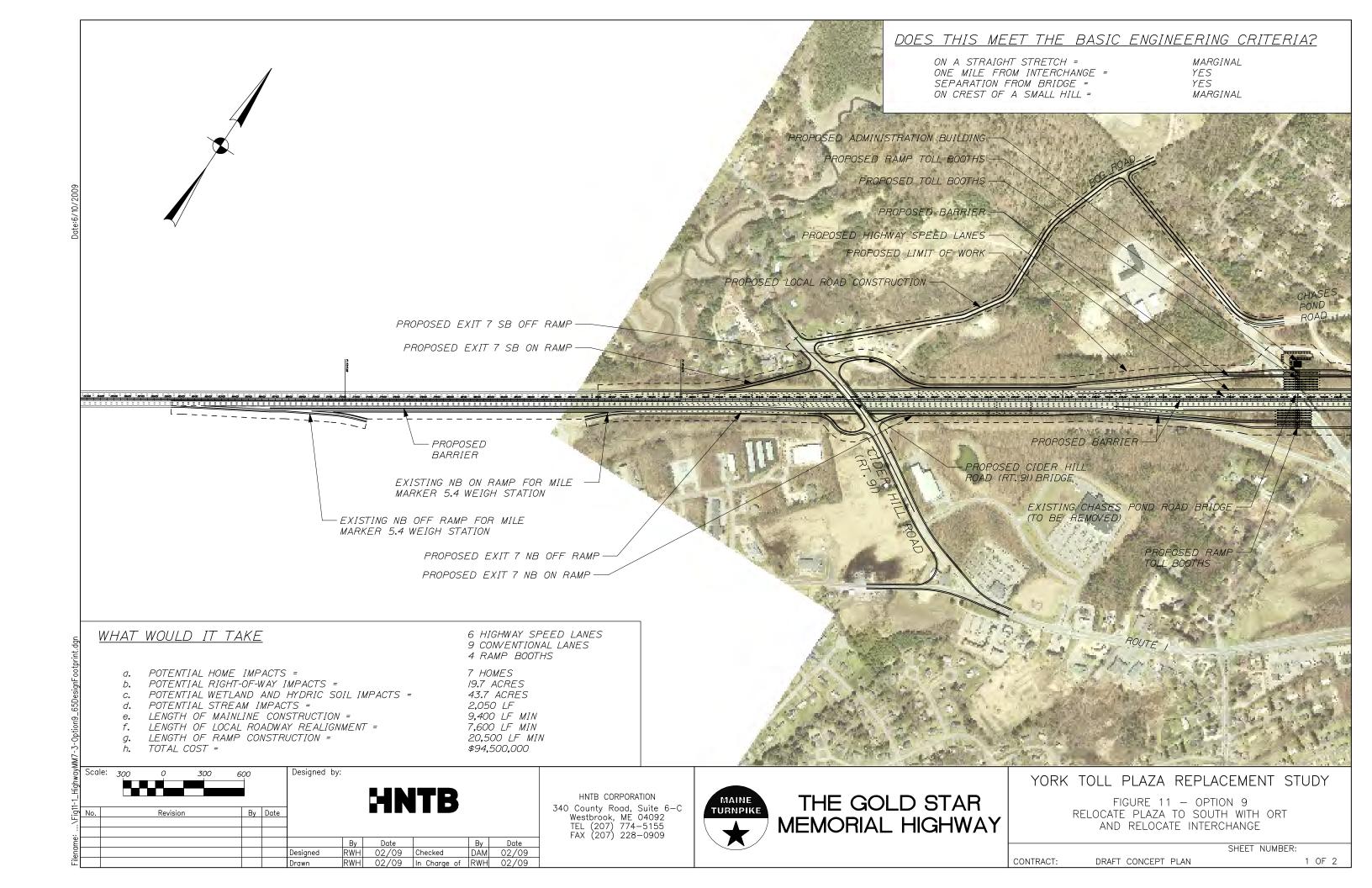


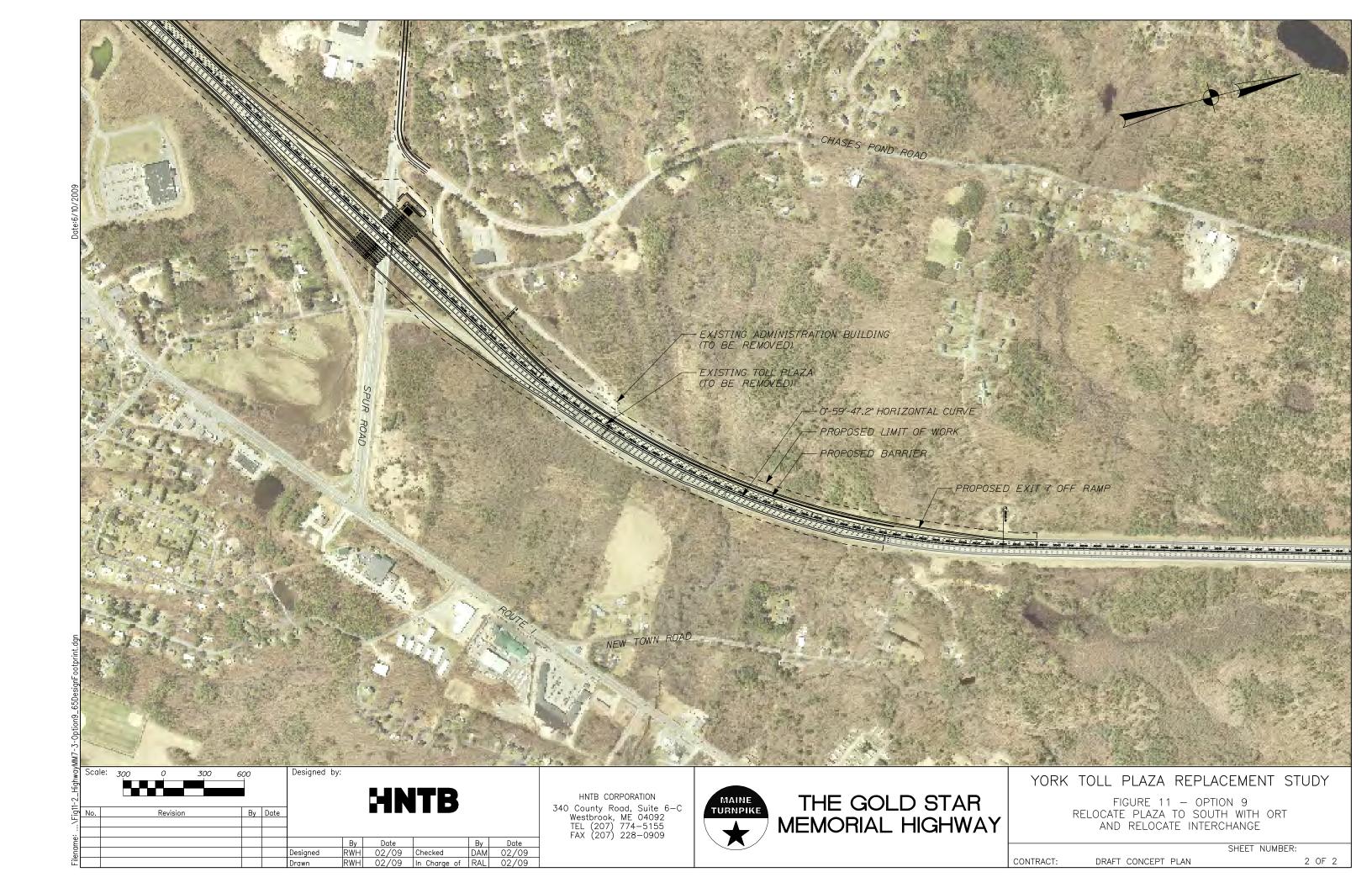
Designed Drawn SHE CONTRACT: DRAFT CONCEPT PLAN

2 OF 2









	Option 1: Existing Site No Build (Maintenance Only)	Option 2: Existing Site Infrastructure Upgrade with No New Capacity	Option 3: Existing Site Upgrade with Conventional Tolling and Separate Ramp Booths	Option 4A: Upgrade Existing Site with Open Road Tolling and Separate Ramp Lanes	Option 4B: Upgrade Existing Site with Open Road Tolling without Separate Ramp Lanes	Option 6: Upgrade Existing Site with Open Road Tolling, East Side Mainline Realignment, and Relocate Interchange	Option 7: Relocate Plaza to West with Open Road Tolling, West Side Mainline Realignment, and Relocate Interchange	Option 8: Relocate Plaza to South with Open Road Tolling and Reconfigure Interchange	Option 9: Relocate Plaza to South with Open Road Tolling and Relocate Interchange
Plaza Capacity	Current capacity issues would escalate while the lane configuration of the plaza would have to be continually changed to optimize the available lanes.		Plaza would accommodate all but the heaviest traffic volumes with acceptable queuing.	Plaza would accommodate the heaviest traffic volumes with minimal queuing for cash patrons and free flow for ETC patrons.	Plaza would accommodate the heaviest traffic volumes with some queuing for cash patrons and free flow for ETC patrons.	Plaza would accommodate the heaviest traffic volumes with minimal queuing for cash patrons and free flow for ETC patrons.	Plaza would accommodate the heaviest traffic volumes with minimal queuing for cash patrons and free flow for ETC patrons.	Plaza would accommodate the heaviest traffic volumes with minimal queuing for cash patrons and free flow for ETC patrons.	Plaza would accommodate the heaviest traffic volumes with minimal queuing for cash patrons and free flow for ETC patrons.
	Similar alignment to the toll plaza, reducing the need for patron decision making. There is familiarity with this traffic pattern.	Similar alignment to the toll plaza, reducing the need for patron decision making. There is familiarity with this traffic pattern.	Similar alignment to the toll plaza, reducing the need for patron decision making. There is familiarity with this traffic pattern.	Vehicles must decide to use highway speed lanes or exit to cash toll lanes. This will be a new traffic pattern for motorists.	Vehicles must decide to use highway speed lanes or exit to cash toll lanes while on a curve. Does not eliminate the weave potential between Cash and Exit vehicles. This will be a new traffic pattern for motorists.		Vehicles must decide to use highway speed lanes or exit to cash toll lanes. This will be a new traffic pattern for motorists.	toll lanes. Decision point for exit will be in advance of expected exit	Vehicles must decide to use highway speed lanes or exit to cash toll lanes. Decision point for exit will be in advance of expected exit point. This new traffic pattern will be confusing to motorists.
Operations	Electronic toll vehicles must slow as they enter the toll plaza area.	Electronic toll vehicles must slow as they enter the toll plaza area.	Electronic toll vehicles must slow as they enter the toll plaza area.	Provides ETC customers with dedicated highway speed lanes with minimal queuing or speed reduction. This provides the best possible level of service for ETC customers with the higher speeds leading to more efficient operation.	Provides ETC customers with dedicated highway speed lanes with minimal queuing or speed reduction. Level of service for ETC customers will not be highest due to curve and proximity to Exit and Cash/ETC separation. ETC patrons using Exit 7 will use Cash lanes.	Provides ETC customers with dedicated highway speed lanes with minimal queuing or speed reduction. This provides the best possible level of service for ETC customers with the higher speeds leading to more efficient operation.	Provides ETC customers with dedicated highway speed lanes with minimal queuing or speed reduction. This provides the best possible level of service for ETC customers with the higher speeds leading to more efficient operation.	Provides ETC customers with dedicated highway speed lanes with minimal queuing or speed reduction. This provides the best possible level of service for ETC customers with the higher speeds leading to more efficient operation.	Provides ETC customers with dedicated highway speed lanes with minimal queuing or speed reduction. This provides the best possible level of service for ETC customers with the higher speeds leading to more efficient operation.
	Processing of patrons remains the same.	Processing of patrons remains the same.	Processing of cash patrons improved with expanded plaza but processing of ETC patrons limited to same slow vehicle speed.	Increased efficiency of processing patrons - both ETC and cash paying.	Increased efficiency of processing patrons - both ETC and cash paying.	Increased efficiency of processing patrons - both ETC and cash paying.	Increased efficiency of processing patrons - both ETC and cash paying.	Increased efficiency of processing patrons - both ETC and cash paying.	Increased efficiency of processing patrons - both ETC and cash paying.
	Vehicles must access the dedicated toll lanes via the toll plaza approach area. Excessive vehicle queue in the approach area impacts access and efficiency of dedicated toll lanes.	Vehicles must access the dedicated toll lanes via the toll plaza approach area. Excessive vehicle queue in the approach area impacts access and efficiency of dedicated toll lanes.	Vehicles must access the dedicated toll lanes via the toil plaza approach area. Excessive vehicle queue in the approach area impacts access and efficiency of dedicated toll lanes.	ETC patrons are not effected by queuing at tolling lanes. Cash lane queues minimized by removal of ETC patrons from cash lanes.	Thru ETC patrons are not effected by queuing at tolling lanes. Exit 7 ETC patrons must utilize Cash lanes. Cash lane queues minimized by removal of ETC patrons from cash lanes.	ETC patrons are not effected by queuing at tolling lanes. Cash lane queues minimized by removal of ETC patrons from cash lanes.	queuing at tolling lanes. Cash lane	ETC patrons are not effected by queuing at tolling lanes. Cash lane queues minimized by removal of ETC patrons from cash lanes.	ETC patrons are not effected by queuing at tolling lanes. Cash lane queues minimized by removal of ETC patrons from cash lanes.
Total Project Cost  Potential wetland impacts	12.3 Million  O acres anticipated	\$23.0 Million  Potential 3 acres impacted	\$40.9 Million  Potential 7 acres impacted.	\$56.3 Million  Potential 9 acres impacted.	\$43.0 Million  Potential 5 acres impacted.	\$155 Million  Potential 18 acres impacted	\$106 Million  Potential 13 acres impacted	\$118 Million  Potential 3 acres impacted	\$94.5 Million  Potential 4 acres impacted
(NWI Certified)  Potential wetland impacts	O acres anticipated	Potential 11 acres impacted	Potential 17.6 acres impacted.	Potential 28 acres impacted.	Potential 22.2 acres impacted.	Potential 57 acres impacted	Potential 62 acres impacted	Potential 52 acres impacted	Potential 43.7 acres impacted
(NRCS soils)		·	Replace plaza approximately 200 ft	Replace plaza approximately 200 ft	Replace plaza approximately 200 ft			Relocate below the Chases Pond	Relocate below the Chases Pond
General Layout	Existing plaza remains  Exit 7 Ramp Traffic and Mainline Traffic remain mixed	north of existing plaza.  Exit 7 Ramp Traffic and Mainline Traffic remain mixed	north of existing plaza.  Exit 7 Ramp Traffic is separated to/from plaza.	north of existing plaza.  Exit 7 Ramp Traffic is separated to/from plaza.	north of existing plaza.  Exit 7 Ramp Traffic is not separated to/from plaza.	Relocate plaza in existing location  Exit 7 Ramp Traffic is separated to/from plaza.	Relocate plaza west of existing site  Exit 7 Ramp Traffic is separated to/from plaza.	Road Bridge  Exit 7 Ramp Traffic is separated to/from plaza.	Road Bridge  Exit 7 Ramp Traffic is separated to/from plaza.
Horizontal Alignment	Plaza is not located on tangent.	Plaza is not located on tangent.	Plaza is not located on tangent.	Plaza is not located on tangent.	Plaza is not located on tangent.	Plaza Area would be located on a tangent.	Plaza Area would be located on a tangent.	Plaza Area would partially be located on a tangent.	Plaza Area would partially be located on a tangent.
Vertical Alignment	Existing Plaza is at a low point, not the recommended high point.	Existing Plaza is at a low point, not the recommended high point.	required to create localized high	required to create localized high	Vertical grade adjustment would be required to create localized high point. Plaza still at base of 5% hill to the North.		Plaza at high point, minor vertical grade adjustments possible.	Vertical grade adjustment would be required to create localized high point. Plaza still at base of 5% hill to the North.	Vertical grade adjustment would be required to create localized high point. Plaza still at base of 5% hill to the North.
Sight Distance	Decision sight distance is not completely satisfied.	Decision sight distance is not completely satisfied.	Decision sight distance is not completely satisfied.	Decision sight distance is not completely satisfied.	Decision sight distance is not completely satisfied.	Decision sight distance is satisfied.	Decision sight distance is satisfied.	Decision sight distance is satisfied.	Decision sight distance is satisfied.
Proximity of plaza to interchanges / bridges	Recommended 1 mile separation from plaza and interchange is not met. Close proximity of Chase's Pond Rd Exit creates safety issues for vehicles. NB mainline lanes between entrance ramp and plaza is a high crash location.	Recommended 1 mile separation from plaza and interchange is not met. Close proximity of Chase's Pond Rd Exit creates safety issues for vehicles. NB mainline lanes between entrance ramp and plaza is a high crash location.	Recommended 1 mile separation from plaza and interchange is not met.	Recommended 1 mile separation from plaza and interchange is marginally met.	Recommended 1 mile separation from plaza and interchange is not met.	Recommended 1 mile separation from plaza and interchange is marginally met	Recommended 1 mile separation from plaza and interchange will be met.	Recommended 1 mile separation from plaza and interchange will be met.	Recommended 1 mile separation from plaza and interchange will be met.
Geotechnical conditions		Existing site has settlement issues. Approach slabs and bumpers at toll booths are settling. This creates hang-up points for vehicles with low ground clearance and safety issues for toll attendants.	Geotechnical issues at toll plaza may require use of light weight fill.	Geotechnical issues at toll plaza may require use of light weight fill.	Geotechnical issues at toll plaza may require use of light weight fill.	Geotechnical issues at toll plaza may require use of light weight fill.	Geotechnical issues are unknown.	Geotechnical issues are unknown.	Geotechnical issues are unknown.
		0 Displacements Possible	O Displacements Possible	O Displacements Possible	O Displacements Possible	89 Displacements Possible	21 Displacements Possible	7 Displacements Possible	7 Displacements Possible
Potential displacements  Potential Right-of-Way	O Displacements Possible	O Displacements 1 ossible	O Displacements Possible	o bispideements i ossibie	o bispideements i ossibie				1 Displacements 1 ossible

#### SECTION 8 - REHABILITATE/RECONSTRUCT RECOMMENDATION

Considering all the factors detailed in this existing site evaluation including the plaza"s crash history, operational inefficiency, structural deficiency, and its location such that these conditions compromise overall staff and patron safety, HNTB recommends replacement, and not repair of the York Toll Plaza. To determine the most effective course of action and meet the project purpose and need the following Option summaries are offered followed by a final recommendation. The Option(s) that warrant further consideration will be recommended to be carried forward into the full Site Identification and Screening process. As mentioned earlier, a full and thorough study will include options at alternative sites. The following is a summary of the nine options evaluated along with their respective recommendation.

#### Option 1: No Build (Maintenance Only)

Option 1 does not satisfy any of York Toll Plaza's safety or operational needs, present or future. This option leaves the Plaza requiring extensive and costly ongoing maintenance. However, standard procedure for permitting agencies is to use the No-Build option as a benchmark and compare it to other proposed possibilities. This Option is required by the permitting agencies to be carried forward for further consideration.

### Option 2: Infrastructure Upgrade

Option 2 addresses only the structural deficiencies of the existing infrastructure. This option does not address the location related deficiencies, does not meet current industry design guidelines and will not address many safety or operational issues for Turnpike patrons and staff. In short, this option does not meet the Maine Turnpike Authority's objective of a safe and efficient modern toll plaza. The layout carries anticipated impacts of 0 home displacements, 1.5 acres of right-of-way, and 11 acres of wetlands and an approximate total cost of \$23 million. The cost to provide this option would be lost without benefit as it would not remedy any of the truly needed safety improvements. **This Option is recommended to be dismissed from further consideration.** 

#### Option 3: Upgrade Existing Site with Conventional Tolling and Separate Ramp Lanes

Option 3 upgrades the infrastructure, addresses some of the traffic flow inefficiency, but does not address the safety and operational concerns associated with the current plaza location. This option does not meet the current basic design guidelines. In short, this option does not meet the Maine Turnpike Authority's objective of a safe and efficient modern toll plaza. The layout carries anticipated impacts of 0 home displacements, 6.3 acres of right-of-way, and 17.6 acres of wetlands and an approximate total cost of \$40.9 million. The cost of this option weighed against the marginal benefits is not prudent. In addition, there is no opportunity for implementing modern Open Road Lanes with this option. **This Option is recommended to be dismissed from further consideration.** 

#### Option 4A: Upgrade Existing Site with Open Road Tolling and Separate Ramp Lanes

Option 4A implements open road tolling, improves traffic capacity and ETC processing time but fails to address some of the safety concerns associated with the current plaza location. The addition of dedicated ramp toll lanes does remove the merge and weave conditions between

mainline and ramp traffic but creates potentially confusing traffic signage. This option addresses the proximity of the interchange in the most effective manner considering the constraints. It removes the weaves and merges by extending the interchange beyond the toll plaza location similar to the Hampton Toll Plaza (Hampton) in New Hampshire. Unlike Hampton, the interchange will not be in view at the decision point, due to the vertical and horizontal geometry, adding to possible confusion. This option does not meet three of the four current basic design guidelines. Full benefits of Open Road Tolling will not be realized due to the location on a curve and near a hill. Environmental impacts of this option, although significant, are less than some others in this evaluation. The layout carries anticipated impacts of 0 home displacements, 8.1 acres of right-of-way, and 28 acres of wetlands and an approximate total cost of \$56.3 million. Option 4A, while not meeting all the MTA goals; does address some of the major safety issues and has comparatively reasonable impacts and cost, and is therefore recommended to be carried forward for further consideration and comparison to other locations.

Option 4B: Upgrade Existing Site with Open Road Tolling without Separate Ramp Lanes

Option 4B marginally improves traffic capacity and ETC processing time but fails to address all traffic safety concerns associated with the current plaza location. Separating open road toll patrons from the cash and ramp traffic improves the merge and weave issue similar to Option 4A along with the potential confusion. However requiring cash and ramp traffic to utilize the same lanes allows continued merge and weave situations for that traffic stream; thus not completely addressing the issue. This option does not meet the four basic design guidelines. In fact, minimizing the length of barrier separation has potentially created a new safety concern. The leading end of barrier only comes into view two seconds earlier than the minimum recommended of 14 seconds. Full benefits of Open Road Tolling will not be realized due to the location on a curve and near a hill requiring slower speeds. Environmental impacts for this option, are significant. The layout carries anticipated impacts of 0 home displacements, 3.3 acres of right-of-way, and 22.2 acres of wetlands and an approximate total cost of \$43 million. Option 4B has comparable impacts and a marginally reduced cost when compared to that of Option 4A but provides far less benefit; in fact it introduces additional safety concerns over Option 4A. However, given the magnitude of home, right-of-way and environmental impacts of the other existing site alternatives, Option 4B offers the next closest approach to Option 4A to meeting design guidelines, MTA goals and project purpose and need and reduced cost and impacts. Therefore Option 4B is recommended to be carried forward for further consideration and comparison to other locations.

### Option 5: Relocate Plaza to Alternate Location with Open Road Tolling

Investigation of alternative locations was suspended, in order to focus the comprehensive evaluation on the existing toll plaza area. It should be noted, as part of the next project phase alternative sites are recommended to be revisited with newly developed plaza sizing and other traffic statistics to continue their development.

Option 6: Upgrade Existing Site with Open Road Tolling, East Side Mainline Realignment, and Relocate Interchange

Option 6 will provide an Open Road Tolling facility that generally meets the basic engineering criteria and improves safety and plaza operations however, the s-curves in the horizontal alignment north of the plaza are not desirable. The layout carries anticipated impacts of 89 home displacements, 202 acres of right-of-way, and 57 acres of wetlands and an approximate total cost of \$155 million. In short, this option is not economically feasible when weighed against other available options; the human and environmental impacts alone are staggering. **This Option is recommended to be dismissed from further consideration.** 

Option 7: Relocate Plaza to West with Open Road Tolling, West Side Mainline Realignment, and Relocate Interchange

Option 7 will provide an Open Road Tolling facility that meets the basic engineering criteria and improves safety and plaza operations. However, the layout carries anticipated impacts of up to 21 home displacements, 106 acres of right-of-way, and 62 acres of wetlands and an approximate total cost of \$106 million. In short, this option is not economically feasible when weighed against other available options; the human and environmental impacts alone are huge. **This Option is recommended to be dismissed from further consideration.** 

Option 8: Relocate Plaza to South with Open Road Tolling and Reconfigure Interchange

Option 8 will provide an Open Road Tolling facility that generally meets the basic engineering criteria and improves safety and plaza operations. One of the more notable drawbacks to this option is the potentially confusing arrangement of interchange ramps and signing packages that would be required to direct motorists through unconventional traffic patterns. The layout carries anticipated impacts of up to 7 home displacements, 17.7 acres of right-of-way and 52 acres of wetlands and an approximate total cost of \$118 million. In short, this option is not economically feasible when weighed against other available options; the environmental impacts alone are huge. **This Option is recommended to be dismissed from further consideration.** 

Option 9: Relocate Plaza to South with Open Road Tolling and Relocate Interchange

Option 9 will provide an Open Road Tolling facility that generally meets the basic engineering criteria and improves safety and plaza operations. One of the more notable drawbacks to this option is the potentially confusing arrangement of interchange ramps, weigh station ramps and signing packages that would be required to direct motorists through unconventional traffic patterns. The layout carries anticipated impacts of up to 7 home displacements, 19.7 acres of right-of-way, and 43.7 acres of wetlands and an approximate total cost of \$94.5 million. In short, this option is not economically feasible when weighed against other available options; the environmental impacts alone are huge. **This Option is recommended to be dismissed from further consideration.** 

#### Recommendation

At the request of the Maine Turnpike Authority, HNTB has completed its "existing site re-evaluation". The goal of the re-evaluation, as described by the York Selectpersons, was to investigate "out-of-the-box" or "what it would take" alternatives that would meet design criteria, minimize impact to right-of-way and avoid taking homes. Based on additional investigation of the existing toll plaza area to identify these potential alternatives which meet basic engineering guidelines, meet MTA goals, and meet the purpose and need for the York Toll Plaza Replacement project, HNTB did not identify any alternative that fully met all parameters. However, two alternatives were identified that warrant further study.

Option 4A - Upgrade Existing Site with Open Road Tolling and Separate Ramp Booths, was an alternative that did meet some of the basic safety criteria, did implement open road tolling and kept home displacements to zero. Resulting right-of-way and environmental impacts, although significant were at the lower end of the existing site alternatives developed. While not meeting all of the MTA goals or the total project purpose and need, and considering all evaluation parameters, Option 4A provides the most improvements and is more reasonable than any of the other existing site alternatives. It should be noted that the cost of Option 4A is quite high especially when considering the few benefits realized and the numerous deficiencies remaining.

Similarly, HNTB recognizes Option 4B - Upgrade Existing Site with Open Road Tolling without Separate Ramp Booths, as an alternative that meets some of the basic safety criteria and does implement open road tolling. However, Option 4B still does not address all the MTA goals, all of the design guidelines, or the total project purpose and need. This option is marginally less expensive than Option 4A but leaves more deficiencies unaddressed. Option 4B is however, the alternative that has the least amount of right-of-way and environmental impacts while still implementing open road tolling. It should be reiterated here that Option 4B does introduce an additional safety concern due to only a partial separation of interchange traffic from mainline traffic.

HNTB recommends Option 4A and Option 4B, in addition to the No-Build Option 1, to be carried forward for further consideration. HNTB further recommends that these three options be included in a full Site Identification and Screening process where they will be more fully developed and compared to alternate site options. This further investigation of alternative sites and comparison to existing site options will be required by the environmental permitting agencies as part of a thorough permitting process.

Finally, based on our accumulated knowledge of this project and the advanced engineering that has resulted from this study of the existing site, including the significant reduction in the size of the plaza, HNTB believes that alternative locations exist that will enable the Authority to:

- Comply with national safety guidelines for toll plazas
- Avoid displacements of any homes
- Minimize wetland and other environmental impacts
- Minimize impacts to private property
- Integrate a more modern and efficient Open Road Tolling technology and
- Reduce the cost of the project.

# PART 3 ALTERNATE SITE EVALUATION

### **SECTION 1 - INTRODUCTION**

The Existing Site Evaluation produced no options at the existing location that fully met the Basic Project Purpose and Need without excessive environmental and social impacts and excessive costs. Therefore, following the Existing Site Evaluation, it was clear that the investigation into replacement of the York Toll Plaza would need to extend beyond the immediate area surrounding the existing Plaza. As recommended by HNTB in the Existing Site Evaluation, and subsequently approved by the Maine Turnpike Authority, and as outlined within USACE"s Highway Methodology, an investigation into alternate sites was completed to provide a reasonable range of alternatives. The Alternate Site Evaluation portion of this report documents the investigation and findings of new, potential locations for the replacement of the York Toll Plaza

New candidate locations were identified by considering the same basic design criteria as was used for the Existing Site Evaluation (ESE). Additionally, the impacts of these new plaza locations on both social and environmental resources were estimated using the same data sets and methodology as in the ESE. The following is a brief summary of each of the variables used for these evaluations, including engineering design criteria, proposed plaza configurations, and site-specific physical features and constraints such as existing natural resources, density of development, and land availability.

## **SECTION 2 - ENGINEERING**

As described in Part 2 Existing Site Evaluation, the MTA voted to advance open road (highway speed) tolling using a single toll plaza configuration and current design guidelines. The conceptual toll plaza is shown in Figure 3.1 and the typical sections are shown in Figure 3.2. This is the same configuration used in the Existing Site Evaluation. The following discussion describes the elements and design features of the proposed toll plaza used for both the Existing Site Evaluation and this Alternate Site Evaluation.

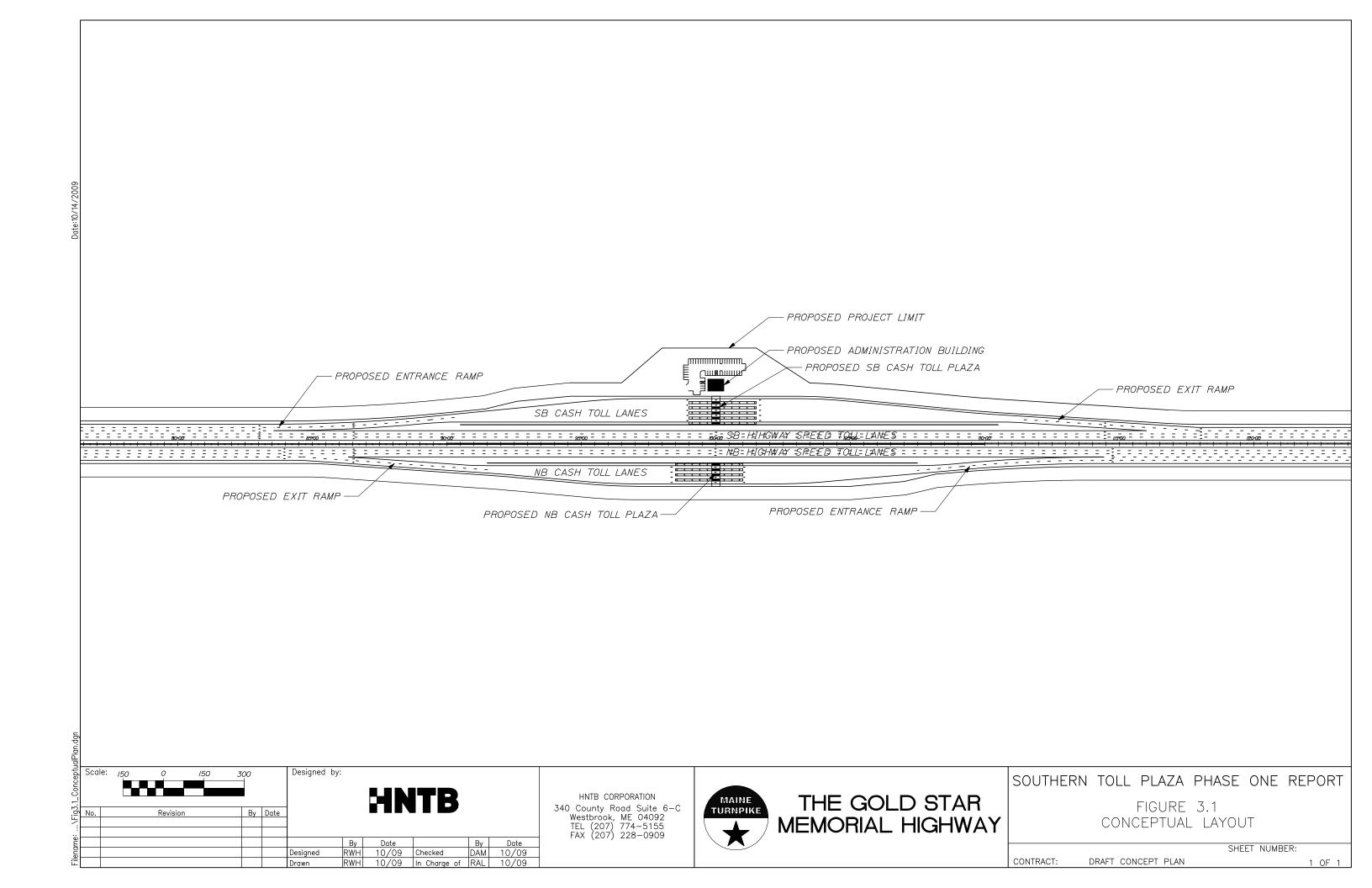
The location of the highway speed lanes has been developed in accordance with FHWA Guidelines. The highway speed lanes are located inside of the conventional plaza (cash lanes) and are a continuation of the existing mainline roadway where the alignment, travel lanes, shoulder, and cross slopes match the existing roadway approaching the plaza. The proposed layout for the opening year provides two highway speed lanes in each direction within the plaza area. Three highway speed lanes in each direction will be provided in a future year based on the demand for E-ZPass and the corresponding decrease in cash lane demand. To minimize the overall plaza footprint, the innermost cash toll booth and lane will be removed and reconstructed as the third highway speed lane when needed. The result would be a reuse of this lane thus minimizing the overall plaza footprint, reducing environment and right-of-way impacts and costs.

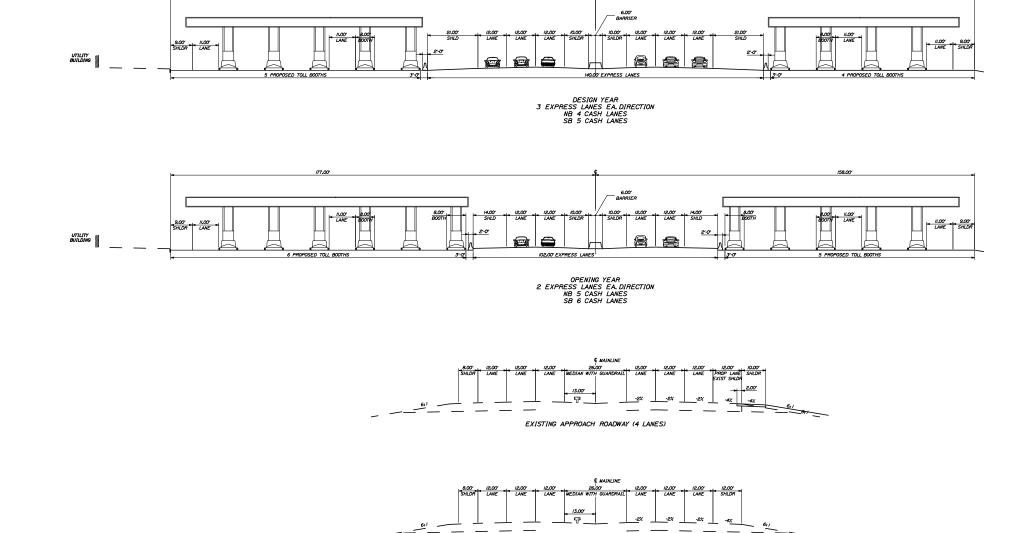
#### **Exit & Entrance Ramps**

The conventional cash plaza exit and entrance ramps have been designed as interchange ramps in accordance with American Association of State Highway and Transportation Officials (AASHTO) policies and with the Federal Highway Authority's State of the Practice and Recommendations on Traffic Control Strategies at Toll Plazas.

**Exit Ramp:** The roadway connecting the mainline turnpike with the cash toll plaza, allowing vehicles to "exit" the mainline turnpike. The layout of the plaza approach transition (or split) between the highway speed lanes and cash lanes was designed with AASHTO policies for a two-lane tapered exit ramp and a major fork.

**Entrance Ramp:** The roadway connecting the cash toll plaza with the mainline turnpike allowing vehicles to "enter" back onto the mainline turnpike. The layout of the plaza departure transition (or merge) between the highway speed lanes and cash lanes was designed in accordance with AASHTO policies for a two-lane tapered entrance ramp.





EXISTING APPROACH ROADWAY (3 LANES)

Scale:	20 0	20 40		Designed by	y:				
						H	ITB		
No.	Revision	Ву	Date						
					By	Date		By	Date
				Designed	RWH	10/09	Checked	DAM	10/09
				Drawn	RWH	10/09	In Charge of	RAL	10/09

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SOUTHERN TOLL PLAZA PHASE ONE REPORT
FIGURE 3.2
TYPICAL SECTIONS

SHEET NUMBER: CONTRACT:

#### Plaza Layout

The open road toll plaza layout, including cash booths lanes and highway speed lanes, is approximately 158 and 177 feet wide from the roadway centerline, northbound and southbound respectively. This layout will accommodate three highway speed lanes in each direction with typical outside shoulders and median widths for the turnpike. The opening year would see two highway speed lanes in each direction, with conversion to three lanes following an increase in E-ZPass use warranting such conversion. The highway speed lanes would be physically separated from the conventional cash plaza by a concrete barrier. In the future, as the demand for additional highway speed lanes is expected to increase, the need for cash lanes within the conventional plaza should decrease.

#### Administration Building, Access and Facilities

To complete development of the conceptual plaza layout, the sizes of support facilities such as the administration building, parking area, and other plaza infrastructure have been approximated.

**Building:** The administration building for the York Toll Plaza is estimated to be 45 feet by 60 feet. This building provides an additional 600 square feet of space when compared with the existing York Toll Plaza building for modern infrastructure needs.

**Access/Parking:** Access to the building's parking lot is proposed to be from a local street and in accordance with FHWA Guidelines. The lot is estimated to accommodate 34 vehicles which includes provisions for:

- 14 parking spaces for booth attendants
- 14 additional parking spaces for booth attendants at shift change
- 2 parking spaces for supervisors
- 2 parking spaces for maintenance
- 2 parking spaces for visitors

The layout of the parking lot, which includes 9 feet by 18 feet parking stalls with 26-foot wide aisles, is in accordance with MaineDOT Design Guide (MDG) parking lot guidelines. The parking lot layout will be finalized during final design given the topography and other site conditions.

**Canopy:** The canopy size installed on recent Maine Turnpike projects including the Westbrook Interchange and Jetport Interchange is 30 feet wide. Similarly, the width of the existing York Toll Plaza canopy is 30 feet, therefore a 30-foot wide canopy is proposed for this new plaza facility.

**Booth Size:** The toll island widths provided on recent Maine Turnpike projects have consistently been 8 feet to accommodate a 6-foot wide toll booth with adequate clearance on either side. This is necessary to accommodate modern toll infrastructure, adequate staff accommodations, and safety.

**Tunnel:** A tunnel or bridge is required for toll personnel to safely access the plaza booths, including access across highway speed lanes, so no attendant will be required to negotiate these highway speed vehicles. Per FHWA Guidelines, toll collectors should not have to cross more

than one live (cash) toll lane, for safety reasons. A tunnel or bridge with access to every third booth is necessary.

The dimensions of tunnel should allow for adequate space for personnel movement, electrical equipment, electronic toll collection (ETC) equipment, and drainage provisions.

**Lighting:** Lighting of the plaza facility including the highway speed lanes is necessary, and will be developed during final design.

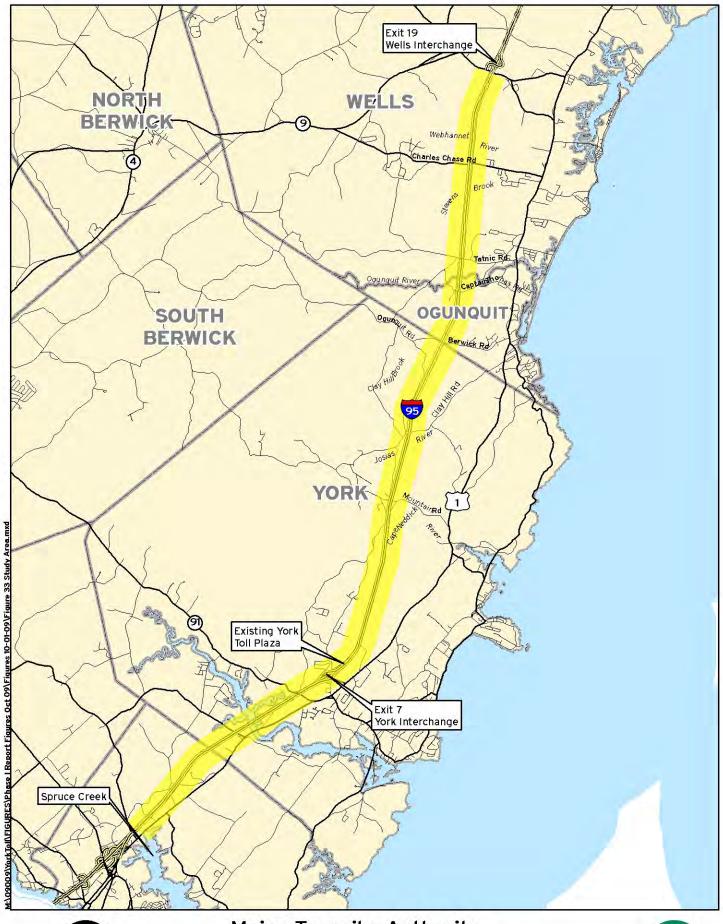
**Drainage:** Based on initial review of stormwater management considerations, a drainage treatment pond is required for new impervious surface. The drainage treatment will be finalized during final design.

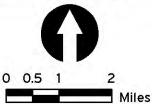
Treatment of storm water in the median between the highway speed lanes and conventional plaza was considered, but dismissed because the available space was only sufficient for water quality treatment and not water quantity management. Also, it resulted in a wider plaza footprint.

If a new toll plaza site is selected, excess impervious area at the existing York Toll Plaza will be removed and re-vegetated, thereby helping to offset some of the new impervious surface impacts from a new toll plaza.

# SECTION 3 - STUDY AREA: PHYSICAL FEATURES, CONSTRAINTS AND IDENTIFICATION OF CANDIDATE TOLL PLAZA LOCATIONS

The York Toll Plaza is the southernmost tolling point and is one of the mainline plazas within the closed barrier system. The first step in finding a replacement site for this plaza is to identify the corridor in which it must be located. The York Toll Plaza must be located such that it collects tolls from the maximum number of patrons entering the State of Maine from I-95 in New Hampshire; maintains equitable tolls for users of Exit 7 in York; minimizes diversion from the turnpike to local roads; and, is located south of Exit 19, Sanford Road (Route 109), in Wells. While there are a number of factors surrounding the location of a toll plaza south of Exit 7 and north of Berwick Road, in the interest of a thorough investigation, this study assesses the merits of such plaza locations. The Study Area is shown on Figure 3.3 and is defined as Spruce Creek in Kittery to Exit 19 in Wells. As was the case for the Existing Site Evaluation, each alternate location must be evaluated with respect to its adherence to engineering guidelines and the various natural and social impacts generated.



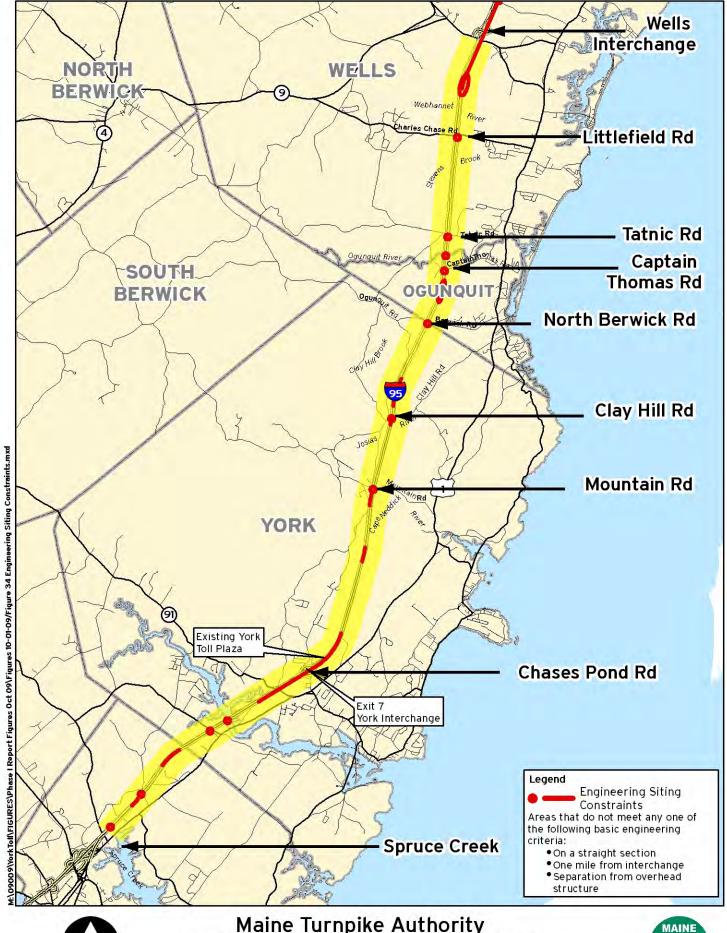


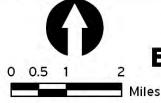
Maine Turnpike Authority Southern Toll Plaza - Phase I Report **Study Area** Figure 3.3



#### **Engineering Constraints**

The initial engineering identification of new plaza locations was based on the vertical and horizontal geometry of the existing turnpike; i.e. seeking locations at vertical high points on horizontal tangents, and based on physical separation from bridge overpasses and interchanges. The study area was reviewed and locations with engineering constraints (for example, bridges, curves, and interchanges) were mapped. The engineering constraints effectively created "redzones" or "out-of-bound" zones which divided the corridor into candidate segments that offered the potential for meeting basic physical and geometric design criteria of a new plaza. The same design criteria used to evaluate the existing plaza in the ESE, Part 2, were also applied here to identify potential new plaza locations. Figure 3.4 shows the engineering constraints and resultant candidate segments for evaluating placement of a new plaza. Candidate sites were then identified within the candidate segments following the basic design criteria.





Maine Turnpike Authority
Southern Toll Plaza - Phase I Report

Engineering Siting Constraints
Figure 3.4



#### **Natural and Social Resources**

Using 2003 aerial photography and the Maine Office of Geographic Information Systems (Maine OGIS) Data Catalog, hydric soils, National Wetland Inventory (NWI) wetlands, floodplains, streams, and rivers within the study area were mapped. Resources exist throughout the Study Corridor to such a degree that no location would be totally unconstrained by them. However, some areas are less constrained by these resources than others. Density of development is also an important consideration. Planning personnel for the towns of York, Ogunquit, and Wells were consulted to document and confirm existing, planned, and potential future development within the Study Area. The resource information gathered was added to the mapping and is noted on Figure 3.5.

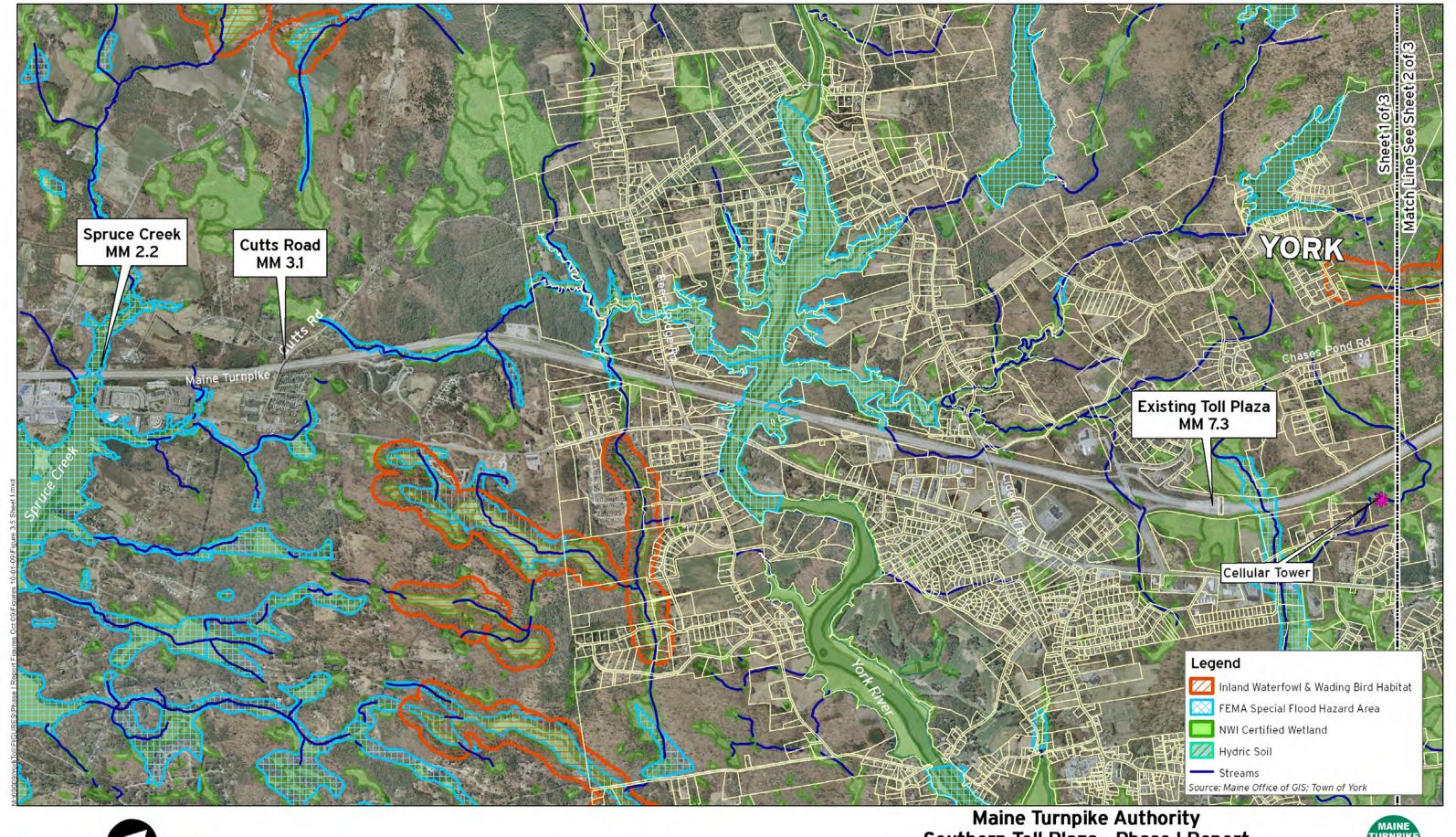
By considering engineering criteria, social and environmental resource information, a full range of Candidate Plaza Locations were identified and shown in Figure 3.6. The following summarizes how those locations, within the previously described segments, were developed considering basic engineering suitability for a new toll plaza, e.g. vertical and horizontal geometry and separation from bridges and interchanges through avoidance and/or minimization of mapped social and environmental resources within the study area. The resulting candidate new toll plaza locations are labeled according to their approximate Mile Marker along the Maine Turnpike, noted from south to north.

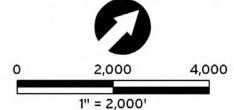
#### Spruce Creek to Chases Pond Road; Mile Marker 2.2 to 6.8 – Kittery & York

This section is constrained by tidal wetlands, hydric soils, NWI wetlands, floodplains, and the tidal York River. Dense commercial and residential development occupies land between US Route 1 and I-95 (Maine Turnpike) near Cutts Road and further south. Two overpasses (Cutts Road and Chases Pond Road), and the Chases Pond Road interchange are constraints in this segment. Figure 3.7, Sheet 1 of 3, provides an aerial view of the Maine Turnpike corridor south of Exit 7. The less-constrained locations in this section would not meet the design criteria for a new toll plaza. Therefore, there are a very limited number of possible locations where a new toll plaza could be considered.

The roadway is relatively flat along this section which also includes curves. The northbound and southbound truck inspection and weigh stations are located in this section along with the Maine Welcome/Visitors" Center. The two locations considered, *Locations 4.5 and 5.4*, are the existing Maine Department of Transportation (MaineDOT) truck inspection and weigh stations southbound and northbound respectively.

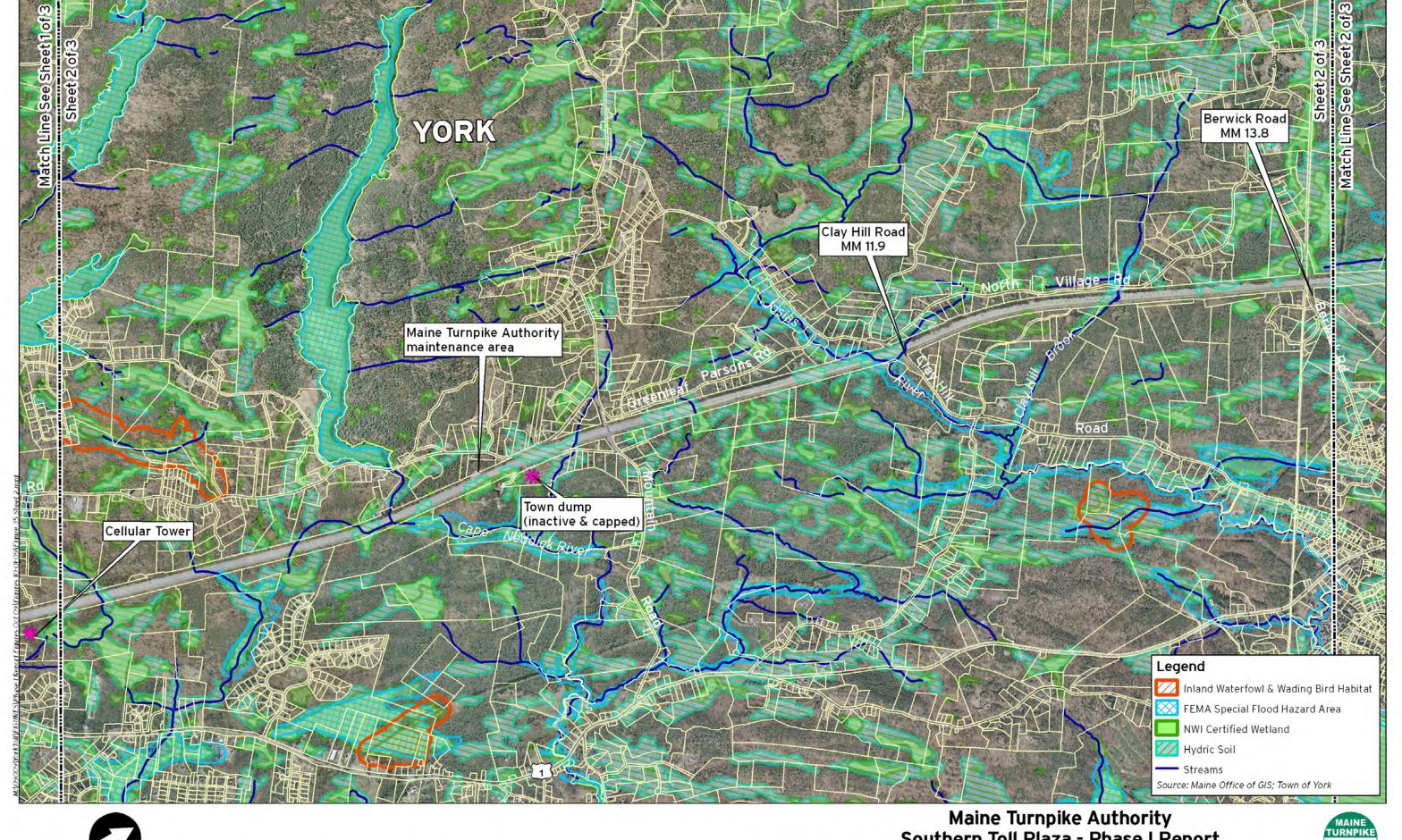
Both weigh stations are vital to the State Police statewide commercial vehicle enforcement effort since they are located at the gateway to Maine's primary transportation corridor. Use of a weigh station location would require finding (and constructing) another suitable replacement weigh station site, which would add to the project impacts. In addition, both weigh stations were recently renovated with the installation of new weigh station monitoring equipment.

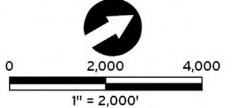




Maine Turnpike Authority
Southern Toll Plaza - Phase I Report
Study Corridor South of Existing Toll Plaza
Figure 3.5
Sheet 1 of 3

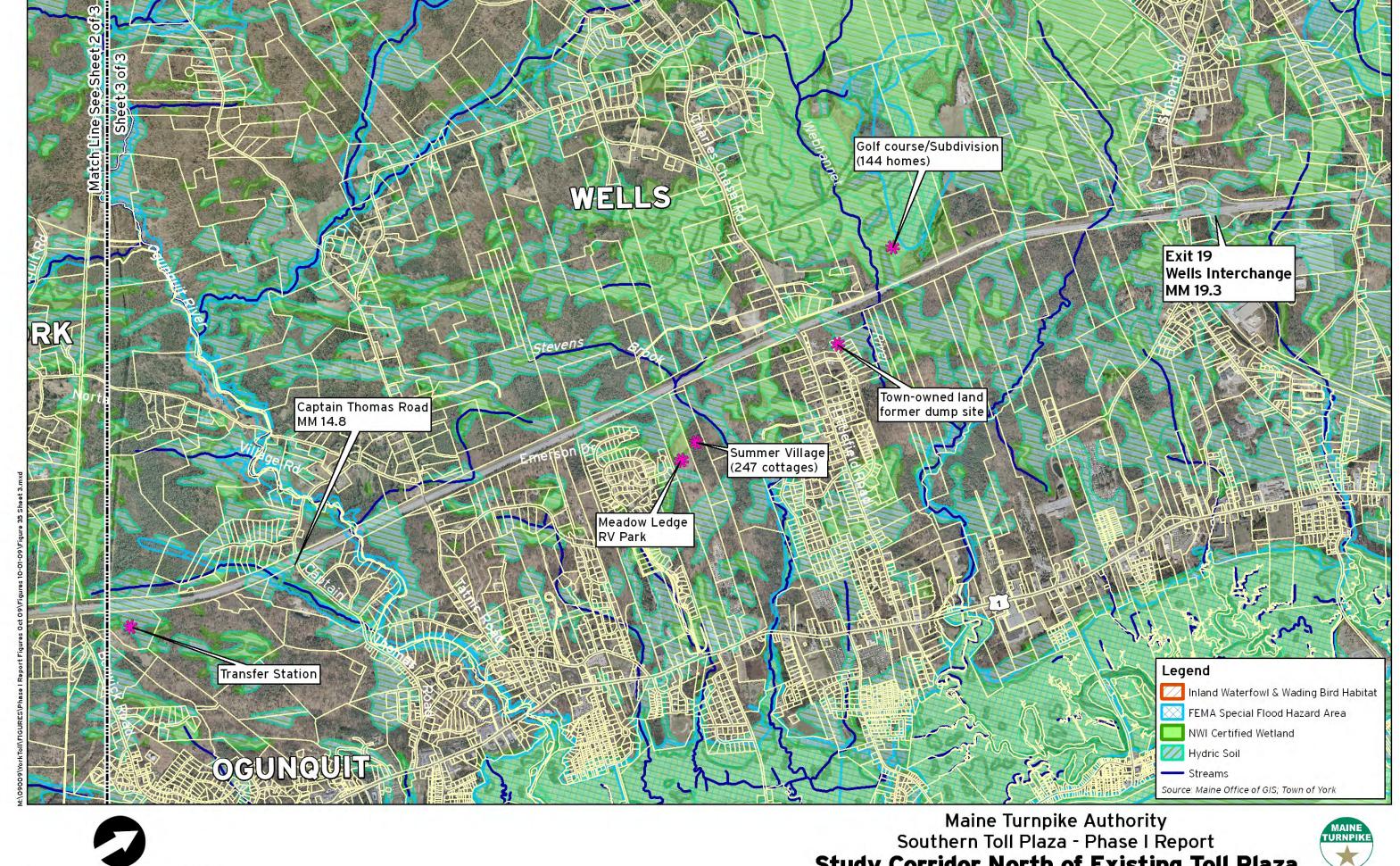


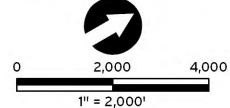




Maine Turnpike Authority
Southern Toll Plaza - Phase I Report
Study Corridor North of Existing Toll Plaza
Figure 3.5
Sheet 2 of 3







Study Corridor North of Existing Toll Plaza
Figure 3.5
Sheet 3 of 3



#### Chases Pond Road to Mountain Road; Mile Marker 6.8 to 10.6 – York

This section (Figure 3.7, Sheet 2 of 3), is relatively unconstrained in comparison to other sections, but hydric soils, NWI wetlands, floodplains, a stream crossing (Cape Neddick River, Class B), and existing development do occur throughout this section of roadway between Chases Pond Road and Mountain Road. Land along the west side of the Maine Turnpike is generally undeveloped, except for a low density of homes along Chases Pond Road, a 3/4 mile section that runs parallel to and immediately adjacent to the Maine Turnpike right-of-way, just south of Mountain Road. There is an existing residential subdivision, known as Whippoorwill, approximately 1/2 mile east of the Maine Turnpike, located approximately between Mile Markers 8.8 and 9.1. A smaller subdivision exists on the east side of the Maine Turnpike, just south of Mountain Road. In general, natural and social resources are relatively scattered along this section, so there are several areas that could potentially accommodate a new toll plaza.

Located at the low point of a hill and on a horizontal curve, the existing York Toll Plaza, *Location 7.3*, is also in close proximity to Chases Pond Road (Exit 7) at Mile Marker 6.8. As detailed in Existing Site Evaluation, Part 2, this location does not satisfy the basic design criteria, does not fully meet the project Purpose and Need, and is not a desirable location for advancement of a new toll plaza because of natural resources and development constraints. However, *Options 4A and 4B* have been advanced as the best possible solutions at the existing location, representing "upgrade alternatives" for comparative purposes.

Seven vertical high points, *Locations 8.1, 8.5, 8.6, 8.7, 8.8, 9.1, and 9.9*, exist along this tangent section of roadway. Locations 8.5, 8.6, 8.7 and 8.8 are located within approximately 1,600 feet of each other.

#### Mountain Road to Clay Hill Road; Mile Marker 10.6 to 11.9 – York

This section (Figure 3.7, Sheet 2 of 3), is somewhat more constrained with natural resources than the section south of Mountain Road. Hydric soils, NWI wetlands, floodplains, and a stream crossing (Josias River, Class B) exist in this section. Existing homes along Greenleaf Parsons Road, while at relatively low density, are within close proximity to the southbound (west) side of the Maine Turnpike right-of-way. Land on the east side of the Maine Turnpike is generally undeveloped. There are several areas that could potentially accommodate a new toll plaza.

Two vertical high points that exist along a tangent section and are separated sufficiently from an overpass are *Locations 11.3 and 11.4*.

#### Clay Hill Road to North Berwick Road; Mile Marker 11.9 to 13.8 - York & Ogunquit

Natural resources along this section (Figure 3.7, Sheet 2 and Sheet 3 of 3), of the Maine Turnpike are relatively sparse. Isolated pockets of NWI wetlands, hydric soils, and floodplains exist on both sides of the Maine Turnpike. Clay Hill Brook, Class B, crosses under the Maine Turnpike approximately at Mile Marker 13. North Village Road generally runs parallel to and within a few hundred feet of the southbound (west) side of the Maine Turnpike right-of-way. The northbound (east) side of the Maine Turnpike is generally undeveloped except in the immediate vicinity of North Berwick Road. Based on these constraints and the presence of homes along North Village Road, potential new toll plaza sites would be limited to the area north of Mile Marker 13.

One vertical high point, *Location 13.2*, exists along this tangent section of roadway.

#### North Berwick Road to Captain Thomas Road; Mile Marker 13.8 to 14.8 - Ogunquit

Extensive areas of hydric soils, NWI wetlands, and floodplains exist along this section (Figure 3.7, Sheet 3 of 3) of the Maine Turnpike. The southerly tributary to the Ogunquit River, Class A, crosses the Maine Turnpike at approximately Mile Marker 14.7. Areas of residential development, particularly along the west side, exist in close proximity to the Maine Turnpike. Based on these constraints, this section is considered severely constrained and unsuitable for placing a new toll plaza.

This section of roadway has a curved horizontal alignment with no vertical high points, therefore, it would not accommodate a new toll plaza that satisfies these design criteria.

#### Captain Thomas Road to Tatnic Road; Mile Marker 14.8 to 15.2 – Ogunquit & Wells

Extensive areas of hydric soils, NWI wetlands, and floodplains, and the crossing of the Ogunquit River, Class A, along with pockets of existing residential development in close proximity to both sides of the Maine Turnpike right-of-way, render this section (Figure 3.7, Sheet 3 of 3) unsuitable for a new toll plaza. At less than one mile in length, this section of roadway would not accommodate a new toll plaza that satisfies these design criteria. Further, the presence of the Ogunquit River crossing midway between these two bridges would present a significant environmental constraint.

#### Tatnic Road to Littlefield Road; Mile Marker 15.2 to 17.3 – Wells

This section (Figure 3.7, Sheet 3 of 3) is constrained by areas of hydric soils and NWI wetlands on both sides of the Maine Turnpike, a stream crossing (Stevens Brook, Class B), and areas of existing residential development, primarily on the east side of the Maine Turnpike. An RV Park and a 247 cottage subdivision are under development on the east side of the Maine Turnpike, increasing the density of residential development on the east side of the Maine Turnpike.

Three vertical high points, *Locations 15.8*, 16.5, and 16.9, exist along this tangent section of roadway.

#### Littlefield Road to Wells Interchange Mile Marker 17.3 to 19.3 – Wells

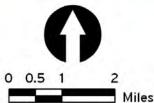
This section (Figure 3.7, Sheet 3 of 3) of the Maine Turnpike has several areas that are constrained by large areas of hydric soils, NWI wetlands, and stream crossings of Webhannet River (Class A) and Crediford Brook (Class B). Land along both sides of the Maine Turnpike is generally undeveloped, except for a recently constructed golf course (Old Marsh) with a proposed 144 home subdivision on the west side of the Maine Turnpike in the area of Mile Marker 18. Based on these constraints, this section of the Maine Turnpike provides limited opportunities for the location of a new toll plaza.

One vertical high point, *Location 17.7*, exists north of Littlefield Road and south of the single horizontal curve along this section of roadway.

In total, 16 alternate locations were identified that could be considered for new toll plaza locations using basic design criteria. The 16 alternate locations along with the two options at the existing plaza location represent the Phase I study alternatives following the U.S. Army Corps of Engineers Highway Methodology. The existing toll plaza options with upgrades to open road tolling (Options 4A & 4B) do not meet the established project Purpose and Need or all of the basic design criteria, but were carried forward, as the best possible options from the Existing Site Evaluation, for comparative evaluation with other new site locations using the same level of assessment. The intent is to give every reasonable opportunity to consider the existing location and test the likely impacts at the same level of analyses as alternate locations.

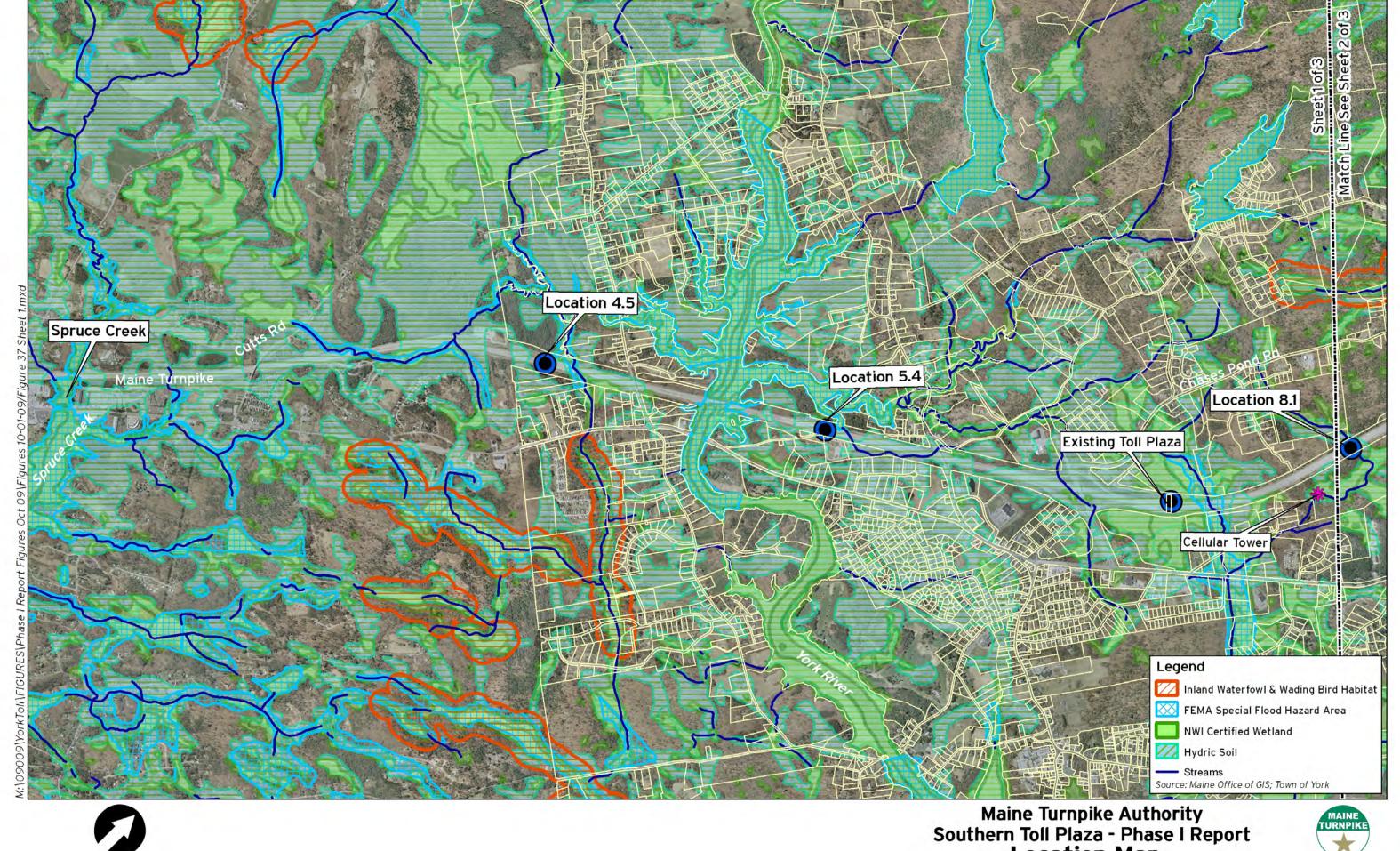
The 16 initial engineering locations as well as the two existing location options are shown in Figure 3.6. They also are shown in Figure 3.7 along with file-level natural resource information obtained through Maine OGIS and the municipalities.





Maine Turnpike Authority
Southern Toll Plaza - Phase I Report
Candidate Toll Plaza Locations
Figure 3.6





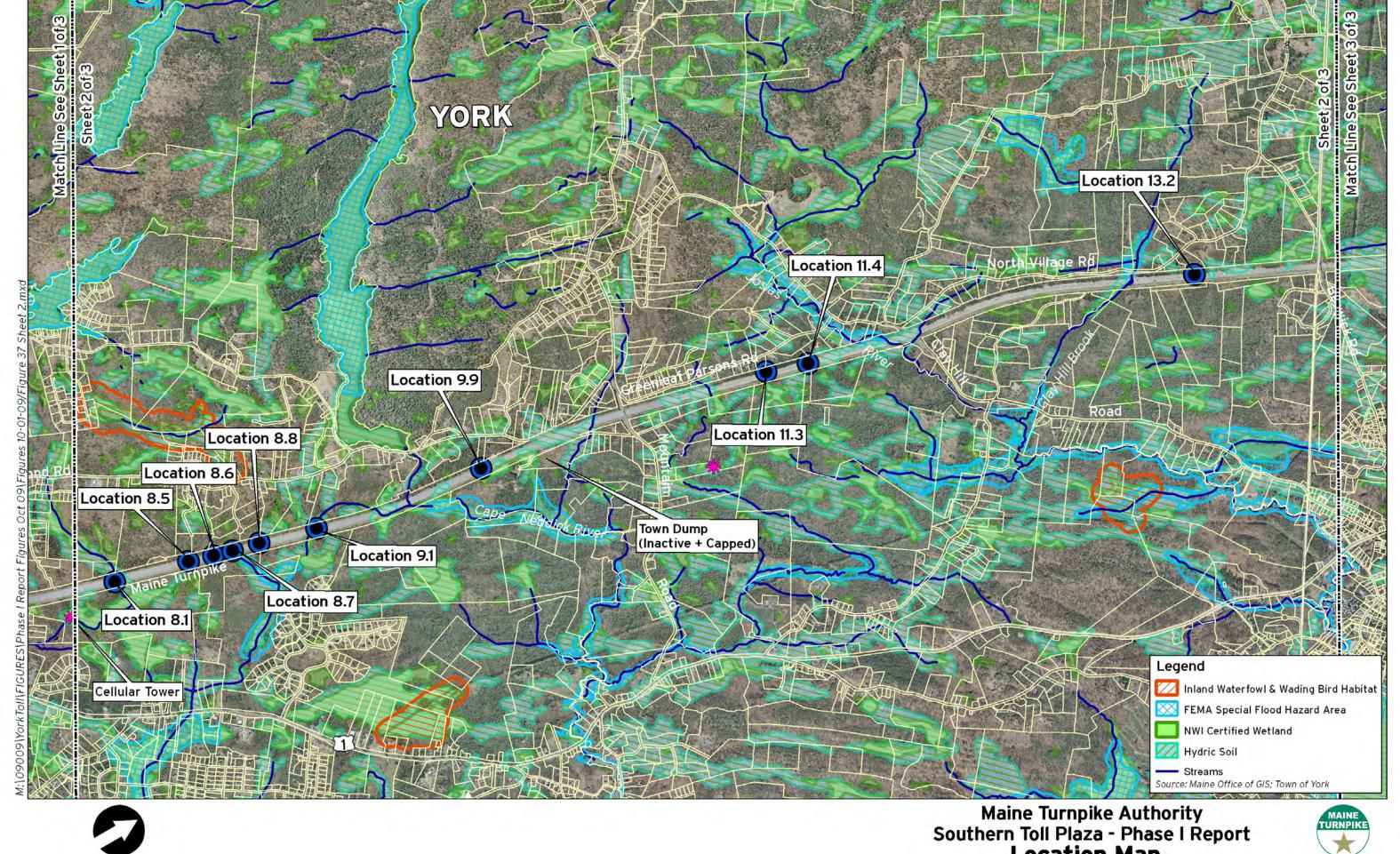
4,000

2,000

1" = 2,000"

Maine Turnpike Authority Southern Toll Plaza - Phase I Report **Location Map** Figure 3.7 Sheet 1 of 3





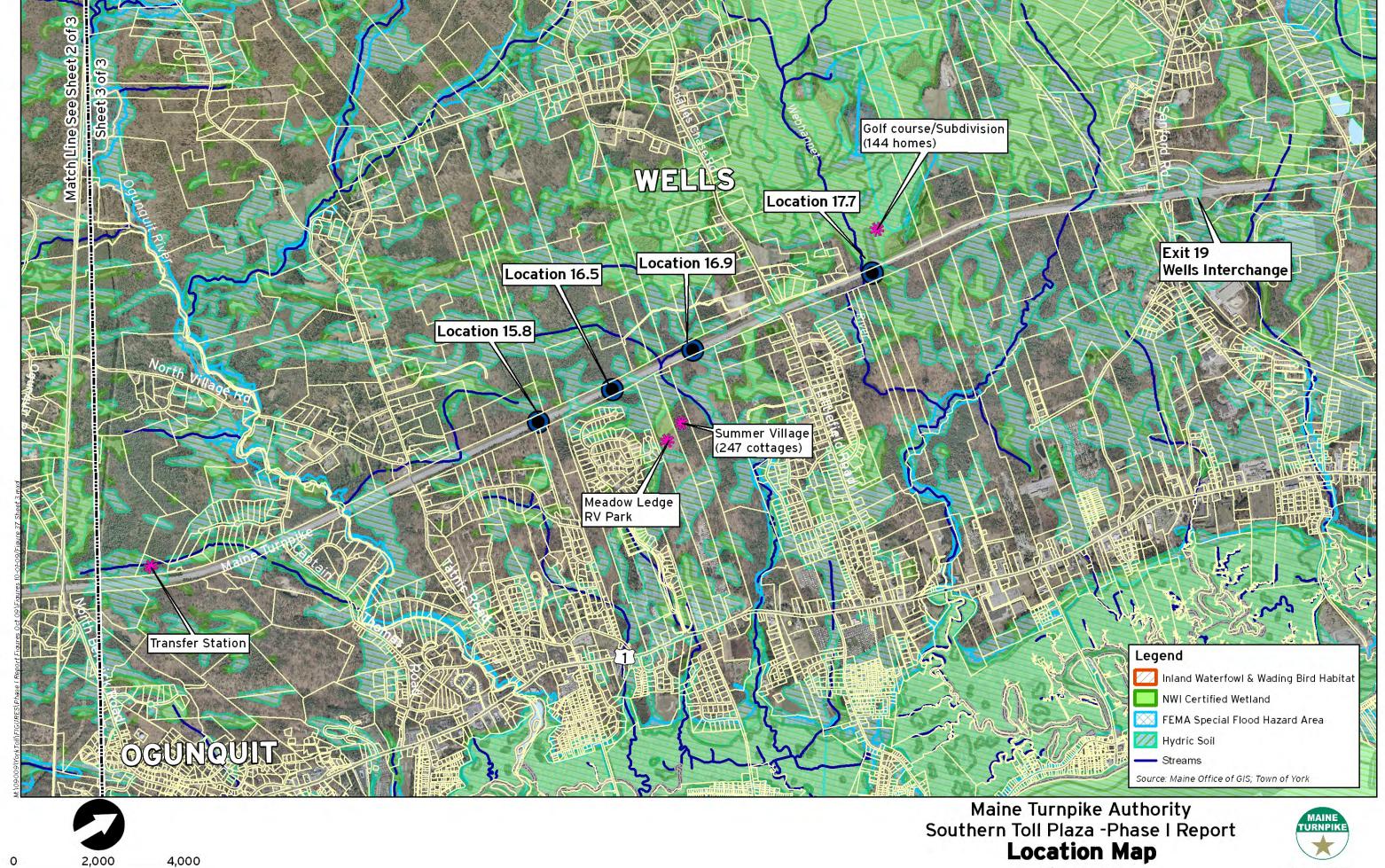
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1" = 2,000"

Maine Turnpike Authority
Southern Toll Plaza - Phase I Report
Location Map
Figure 3.7
Sheet 2 of 3





1" = 2,000

Figure 3.7 Sheet 3 of 3



# PART 4 SITE SCREENING

# SECTION 1 - EXISTING AND ALTERNATE SITE COMPARATIVE SCREENING

Continuing to follow to the USACE"s Highway Methodology, Part 4 – Site Screening, is the final step of the Phase 1 investigation. The goal of the site screening is to develop a shortlist of sites that, when compared with others meet the Project Purpose and Need, are less environmentally damaging, and are more practicable than the other potential options and locations. The resulting shortlist of options and/or sites will then be recommended for further evaluation as part of Phase II of the Highway Methodology.

Following, the 16 alternate locations that passed the initial location screening based upon basic design criteria, along with the no-build option and existing plaza upgrades Options 4A and 4, are reviewed against a series of natural and social resources and constraint maps. The following resources and factors are some of those considered in the site screening along with the engineering location considerations. They are not presented in any particular order of importance or weight in the evaluations.

- Right-of-way
- Potential home displacements
- Proximity to homes and subdivisions
- National Wetland Inventory
- Wetland Soils (i.e., hydric soils)
- Streams
- FEMA 100 year Floodplains

Enlarged illustrations overlaid on resource information, and including a conceptual-level toll plaza footprint are included as Figures 4.1 through 4.18.

Available information was used to further evaluate the conceptual locations using Geographic Information System (GIS) methods for quantitative assessments, and interpretive methods for qualitative considerations. Using a preliminary design footprint, the candidate sites were then evaluated against screening criteria to determine potential direct impacts using both quantitative and qualitative evaluation methods. The findings are considered in conjunction with the initial engineering site screening to help select less-damaging and practicable alternatives. It should be noted that resources used in the initial assessment were adjusted for overlap with the existing Maine Turnpike. For example, wetland soils shown overlapping the roadway were not counted where pavement clearly exists already.

Table 4.1 is an evaluation matrix of the sites with both quantified impacts and qualitative comments. For each resource category, the impacts were assigned a relative rating by determining the total range and dividing equally into three groups. The relative rating is then shown by color to help visualize and show trends when comparing locations and when comparing dissimilar resources. In the table, the least impact range is green, and the most impact range is orange, with yellow representing the middle range. From the table, the alternate sites

can be compared both individually by resource, and collectively between resources, which helps determine sites to investigate further and sites that should be dismissed. From the refined evaluation considering social resources, natural resources, and engineering, the following conclusions were reached.

#### Spruce Creek to Chases Pond Road; Mile Marker 2.2 to 6.8 – Kittery & York

Locations 4.5 and 5.4 - Both locations (Figure 4.1 and Figure 4.2) are south of Chases Pond Road. Location 4.5 does not meet the basic design criteria due to the presence of a horizontal curve, and has extensive wetland impacts including coastal wetlands, and is in close proximity to higher density residential development. Location 4.5 is more environmentally damaging than other locations and does not satisfy the engineering geometry. Location 5.4 has similar natural resource impacts involving coastal wetlands, but meets the basic design criteria. Both locations would impact the state police truck inspection and weigh stations and would require replacement of those operations as well as building the new toll plaza. Co-locating a new southern toll plaza at one or both of the existing weigh stations, would present significant traffic flow and safety concerns. At issue is the inability to develop a design that would safely and efficiently segregate the traffic streams that would consist of mainline highway speed autos, cash-paying autos through a conventional toll plaza, and trucks that would be required to stop at the weigh station. Trucks and cash-paying autos would exit the turnpike and would then need to be separated again to the weigh station or to the conventional toll plaza. These designs would require significantly more land than the current weigh stations occupy. The number of driver decision points and the high concentration of trucks mixed with the general traffic in the area of the conventional toll plaza create significant safety concerns. Further, truckers, who comprise a large portion of ETC users, would lose the time savings of ETC, since they would be required to use the conventional toll plaza before or after stopping at the weigh station. Project costs for these locations, considering potential impacts to wetlands, homes and right-of-way are estimated to be \$40-\$41 million and this does not include the cost to relocate the state police weigh stations.

Therefore, because these alternate locations are not practicable, and considering that other alternates are less environmentally damaging and satisfy the tolling strategy, both Location 4.5 and 5.4 are not recommended to be carried forward for further evaluation.

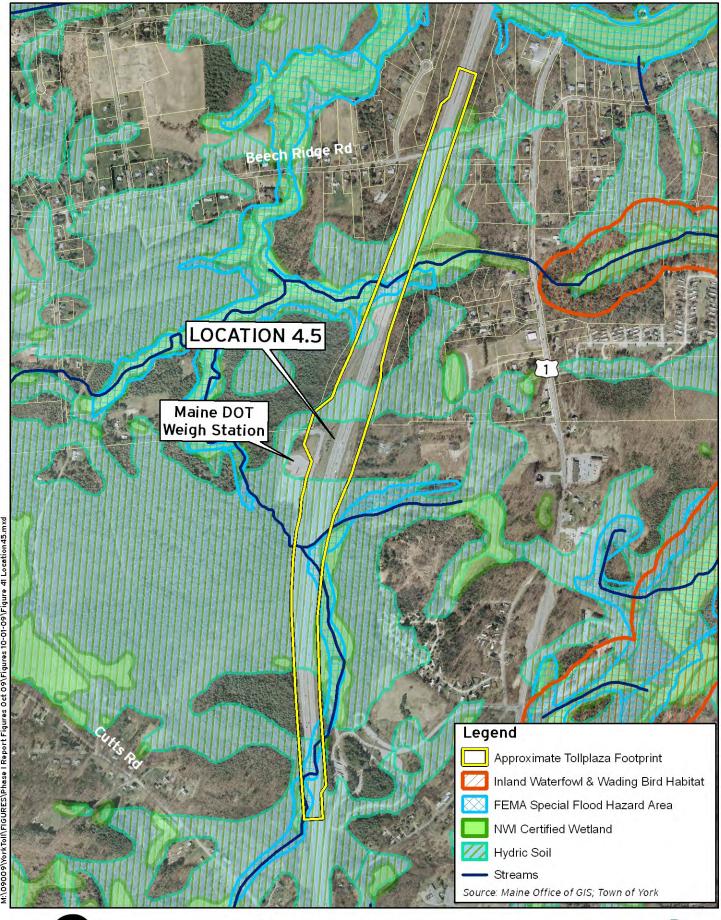
Prince   P		1	2	3	4	5	6	7	8	9	10	11	12	13
		Engineering Criteria								Natural Res	ource & Built Environr	nent Impacts		
Control of	Location\Evaluation Parameter	Horizontal Alignment	Vertical Alignment		Overhead Structure	Sight Distance	· ·	-	Impacts - National Wetland Inventory	Impacts - Natural Resource Conservation Services	Impacts - Maine OGIS (LF)	Impacts - Federal Emergency Management Agency	Displacements	Homes Within 1000 f (Homes)
Location Inf   Configuration   A Constraint   Yes   Yes   Oxcord discuss   No   2.3   3.6   12.1   711   3.2   2	SPRUCE CREEK													
Control Cont	Location 4.5 <sup>1</sup>		At Crest of Hill	Yes	Yes	Good, both directions	NO	0.8	1.8	18.6	958	2.4	0	28
Option   Country   Count	Location 5.4 <sup>1</sup>		At Crest of Hill	Yes	Yes	Good, both directions	NO	6.3	3.0	17.1	711	3.2	2	27
Control   Cont	EXISTING LOCATION													
Steelth Highway Specificality   Steelth   St	Option 1 (Existing Site, No Build)		NOT At Crest of Hill	No	No	Poor, both directions	NO	0.0	0.0	0.0	0	0.0	0	5
Street History Specification   Street Training   Street History Specification   Act Creat Field   Vision   Vision   Code, test Precision   Code, test Preci			NOT At Crest of Hill		No	Poor, both directions	MARGINAL	8.1	8.8	28.0	729	4.3	0	41
Location 8.1	Site with Highway Speed Tolling)		NOT At Crest of Hill	No	No	Poor, both directions	NO	3.3	4.9	22.2	509	2.8	0	32
Location 8.6   On Straying Section   ALC rest of Hill   Yes   Yes   Coop both directions   YES   6.9   1.6   1.4   1.0   0.5   0.5		On Straight Section	At Crest of Hill <sup>2</sup>	Yes	Yes	Good, both directions	MARGINAL	7.3	0.5	5.7	662	1.2	0	8
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Location 9:1	Location 8.7	On Straight Section	At Crest of Hill	Yes	Yes	Good, both directions	YES	7.0	2.4	1.7	939	0.5	0	6
Location 19.9   On Straight Section   At Crist of Hill   Viss   Viss   Good, both directions   VES   0.4   3.4   6.5   316   0.0   2	Location 8.8	On Straight Section	At Crest of Hill	Yes	Yes	Good, both directions	YES	7.2	1.7	2.7	1487	0.4	0	12
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Location 11.4 On Straight Section At Crest of Hill Yes Yes Good, both directions YES 8.7 0.2 11.0 667 0.6 1  CLAY HILL ROAD Location 13.2 On Straight Section At Crest of Hill Yes Yes Good, both directions YES 11.1 0.1 3.9 160 Negligible 2  TATNIC ROAD Location 15.8 On Straight Section At Crest of Hill Warginal, would require future barrier separated ramps Location 16.6 On Straight Section At Crest of Hill Marginal, would require future barrier separated ramps Location 16.6 On Straight Section At Crest of Hill Marginal, would require future barrier separated ramps Location 16.6 On Straight Section At Crest of Hill Marginal, would require future barrier separated ramps Location 16.6 On Straight Section At Crest of Hill Marginal, would require future barrier separated ramps Location 16.6 On Straight Section At Crest of Hill Nove Yes Good, both directions MARGINAL 13.9 1.0 7.6 576 0.4 0.7 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0														
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Location 13.2 On Straight Section At Crest of Hill Yes Yes Good, both directions YES 11.1 0.1 3.9 160 Negligible 2  TATNIC ROAD Location 15.8 <sup>4</sup> On Straight Section At Crest of Hill Require future barrier separated ramps Location 16.5 <sup>4</sup> On Straight Section At Crest of Hill Require future barrier separated ramps Location 16.9 <sup>4</sup> On Straight Section At Crest of Hill Marginal, would require future barrier separated ramps Location 16.9 <sup>4</sup> On Straight Section At Crest of Hill Marginal, would require future barrier separated ramps Littrice field ROAD Location 17.7 Not On Straight Section Not On Straight At Crest of Hill No Yes Good, both directions MARGINAL 13.5 2.9 9.7 1095 3.3 0  Littrice field ROAD Location 17.7 Not On Straight Section Sectio	Location 11.4	On Straight Section	At Crest of Hill	Yes	Yes	Good, both directions	YES	8.7	0.2	11.0	667	0.6	1	38
TATNIC ROAD Location 15.8 <sup>4</sup> On Straight Section Location 16.5 <sup>4</sup> On Straight Section At Crest of Hill Arginal, would require future barrier separated ramps Location 16.5 <sup>4</sup> On Straight Section At Crest of Hill Arginal, would require future barrier separated ramps  Location 16.5 <sup>4</sup> On Straight Section At Crest of Hill Arginal, would require future barrier separated ramps  Location 16.5 <sup>4</sup> On Straight Section At Crest of Hill Arginal, would require future barrier separated ramps  Location 16.5 <sup>4</sup> On Straight Section At Crest of Hill Arginal, would require future barrier separated ramps  LittleFIELD ROAD Location 17.7 Not On Straight Section  Not On Straight At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill At Crest of Hill At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill At Crest of Hill At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill At Crest of Hill At Crest of Hill No Yes Good, both directions MARGINAL At Crest of Hill At Crest of Hill At Crest of Hill No Yes Good, both directions M														
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Location 16.5 <sup>4</sup> On Straight Section At Crest of Hill Marginal, would require future barrier separated ramps  Location 16.9 <sup>4</sup> On Straight Section At Crest of Hill Marginal, would require future barrier separated ramps  LittleFIELD ROAD  LittleFIELD ROAD  Location 17.7 Not On Straight Section At Crest of Hill No Yes Good, both directions MARGINAL  Location 17.7 Not On Straight At Crest of Hill No Yes Good, both directions MARGINAL  Location 17.7 Not On Straight Section At Crest of Hill No Yes Good, both directions MARGINAL  Location 17.7 Not On Straight Section Not On Straigh	TATNIC ROAD													
Location 16.9 <sup>4</sup> On Straight Section At Crest of Hill Marginal, would require future barrier separated ramps  LITTLEFIELD ROAD  Location 17.7 Not On Straight Section At Crest of Hill No Yes Good, both directions MARGINAL 22.3 4.2 7.0 466 0.0 0  WELLS INTERCHANGE  Footnotes:  Low-Range of impacts 0-7.4 0-2.9 0-9.3 0-527 0-1.4 0  Location would change tolling structure (plaza south of exit 7). New weight station Middle-Range of impacts 7.5-14.8 3.0-5.8 9.4-18.6 528-1054 1.5-2.8 0	Location 15.8 <sup>4</sup>	On Straight Section	At Crest of Hill	require future barrier		Good, both directions	MARGINAL	11.6	0.4	5.3	593	0.1	0	26
LITTLEFIELD ROAD  Location 17.7 Not On Straight Section  WELLS INTERCHANGE  Footnotes:  Location would change tolling structure (plaza south of exit 7). New weight station  Location 17.8 New weight station  Location 17.7 Not On Straight Section  Not On Straight At Crest of Hill No Yes Good, both directions MARGINAL 22.3 4.2 7.0 466 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0	Location 16.5 <sup>4</sup>	On Straight Section	At Crest of Hill	require future barrier	Yes	Good, both directions	MARGINAL	13.9	1.0	7.6	576	0.4	0	18
Location 17.7 Not On Straight At Crest of Hill No Yes Good, both directions MARGINAL 22.3 4.2 7.0 466 0.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Location 16.9 <sup>4</sup>	On Straight Section	At Crest of Hill	require future barrier	Yes	Good, both directions	MARGINAL	13.5	2.9	9.7	1095	3.3	0	12
WELLS INTERCHANGE         Low-Range of impacts         0-7.4         0-2.9         0-9.3         0-527         0-1.4         0           1. Location would change tolling structure (plaza south of exit 7). New weight station         Middle-Range of impacts         7.5-14.8         3.0-5.8         9.4-18.6         528-1054         1.5-2.8         0														
Footnotes: Low-Range of impacts 0-7.4 0-2.9 0-9.3 0-527 0-1.4 0  1. Location would change tolling structure (plaza south of exit 7). New weight station Middle-Range of impacts 7.5-14.8 3.0-5.8 9.4-18.6 528-1054 1.5-2.8 0			At Crest of Hill	No	Yes	Good, both directions	MARGINAL	22.3	4.2	7.0	466	0.0	0	5
1. Location would change tolling structure (plaza south of exit 7). New weight station Middle-Range of impacts 7.5-14.8 3.0-5.8 9.4-18.6 528-1054 1.5-2.8 0														
	Footnotes:		,	,,										0-17
required to replace displaced weight station. Additional environmental impacts for High-Range of impacts >14.8 >5.8 >18.6 >10.54 >2.8 >0									3.0-5.8 >5.8	9.4-18.6 >18.6	528-1054 >1054	1.5-2.8 >2.8	>0	18-29 >29

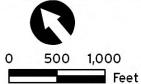
<sup>1.</sup> Location would change tolling structure (plaza south of exit /). New weight station required to replace displaced weight station. Additional environmental impacts for new weigh station likely but not estimated here.

2. Vertical grade excessive at toll plaza.

3. Taking of any homes is considered a "high-range of impact"

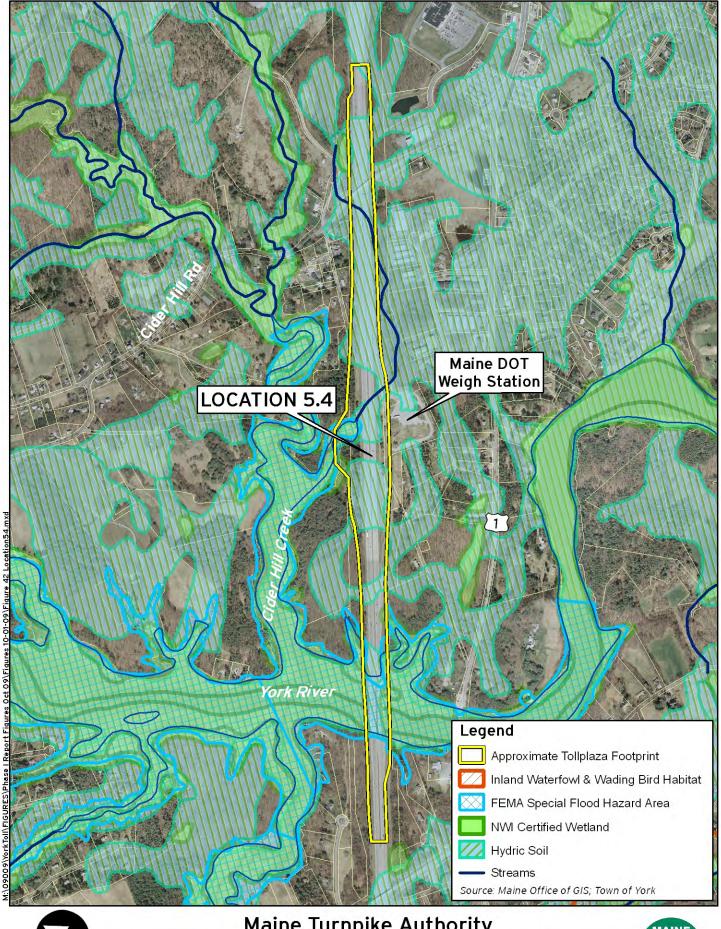
4. Barrier separated ramps to accomodate an interchange would require additional environmental and social impacts. Additional impacts not estimated here.

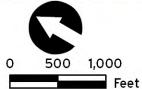




Maine Turnpike Authority Southern Toll Plaza - Phase I Report **Location 4.5** Figure 4.1







Maine Turnpike Authority Southern Toll Plaza - Phase I Report **Location 5.4** Figure 4.2



# Existing Location Mile Marker 7.3 – York (Option 4A and Option 4B)

Located at the low point of a hill and on a horizontal curve, *Location 7.3*, (upgrade Options 4A and 4B) is in close proximity to Chases Pond Road (Exit 7) Mile Marker 6.8 (Figure 4.3 and 4.4 respectively). These options, at Location 7.3, do not accommodate a toll plaza that satisfies the basic design criteria, (too close to an interchange, near non-existent sight distance, and at the bottom of a steep hill) and these upgrade options would have the greatest wetland and floodplain impacts compared with other potential alternate locations. Although no homes would be displaced, this location is in proximity to more homes than any of the other potential alternate locations. Project costs, as detailed in the Existing Site Evaluation, are estimated at \$56 and \$43 million for 4A and 4B respectively. Of the two options near the existing toll plaza location that were carried forward from Part 2, Option 4A is better from an operational perspective than Option 4B as it partially meets one of the basic design criteria. Therefore, Location 7.3 – Option 4B is recommended to be dismissed from further consideration and that only Location 7.3 – Option 4A be carried forward for further evaluation as the best of the upgrade options. More detailed analysis of the existing site options, including a no-build option, can be found in Part 2 – Existing Site Evaluation.

# Chases Pond Road to Mountain Road; Mile Marker 6.8 to 10.6 - York

Seven vertical high points, *Locations 8.1, 8.5, 8.6, 8.7, 8.8, 9.1, and 9.9*, are located along this tangent section of roadway. All seven of these locations (Figures 4.5 to 4.11) meet the basic design criteria except for Location 8.1, which would require an excessive vertical approach grade.

Locations 8.1, 8.6, 8.7, 8.8, and 9.1 would displace no homes and have the fewest number of homes within 1,000 feet of the Maine Turnpike; i.e. a range of 6-12 homes versus locations with 18 to 41 homes. Location 8.5, while also having few homes within 1,000 feet of the Maine Turnpike, would displace two homes.

Locations 8.8, 9.1, and 9.9 in the northern end of this section would generally have higher wetland and stream impacts than locations at the southern end of this section of the Maine Turnpike (Locations 8.5, 8.6, and 8.7), excluding Location 8.1. Further, wetland and stream impacts for Locations 8.5, 8.6, and 8.7 are lower than or comparable to all other locations in the Study Area.

Project costs for Locations 8.1 to 9.9 are relatively uniform and estimated at \$34 to \$37 million. The lower costs are estimated for Locations 8.6, 8.7, 8.8 and 9.1 due to lower combined wetland and right-of-way impacts.

#### Summary:

Location 8.1 is more environmentally damaging than other potential locations (higher wetland impacts) and having an unacceptable vertical grade is considered not practicable and not recommended to be carried forward for further evaluation.

Location 8.5 is generally less environmentally damaging (lower wetland and stream impacts) than other potential locations in this section. However, it has the highest right-

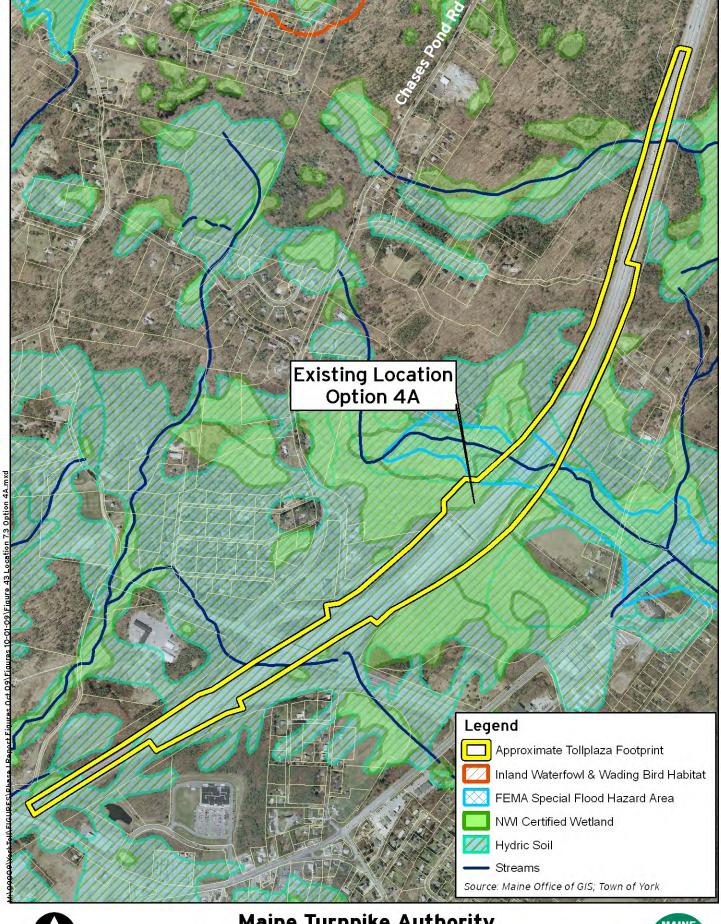
of-way and home displacements in this section and is therefore considered not practicable and **not recommended to be carried forward for further evaluation.** 

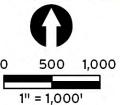
Locations 8.6, 8.7, and 8.8, are located within approximately 1,000 feet of each other. They are less environmentally damaging than most other alternates in the Study Area and would have no home displacements. Also, considering the proximity to minimal homes and a minimal amount of right-of-way impacts a location near 8.7 is a reasonable candidate to carry forward. For purposes of this screening, Location 8.7 is the best representative of these three locations and is recommended to be carried forward for further evaluation.

Although its" potential stream impacts are in the high range compared to other locations, Location 9.1"s wetland, right-of-way, and proximity impacts to homes are all in the low range of the alternates. In addition, Location 9.1 has no home displacements. Therefore, Location 9.1 is recommended to be carried forward for further evaluation.

Location 9.9 would displace two homes, is in closer proximity to more residences and has the highest wetland impacts among the seven potential locations evaluated in this section. Therefore, Location 9.9 is determined to be not practicable for its displacement of two homes and more environmentally damaging than other potential locations (higher wetland impacts) and is **not recommended to be carried forward for further evaluation**.

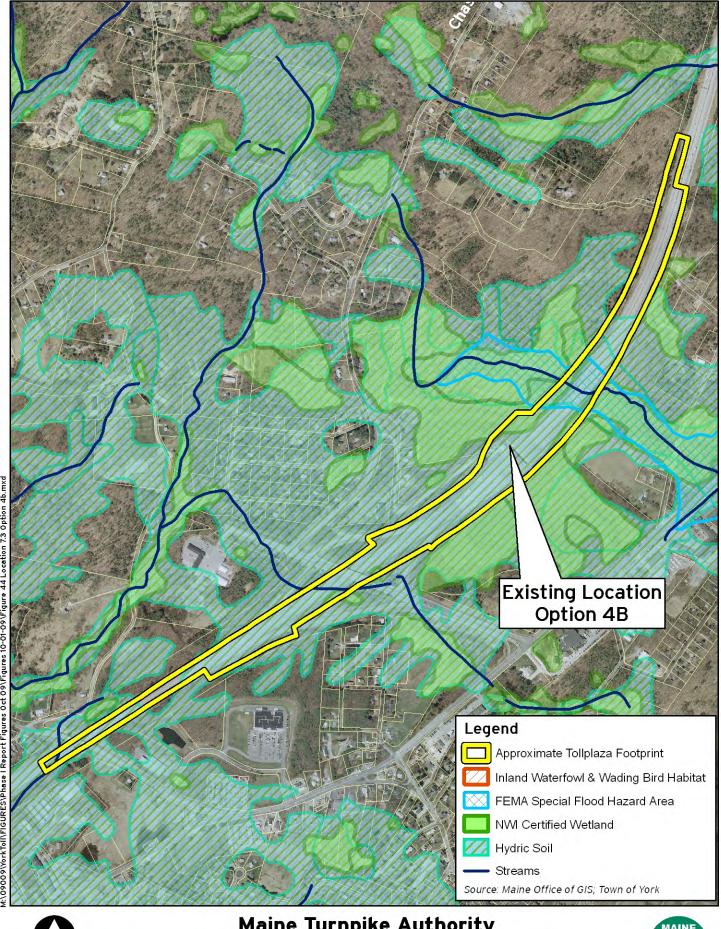
Summarizing the Chases Pond Road to Mountain Road findings; Locations 8.7 and 9.1 are recommended to be carried forward for further evaluation.

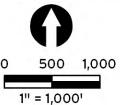




Maine Turnpike Authority Southern Toll Plaza - Phase I Report **Location 7.3 Option 4A** Figure 4.3

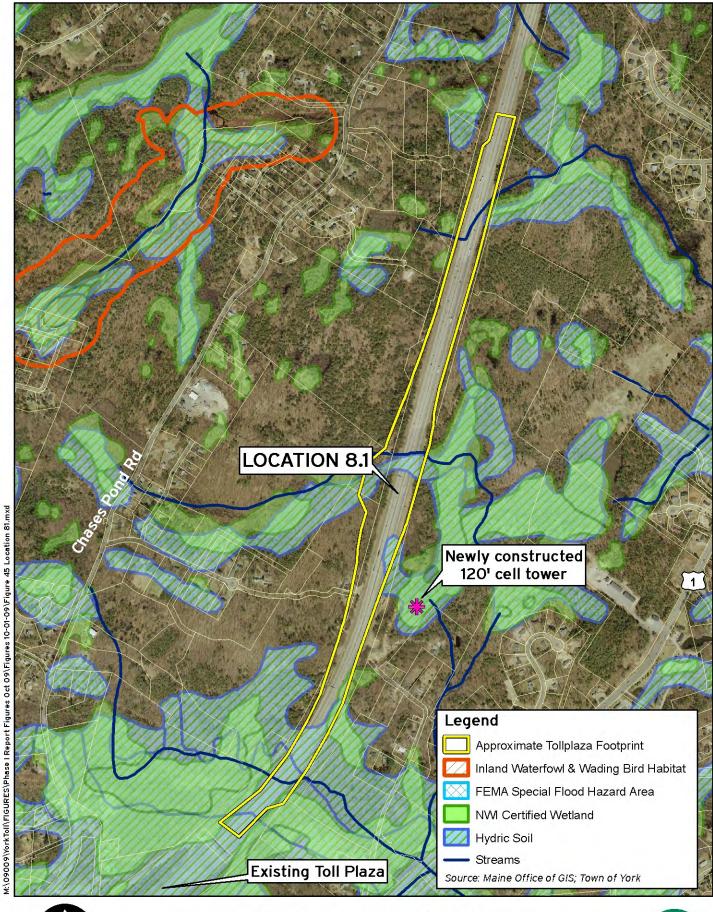


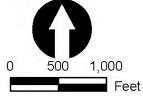




Maine Turnpike Authority
Southern Toll Plaza - Phase I Report
Location 7.3 Option 4B
Figure 4.4

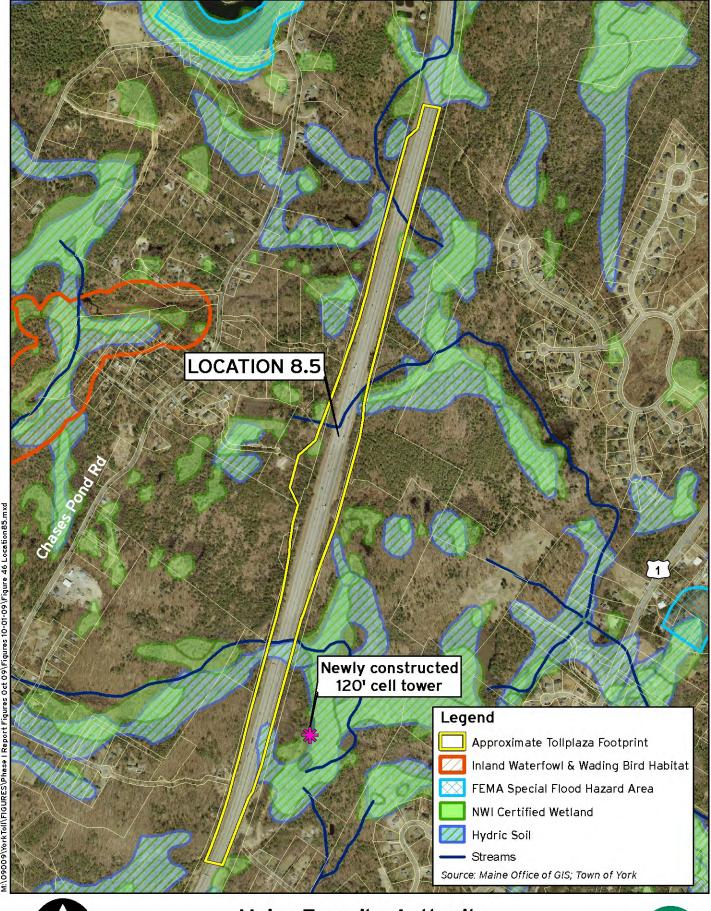


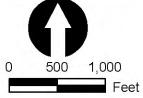




Maine Turnpike Authority Southern Toll Plaza - Phase I Report **Location 8.1** Figure 4.5

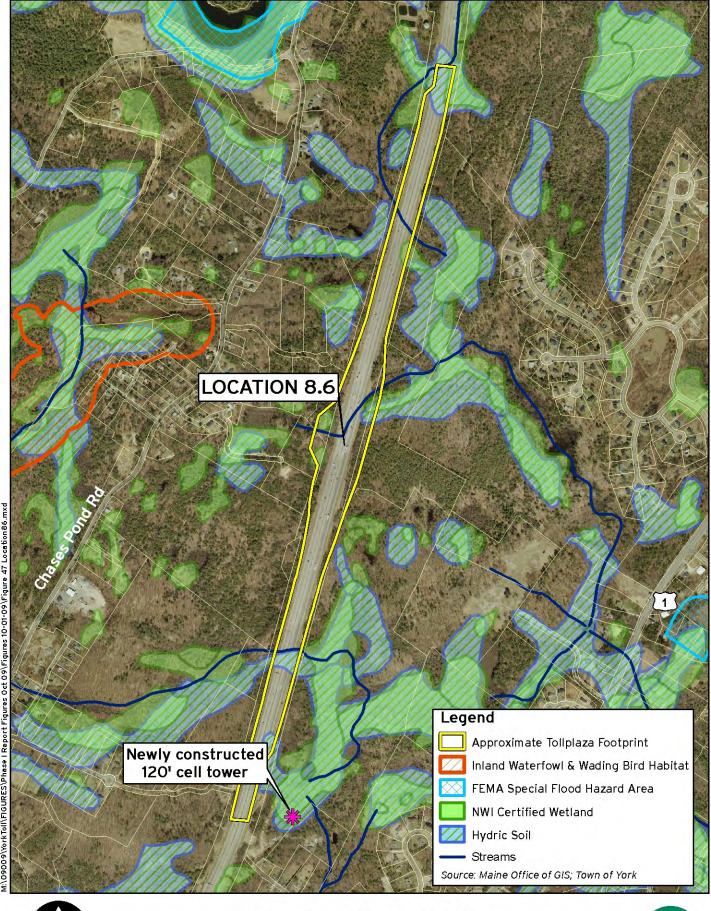


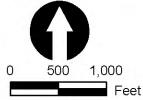




**Location 8.5** Figure 4.6

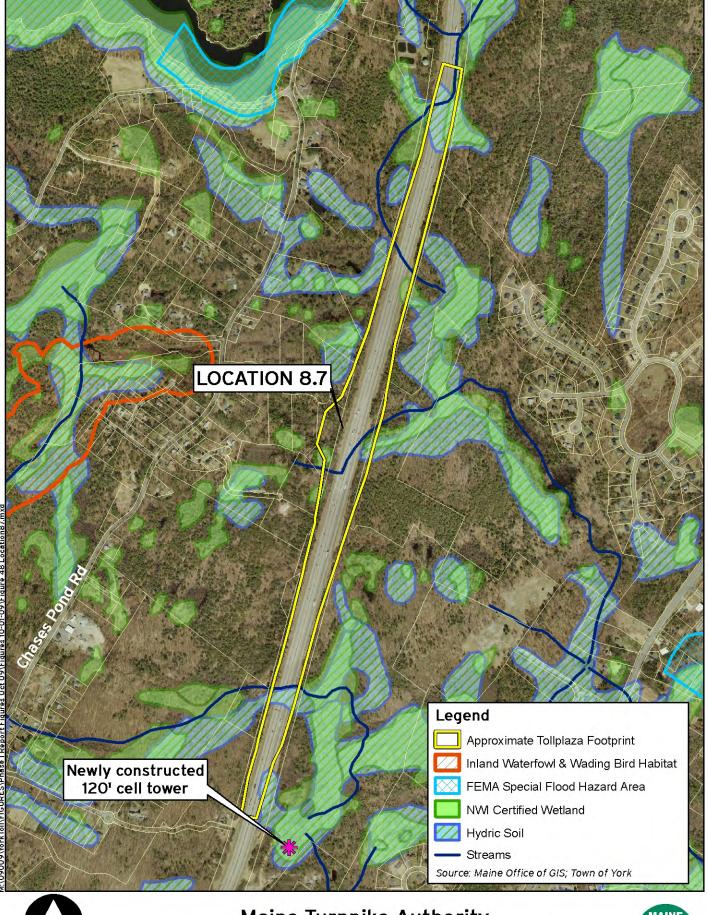


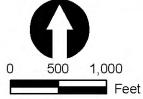




Maine Turnpike Authority Southern Toll Plaza - Phase I Report **Location 8.6** Figure 4.7

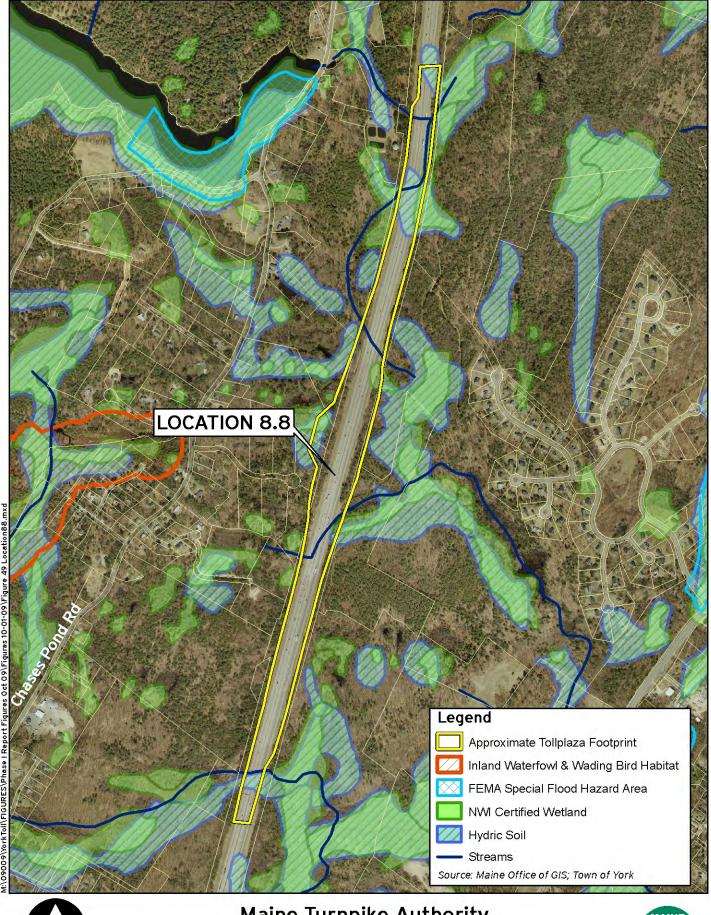


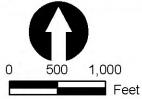




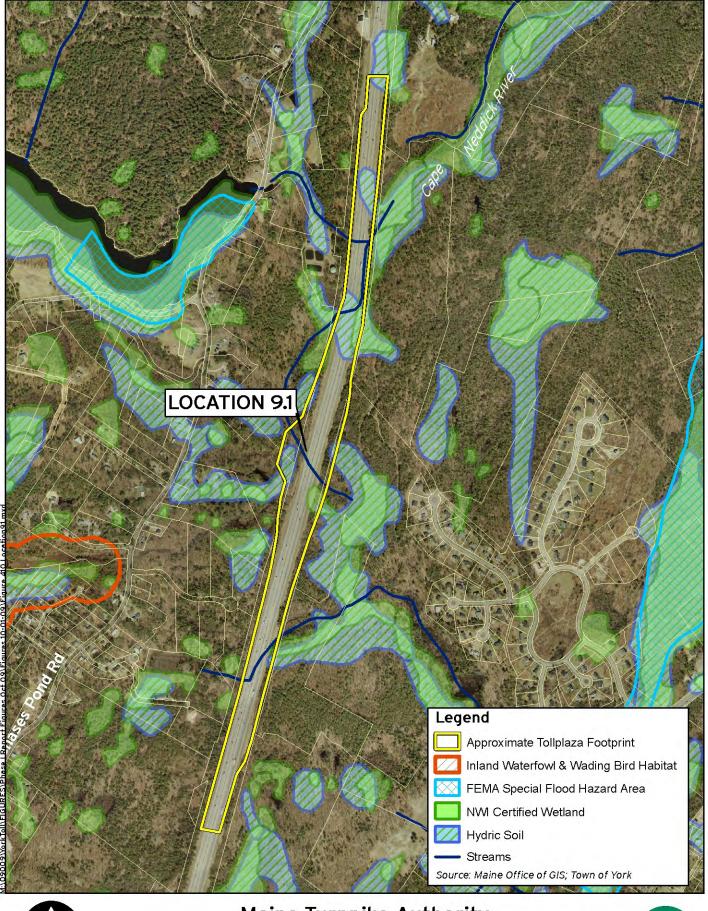
Maine Turnpike Authority Southern Toll Plaza - Phase I Report **Location 8.7** Figure 4.8

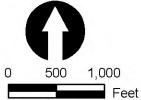






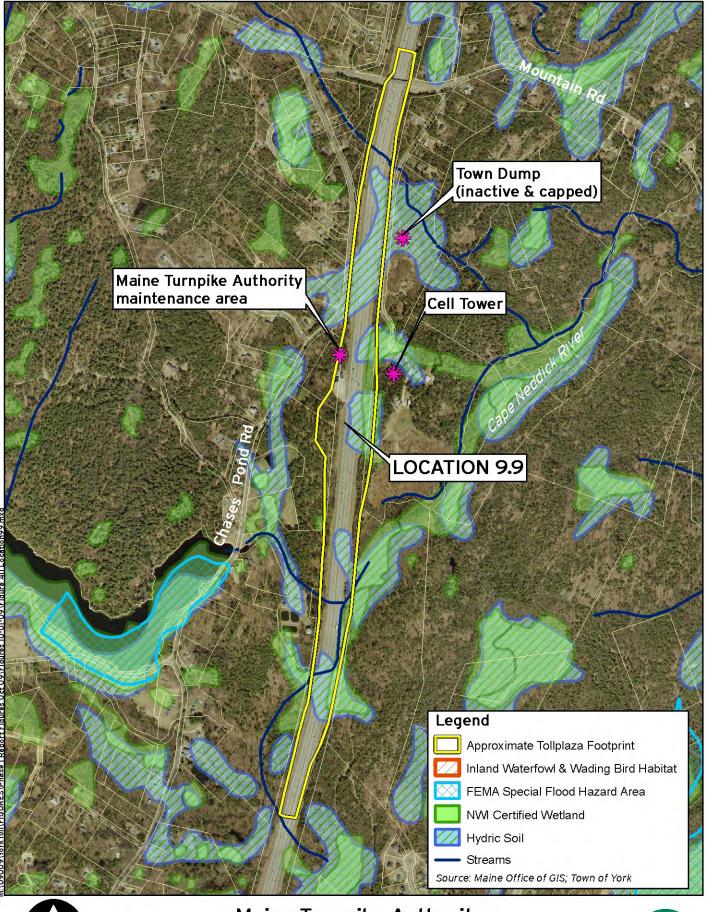


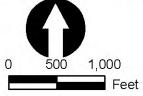




Maine Turnpike Authority Southern Toll Plaza - Phase I Report **Location 9.1** Figure 4.10







**Location 9.9** Figure 4.11



#### Mountain Road to Clay Hill Road; Mile Marker 10.6 to 11.9 – York

Two vertical high points, *Locations 11.3 and 11.4*, are located along this tangent section of roadway. Both locations (Figure 4.12 and Figure 4.13 respectively) in this section have the highest number of homes within 1,000 feet of the Maine Turnpike and Location 11.3 has the highest number of potential home displacements (5) of all 17 locations (including the existing site options). These locations would also have much higher wetland impacts compared with many other locations, and greater right-of-way requirements. Project costs for these locations are estimated at \$39 and \$38 million respectively. **Based upon home displacements and impacts both of these locations, 11.3 and 11.4, are determined to be not practicable and furthermore to be more environmentally damaging than other potential locations and are not recommended to be carried forward for further evaluation.** 

# Clay Hill Road to North Berwick Road; Mile Marker 11.9 to 13.8 - York & Ogunquit

One vertical high point, *Location 13.2*, is located along this tangent section of roadway. This location (Figure 4.14) meets the basic design criteria, has the least stream impacts, and is in the lower range of wetland impacts. However, Location 13.2 would displace two homes and is in the middle of the comparison range of residences within 1,000 feet of the Maine Turnpike. While environmental impacts are low and similar to other available alternates, Location 13.2 is determined to be not practicable based on displacements of two homes. Project costs are estimated at \$36 million. Location 13.2 is not recommended to be carried forward for further evaluation.

# North Berwick Road to Captain Thomas Road; Mile Marker 13.8 to 14.8 - Ogunquit

This section of roadway has numerous environmental and social constraints including being on a curved horizontal alignment with no vertical high points. Therefore, this section would not accommodate a new toll plaza that satisfies design criteria.

## Captain Thomas Road to Tatnic Road; Mile Marker 14.8 to 15.2 – Ogunquit & Wells

This section is less than one mile in length with extensive environmental constraints and a crossing of the Ogunquit River. This section of roadway would not accommodate a new toll plaza that satisfies design criteria.

# Tatnic Road to Littlefield Road; Mile Marker 15.2 to 17.3 – Wells

Three vertical high points, *Locations 15.8*, *16.5*, *and 16.9*, are located along this tangent section of roadway. Locations 15.8 and 16.5 (Figure 4.15 and Figure 4.16) would have no displacements of homes, but would occur near an area of moderate density neighborhoods, including Meadow Ledge RV Park and Summer Village Cottages; essentially ranking these two alternatives in the highest range of nearby homes when compared to other alternatives with no home impacts. Location 16.9 (Figure 4.17) also would have no direct impact to homes and would have less homes in proximity to the proposed toll plaza.

Wetland impacts at Locations 15.8 and 16.5 would be in the low range of the 17 potential locations; however, these impacts would be greater than impacts at Locations 8.6, 8.7, and 8.8 which have the least. Right-of-way impacts at Locations 15.8 and 16.5 are in the moderate range

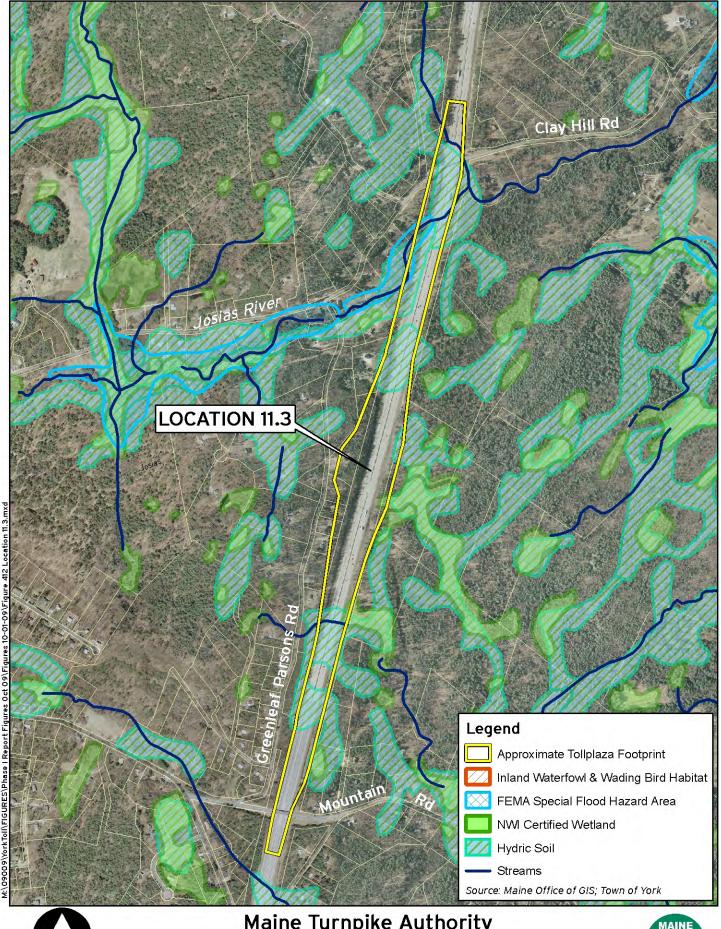
of the 17 potential locations, but would be higher than impacts at Locations 8.6, 8.7, and 8.8. Potential stream impacts at Location 15.8 and 16.5 are in the moderate range of the 17 potential locations, and these impacts are similar to or less than stream impacts at potential locations between Chase Pond Road and Mountain Road (Locations 8.6, 8.7, 8.8). Location 16.9 would have the highest floodplain, stream and wetland impacts and nearly the highest right-of-way impacts of the three potential locations in this section. Project costs for these locations are estimated at \$36 to \$38 million with the higher costs being for the northern location due to the increased amount of environmental and social impacts.

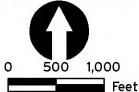
There has been previous consideration of constructing a new interchange on the Maine Turnpike between the York and Wells exits for providing improved access to the Ogunquit region. These previous studies have indicated that a potential connection from the Maine Turnpike with a new interchange would be in the vicinity of the Tatnic Road overpass. The construction of a new toll plaza in the vicinity of the Tatnic Road overpass would make future construction of an interchange in this area not practicable due to recommended engineering spacing constraints between interchanges and mainline toll plazas.

Therefore Locations 15.8 and 16.5 should be dismissed from further consideration because there are other alternatives that are less environmentally damaging and are not candidate locations for a future interchange location. Location 16.9 is also not recommended to be advanced for further study because it also is more environmentally damaging than other new location alternatives.

# <u>Littlefield Road to Wells Interchange Mile Marker 17.3 to 19.3 – Wells</u>

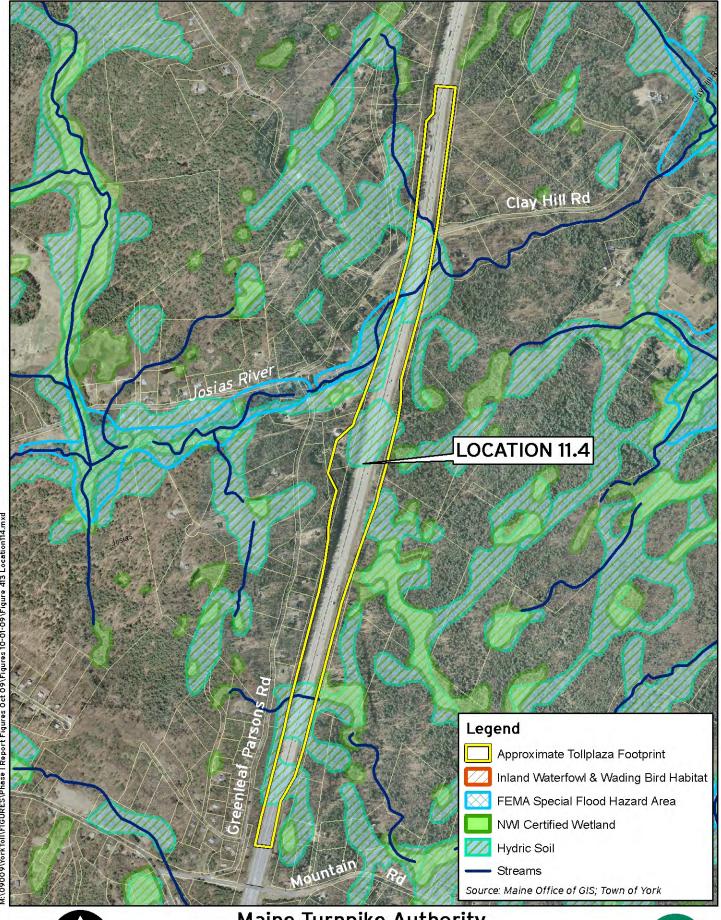
One vertical high point, *Location 17.7*, exists north of Littlefield Road and south of the single horizontal curve along this section of roadway. Location 17.7 (Figure 4.18) does not completely satisfy the separation from an interchange engineering criteria, or the horizontal alignment criteria. This location is in the vicinity of the Webhannet River and its associated wetlands although impacts are in the low- to mid-range. Location 17.7 does have significant right-of-way impacts, in fact, the highest of the 17 sites. Furthermore, this location is more environmentally damaging than other potential locations. Project costs are estimated at \$37 million. **Based upon not completely meeting the basic design criteria, the significant right-of-way impacts, and environmental damage, Location 17.7 is determined to be not practicable and is not recommended to be carried forward for further evaluation.** 

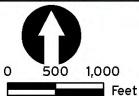




Location 11.3

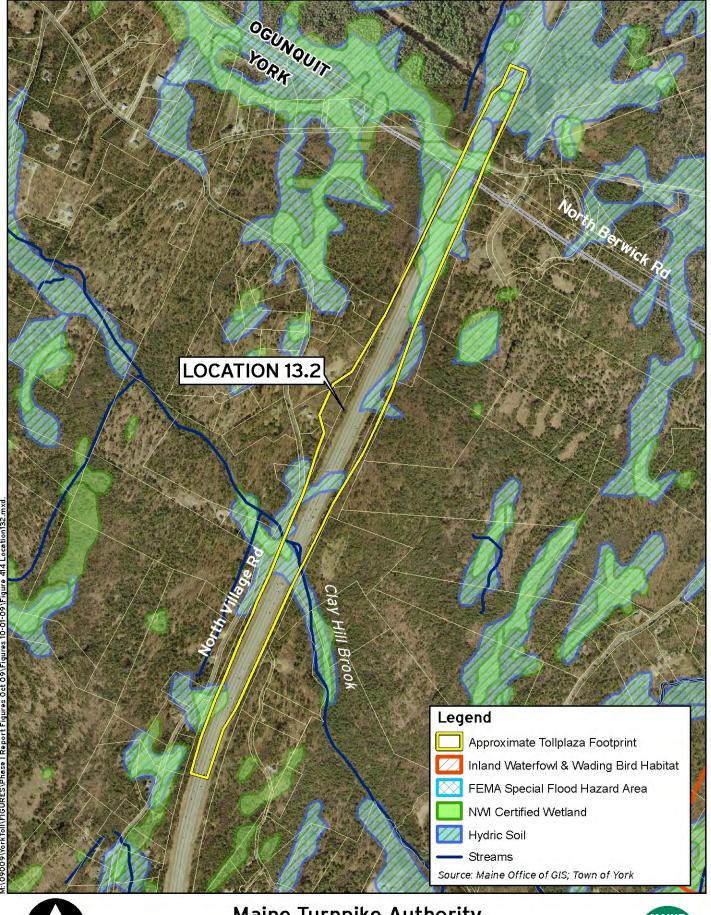


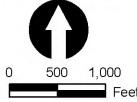






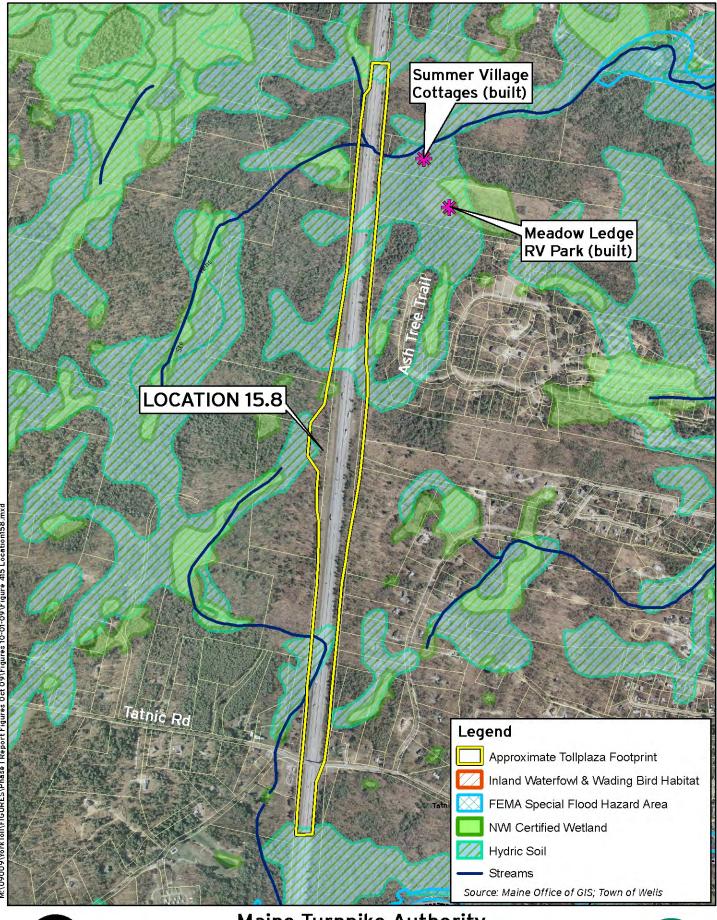


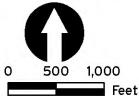




Location 13.2

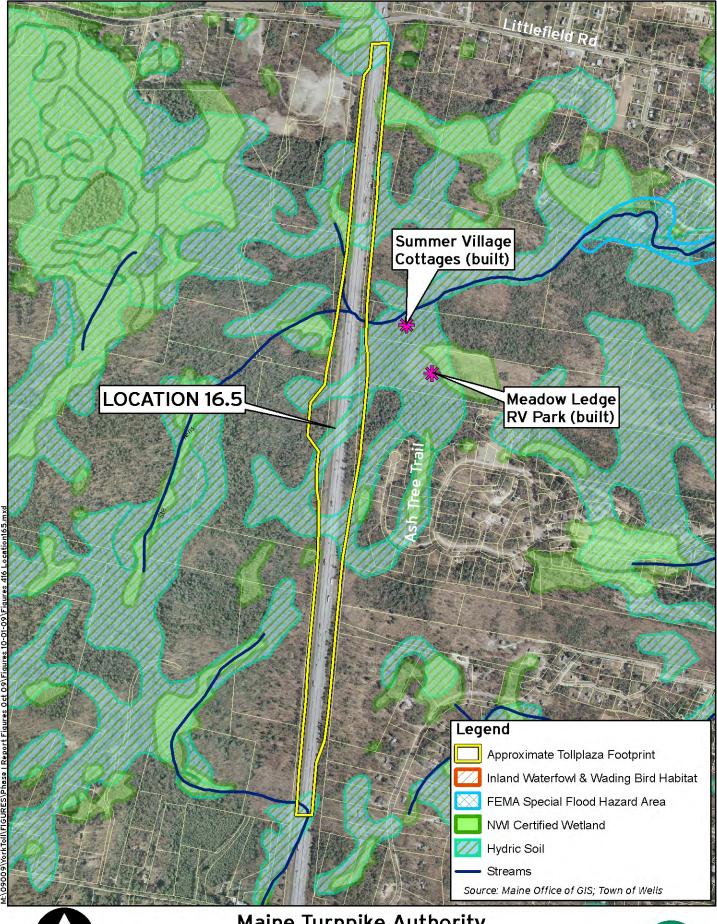


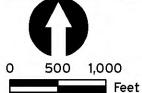




> Location 15.8 Figure 4.15

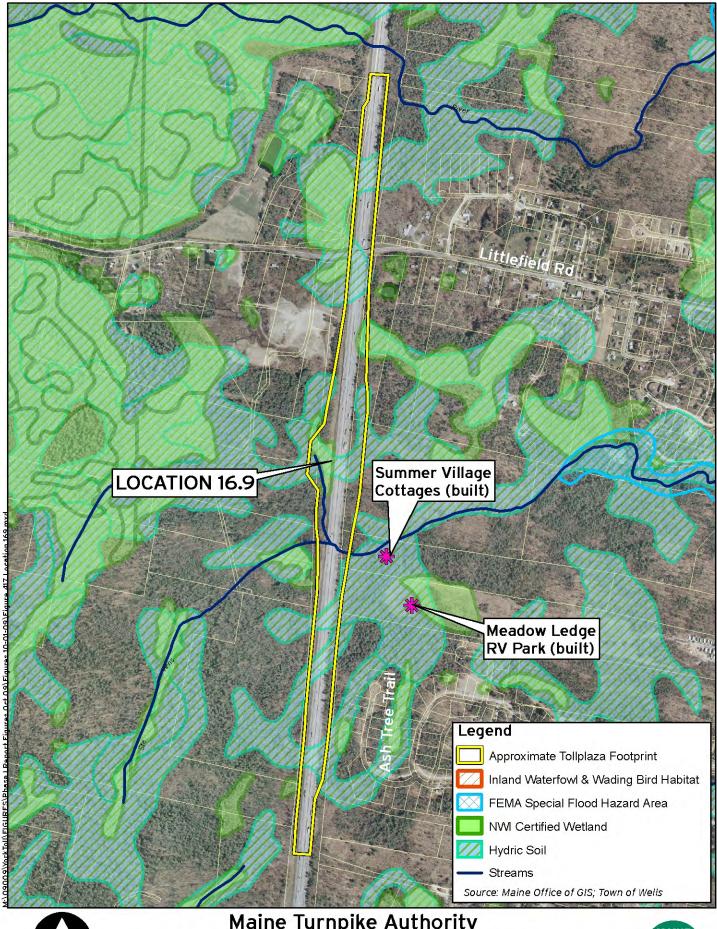


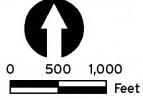




Location 16.5 Figure 4.16

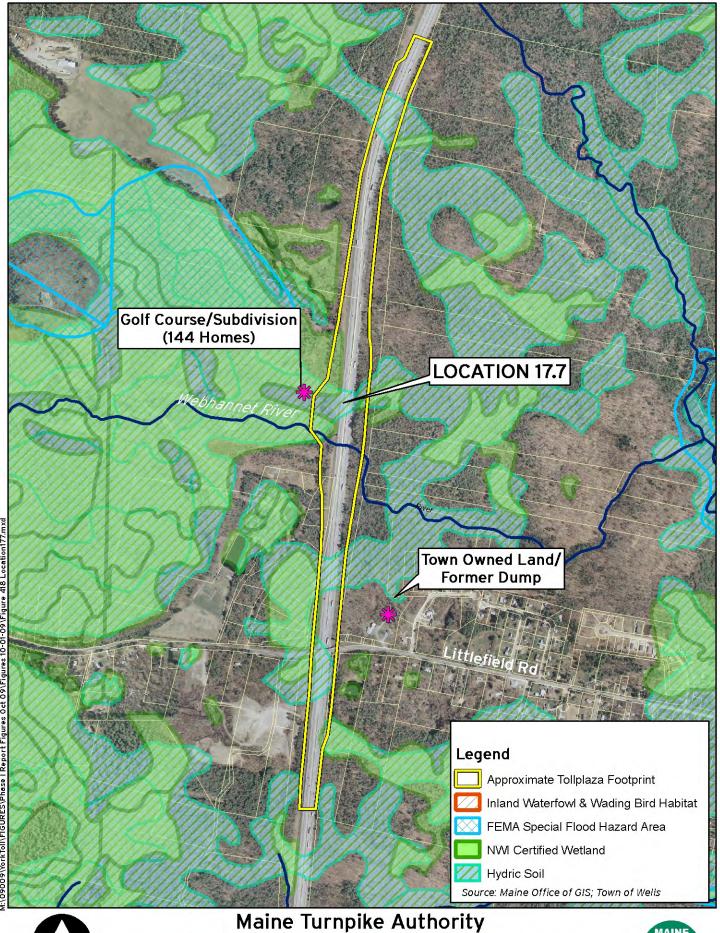


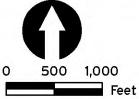




Location 16.9









# **SECTION 2 - SUMMARY AND RECOMMENDATIONS**

This Phase I report and application provides:

- Reasons for the York Toll Plaza replacement (purpose);
- Explanation of the toll technology, tolling operations, and traffic implications of the tolling facility (needs);
- Description of impacts associated with utilizing the existing location;
- Description of engineering and planning methods to identify candidate locations;
- Description of potential alternate locations and reasoning for selecting those locations;
- Description of the impacts of candidate locations;
- Description of the screening of candidate locations; and,
- Recommendation of locations for further study.

Several options for utilizing the existing York Toll Plaza location were evaluated, including options with minimal modifications and with extensive modifications. Most of the existing site options were not capable of satisfying the basic design criteria and all options would have considerably more direct environmental impacts and construction costs. Of the existing site options, Option 4A at Location 7.3 partially satisfies the purpose and need, partially meets the basic engineering criteria, and although having significant environmental impacts, they are on the lower end of the nine upgrade options evaluated in Part 2 (Existing Site Evaluation). However Option 4A is the most environmentally damaging when compared to the 16 potential alternate locations. Applying unit costs to the base plaza construction as well as the significant amount of environmental and social impacts allows for another perspective in the overall site evaluation. The results are, Option 4A has an estimated project cost 40-50% higher than the majority of the 16 potential alternate sites and nearly 60% higher than two (of the 16) alternate sites recommended to be carried forward for further evaluation. Nevertheless, in order to compare an upgrade option with the alternate locations, it is recommended that Option 4A at Location 7.3 be carried forward for further evaluation as the best of the existing site upgrade options into Phase II of the USACE Highway Methodology.

Of the 16 potential alternate locations evaluated for a new toll plaza, two of the locations, Location 8.7 and 9.1 satisfy the purpose and need, were determined to be the most practicable as it pertains to home displacements and impacts, and were the least environmentally damaging and are recommended to be carried forward for further evaluation into Phase II of the Highway Methodology Process.

These three locations, Option 4A at Location 7.3, Location 8.7, and Location 9.1, are proposed to be further evaluated and compared with each other and against the baseline No-Build Option with regard to cost, design features, natural resource impacts, community (proximity) effects, permitting, operations and maintenance, utility requirements, and constructability in Phase II of the Highway Methodology Process.

This document represents Phase I of the USACE Highway Methodology. The Maine Turnpike Authority will continue with the York Toll Plaza Replacement project upon receiving approval by the USACE that the three alternates mentioned above, along with the No-Build Option, are approved to be carried forward into **Phase II** of the Highway Methodology.

# APPENDIX A DESIGN GUIDELINES

Following is a summary list of national engineering design guidelines and standards used throughout development of the Maine Turnpike Southern Toll Plaza Draft Phase I report. A select few pages are included here for reference as they note the importance of nationally accepted and implemented guidelines. These pages comment on uniformity in design practices being a key factor in the safety of travelers on our Nation's highways.

A Policy on Geometric Design of Highways and Streets 5<sup>th</sup> ed. Washington D.C.: AASHTO, 2004: xliii – xliv, 115-116

Manual of Uniform Traffic Control Devices for Highways and Streets 2003 ed. Washington D.C.: FHWA, ATSSA, AASHTO, ITE: 1A1 – 1A-2

Roadside Design Guide 3<sup>rd</sup> ed. Washington D.C.: AASHTO, 2006: vii

State of the Practice and Recommendation on Traffic Control Strategies at Toll Plazas 1<sup>st</sup> ed.: FHWA, Wilbur Smith Associates, 2006: 1-2, 16, 133-134

# **Foreword**

As highway designers, highway engineers strive to provide for the needs of highway users while maintaining the integrity of the environment. Unique combinations of design requirements that are often conflicting result in unique solutions to the design problems. The guidance supplied by this text, *A Policy on Geometric Design of Highways and Streets*, is based on established practices and is supplemented by recent research. This text is also intended to form a comprehensive reference manual for assistance in administrative, planning, and educational efforts pertaining to design formulation.

Design values are presented in this document in both metric and U.S. customary units and were developed independently within each system. The relationship between the metric and U.S. customary values is neither an exact (soft) conversion nor a completely rationalized (hard) conversion. The metric values are those that would have been used had the policy been presented exclusively in metric units; the U.S. customary values are those that would have been used if the policy had been presented exclusively in U.S. customary units. Therefore, the user is advised to work entirely in one system and not attempt to convert directly between the two.

The fact that new design values are presented herein does not imply that existing streets and highways are unsafe, nor does it mandate the initiation of improvement projects. This publication is not intended as a policy for resurfacing, restoration, or rehabilitation (3R) projects. For projects of this type, where major revisions to horizontal or vertical curvature are not necessary or practical, existing design values may be retained. Specific site investigations and crash history analysis often indicate that the existing design features are performing in a satisfactory manner. The cost of full reconstruction for these facilities, particularly where major realignment is not needed, will often not be justified. Resurfacing, restoration, and rehabilitation projects enable highway agencies to improve highway safety by selectively upgrading existing highway and roadside features without the cost of full reconstruction. When designing 3R projects, the designer should refer to TRB Special Report 214, *Designing Safer Roads: Practices for Resurfacing, Restoration, and Rehabilitation* and related publications for guidance.

The intent of this policy is to provide guidance to the designer by referencing a recommended range of values for critical dimensions. It is not intended to be a detailed design manual that could supercede the need for the application of sound principles by the knowledgeable design professional. Sufficient flexibility is permitted to encourage independent designs tailored to particular situations. Minimum values are either given or implied by the lower value in a given range of values. The larger values within the ranges will normally be used where the social, economic, and environmental (S.E.E.) impacts are not critical.

The highway, vehicle, and individual users are all integral parts of transportation safety and efficiency. While this document primarily addresses geometric design issues, a properly equipped and maintained vehicle and reasonable and prudent performance by the user are also necessary for safe and efficient operation of the transportation facility.

Emphasis has been placed on the joint use of transportation corridors by pedestrians, cyclists, and public transit vehicles. Designers should recognize the implications of this sharing of the transportation corridors and are encouraged to consider not only vehicular movement, but also movement of people, distribution of goods, and provision of essential services. A more comprehensive transportation program is thereby emphasized.

Cost-effective design is also emphasized. The traditional procedure of comparing highway-user benefits with costs has been expanded to reflect the needs of non-users and the environment. Although adding complexity to the analysis, this broader approach also takes into account both the need for a given project and the relative priorities among various projects. The results of this approach may need to be modified to meet the needs-versus-funds problems that highway administrators face. The goal of cost-effective design is not merely to give priority to the most beneficial individual projects but to provide the most benefits to the highway system of which each project is a part.

Most of the technical material that follows is detailed or descriptive design information. Design guidelines are included for freeways, arterials, collectors, and local roads, in both urban and rural locations, paralleling the functional classification used in highway planning. The book is organized into functional chapters to stress the relationship between highway design and highway function. An explanation of functional classification is included in Chapter 1.

These guidelines are intended to provide operational efficiency, comfort, safety, and convenience for the motorist. The design concepts presented herein were also developed with consideration for environmental quality. The effects of the various environmental impacts can and should be mitigated by thoughtful design processes. This principle, coupled with that of aesthetic consistency with the surrounding terrain and urban setting, is intended to produce highways that are safe and efficient for users, acceptable to non-users, and in harmony with the environment.

This publication supersedes the 2001 AASHTO publication of the same name. Because the concepts presented could not be completely covered in one book, references to additional literature are given at the end of each chapter.

2003 Edition Page 1A-1

#### CHAPTER 1A. GENERAL

#### **Section 1A.01 Purpose of Traffic Control Devices**

Support:

The purpose of traffic control devices, as well as the principles for their use, is to promote highway safety and efficiency by providing for the orderly movement of all road users on streets and highways throughout the Nation.

Traffic control devices notify road users of regulations and provide warning and guidance needed for the reasonably safe, uniform, and efficient operation of all elements of the traffic stream.

#### Standard:

Traffic control devices or their supports shall not bear any advertising message or any other message that is not related to traffic control.

Support:

Tourist-oriented directional signs and Specific Service signs are not considered advertising; rather, they are classified as motorist service signs.

## **Section 1A.02 Principles of Traffic Control Devices**

Support:

This Manual contains the basic principles that govern the design and use of traffic control devices for all streets and highways open to public travel regardless of type or class or the public agency having jurisdiction. This Manual's text specifies the restriction on the use of a device if it is intended for limited application or for a specific system. It is important that these principles be given primary consideration in the selection and application of each device.

#### Guidance:

To be effective, a traffic control device should meet five basic requirements:

- A. Fulfill a need:
- B. Command attention;
- C. Convey a clear, simple meaning;
- D. Command respect from road users; and
- E. Give adequate time for proper response.

Design, placement, operation, maintenance, and uniformity are aspects that should be carefully considered in order to maximize the ability of a traffic control device to meet the five requirements listed in the previous paragraph. Vehicle speed should be carefully considered as an element that governs the design, operation, placement, and location of various traffic control devices.

Support:

The definition of the word "speed" varies depending on its use. The definitions of specific speed terms are contained in Section 1A.13.

#### Guidance:

The actions required of road users to obey regulatory devices should be specified by State statute, or in cases not covered by State statute, by local ordinance or resolution consistent with the "Uniform Vehicle Code."

The proper use of traffic control devices should provide the reasonable and prudent road user with the information necessary to reasonably safely and lawfully use the streets, highways, pedestrian facilities, and bikeways.

#### Support:

Uniformity of the meaning of traffic control devices is vital to their effectiveness. The meanings ascribed to devices in this Manual are in general accord with the publications mentioned in Section 1A.11.

#### Section 1A.03 Design of Traffic Control Devices

## Guidance:

Devices should be designed so that features such as size, shape, color, composition, lighting or retroreflection, and contrast are combined to draw attention to the devices; that size, shape, color, and simplicity of message combine to produce a clear meaning; that legibility and size combine with placement to permit adequate time for response; and that uniformity, size, legibility, and reasonableness of the message combine to command respect.

Page 1A-2 2003 Edition

#### **Standard:**

All symbols shall be unmistakably similar to or mirror images of the adopted symbol signs, all of which are shown in the "Standard Highway Signs" book (see Section 1A.11). Symbols and colors shall not be modified unless otherwise stated herein. All symbols and colors for signs not shown in the "Standard Highway Signs" book shall follow the procedures for experimentation and change described in Section 1A.10.

#### Guidance:

Aspects of a device's design should be modified only if there is a demonstrated need.

#### Support

An example of modifying a device's design would be to modify the Side Road (W2-2) sign to show a second offset intersecting road.

#### Option:

Highway agencies may develop word message signs to notify road users of special regulations or to warn road users of a situation that might not be readily apparent. Unlike symbol signs and colors, new word message signs may be used without the need for experimentation. With the exception of symbols and colors, minor modifications in the specific design elements of a device may be made provided the essential appearance characteristics are preserved. Although the standard design of symbol signs cannot be modified, it may be appropriate to change the orientation of the symbol to better reflect the direction of travel.

#### Section 1A.04 Placement and Operation of Traffic Control Devices

#### Guidance:

Placement of a traffic control device should be within the road user's view so that adequate visibility is provided. To aid in conveying the proper meaning, the traffic control device should be appropriately positioned with respect to the location, object, or situation to which it applies. The location and legibility of the traffic control device should be such that a road user has adequate time to make the proper response in both day and night conditions.

Traffic control devices should be placed and operated in a uniform and consistent manner.

Unnecessary traffic control devices should be removed. The fact that a device is in good physical condition should not be a basis for deferring needed removal or change.

#### Section 1A.05 Maintenance of Traffic Control Devices

#### Guidance:

Functional maintenance of traffic control devices should be used to determine if certain devices need to be changed to meet current traffic conditions.

Physical maintenance of traffic control devices should be performed to retain the legibility and visibility of the device, and to retain the proper functioning of the device.

#### Support:

Clean, legible, properly mounted devices in good working condition command the respect of road users.

## Section 1A.06 <u>Uniformity of Traffic Control Devices</u>

#### Support:

Uniformity of devices simplifies the task of the road user because it aids in recognition and understanding, thereby reducing perception/reaction time. Uniformity assists road users, law enforcement officers, and traffic courts by giving everyone the same interpretation. Uniformity assists public highway officials through efficiency in manufacture, installation, maintenance, and administration. Uniformity means treating similar situations in a similar way. The use of uniform traffic control devices does not, in itself, constitute uniformity. A standard device used where it is not appropriate is as objectionable as a nonstandard device; in fact, this might be worse, because such misuse might result in disrespect at those locations where the device is needed and appropriate.

# Section 1A.07 <u>Responsibility for Traffic Control Devices</u>

#### Standard:

The responsibility for the design, placement, operation, maintenance, and uniformity of traffic control devices shall rest with the public agency or the official having jurisdiction. 23 CFR 655.603 adopts the Manual on Uniform Traffic Control Devices as the national standard for all traffic control devices installed on any street, highway, or bicycle trail open to public travel. When a State or other Federal agency

#### **PREFACE**

The Roadside Design Guide is developed and maintained by the AASHTO Subcommittee on Design, Technical Committee for Roadside Safety. The guide presents a synthesis of current information and operating practices related to roadside safety and is written in dual units—metric and U.S. Customary units. The 2006 edition of the guide supersedes the 1996 AASHTO publication of the same name and includes an update to Chapter 6, "Median Barriers," which replaces Chapter 6 of the 2002 edition.

In this guide, the roadside is defined as that area beyond the traveled way (driving lanes) and the shoulder (if any) of the roadway itself. Consequently, roadside delineation, shoulder surface treatments, and similar onroadway safety features are not extensively discussed. While it is a readily accepted fact that safety can best be served by keeping motorists on the road, the focus of the guide is on safety treatments that minimize the likelihood of serious injuries when a driver runs off the road.

A second noteworthy point is that this document is a guide. It is not a standard, nor is it a design policy. It is intended for use as a resource document from which individual highway agencies can develop standards and policies. While much of the material in the guide can be considered universal in its application, there are several recommendations that are subjective in nature and may need modification to fit local conditions. However, it is important that significant deviations from the guide be based on operational experience and objective analysis.

To be consistent with AASHTO's A Policy on Geometric Design of Highways and Streets, design speed is as the basic speed parameter to be used in this guide. However, since the design speed is often selected based on the most restrictive physical features found on a specific project, there may be a significant percentage of a project length where that speed will be exceeded by a reasonable and prudent driver. Conversely, there will be other instances where roadway conditions will prevent most motorists from driving as fast as the design speed. Because roadside safety design is intended to minimize the consequences of a motorist leaving the roadway inadvertently, the designer should consider the speed at which encroachments are most likely to occur when selecting an appropriate roadside design standard or feature.

Design values are presented in this document in both metric and U.S. Customary units. The relationship between the metric and U.S. Customary values is neither an exact (soft) conversion nor a completely rationalized (hard) conversion. The metric values are those that would have been used had the guide been presented exclusively in metric units; the U.S. Customary values are those that would have been used if the guide had been presented exclusively in U.S. Customary units. Therefore, the user is advised to work entirely in one system and not to attempt to convert directly between the two.

The reader is cautioned that roadside safety is a rapidly changing field of study, and changes in policy, criteria, and technology are certain to occur after this document is published. Efforts should be made to incorporate the appropriate current design elements into the project development. Comments from users of this guide on suggested changes or modifications resulting from further developmental work or hands-on experience are appreciated. All such comments should be addressed to the American Association of State Highway and Transportation Officials, Engineering Program, 444 North Capitol Street NW, Suite 249, Washington, DC 20001.

## CHAPTER 1

# INTRODUCTION

This report has been prepared under a project initiative by the Federal Highway Administration (FHWA) to identify the 'state-of-the-practice' for traffic control strategies at toll plazas, and to develop recommended guidelines for agencies and departments that operate or plan to design and build such facilities.

The report contents begin with this introductory chapter. This chapter includes sections that outlines the purpose of this Project, provides a problem statement, articulating the focus of the project efforts, lists the study objectives, describes the methodology used to achieve the objectives, and concludes with the intended use of this report.

The introduction is followed by four chapters that include the state-of-the-practice and recommended guidelines for the following technical areas encompassing the development of traffic control strategies at toll plazas: Plaza Operations/Lane Configuration', 'Signing, Markings and Channelization', 'Geometric and Safety Design', and 'Toll Collection Equipment Technology'. The aggregation of these chapters provides useful historical information and a comprehensive analysis of when and where to apply various traffic control strategies.

The final chapter concludes this Report by identifying further research needs, which require more rigorous study including field verification of performance. This chapter also lists all of the recommended guidelines presented in the preceding chapters. A glossary of terms, definitions and diagrams to assist the reader's understanding of the topic material follows along with Appendix A Summary of Survey Results, Appendix B Expert Panel Workshop Summary, and Appendix C Literature Search.

#### 1.1 PURPOSE

The purpose and focus of this report is to develop guidelines for designing and implementing traffic control strategies and devices at toll plazas that, for example, inform drivers which lanes to use for specific methods of payment, reduce speed variance, discourage lane changing and properly install equipment and devices. This was accomplished after researching related studies and reports, surveying current practices, and learning from the experience of experts within the toll collection industry. The goal is to achieve a consistent strategy for handling potential points of conflict, controlling flow of various vehicle types and conveying information at toll plazas so that safety and operations are enhanced, better efficiency and economy of design are achieved, and motorist recognition and comprehension are improved. This must be accomplished in consideration of the fact that each toll facility may desire its own unique identity.

approaches to bridges and tunnels. Different types of toll collection processes are addressed, including: automated cash/card/ticket, manual cash/card/ticket, and electronic toll collection (ETC). While this report covers plazas on roadway mainlines, interchange and access ramps, and approaches to bridges, and tunnels, the scope of the survey contained in Appendix A is limited to mainline plazas and approaches to bridges and tunnels. Therefore, design considerations and elements unique to ramp plazas may not be addressed in this report.

#### 1.2 PROBLEM STATEMENT

Many decision points exist while approaching the plaza, at the plaza, and on departure from the plaza. The decision points can lead to vehicle merging, weaving, queuing, diverging and differential speeds. Diverging and weaving occurs on the approach to the plaza as electronic toll collection (ETC) users separate from cash paying customers, who then further diverge based on selected cash payment lane type, shortest traffic queue, and lane status (i.e., open or closed). Multiple collection methods can increase the potential for side swipe and rear-end collisions if the lane groupings are not clear to users who are making choices of which lane to use for payment. Potential safety hazards particularly exist when approaching and departing ETC dedicated lanes. When an driver unfamiliar with the toll plaza realizes their vehicle is in the wrong payment lane and suddenly stops, a following high-speed, ETC-equipped vehicle can easily collide with the stopped vehicle. Consequently, speed variance is another important factor to be considered at mixed use toll facilities. Similarly, merging and weaving occurs on the departure side of the plaza as the number of toll lanes tapers down to the width of the continuing mainline.

Various studies and reports have presented summaries of the state-of-the-practice within the industry, primarily related to specific design elements or practices of toll agencies. The present environment is seeing significant increases in new toll highway miles, resulting in more toll plazas, most of which include high speed express lanes for ETC users only. Further trends show toll roads facing greater commuter and recreational demands, resulting in cash paying and ETC users familiar with the toll road mixed with unfamiliar cash paying users. Without the use of good design practice, including effective deployment of various traffic control devices, this mix can result in unsafe and inefficient operations. ETC users now expect non-stop, high speed travel through toll plazas without incurring any delays. Development of national guidelines that address the implications of electronic toll collection on plaza operations has therefore become much more critical.

Toll plazas have been designed and constructed in the United States without the benefit of national toll plaza design guidelines and standards, often resulting in driver unfamiliarity and inefficient vehicle throughput. Without national guidelines and standards, designs have evolved placing undue focus on monetary constraints, deploying signs with too little or too much information, inefficiently configuring toll lanes and embodying design features with greater emphasis on establishing a unique identity than on plaza safety and operations. As a result, toll plaza design elements and practices vary from agency to agency, and are often dictated by either legacy toll plaza design practices or variations to historical designs that retains a distinctive appearance while incorporating enhancements to correct deficiencies. Plaza modifications made to add electronic toll collection (ETC) to existing plaza facilities

operations, and relatively low commuter traffic volumes are forecasted.

The expectation based on recent toll facility projects is new mainline toll plaza requirements will include non-stop ETC express lanes, and new ramp plaza requirements will include non-stop ETC dedicated lanes. In these cases, the driver approaching a plaza will have to make a choice between the non-stop lanes and the conventional plaza lanes or adjacent cash lane(s).

# Plaza Location Guidelines

Guideline	Plaza Locations Guideline 1						
Title	Plaza and Interchange Intervals						
Text	The 2001 AASHTO A Policy on Geometric Design of Highways and Streets (the "Green Book") recommends separation of 1 mile (urban sections) or 3 miles (rural sections) between interchanges. This criteria should be used as a guideline for selection of new mainline toll plaza sites (i.e., the interstate standards require 1 mile to the nearest interchange in urban areas and 3 miles in rural areas).						
Commentary	Although it may not be possible to meet this design guideline at bridge and tunnel crossings, the interval spacing minimums should remain a goal.						

Guideline	Plaza Locations Guideline 2						
Title	Site Selection and Sight Distance						
Text	New toll plazas should be sited such that motorists will be able to see the plaza, while driving at posted speeds with adequate stopping sight distance before the queue zone. The plaza site should be on a tangent pavement section.						
Commentary	None.						

Guideline	Plaza Locations Guideline 3						
Title	Ramp Plaza Movements						
Text	New toll plazas should not have merging or diverging movements within the plaza approach and departure zones. New plaza construction should not occur within trumpet interchange areas, if possible.						
Commentary	Some existing toll plaza locations have merging and diverging movements within the plaza approach and departure zones. Other appropriate treatment options could be applied to improve their operations.						

## 4.7 VERTICAL PROFILE GRADES

The vertical profile grade is the percent of elevation change along the centerline of the roadway. Vertical grades are necessary to assure drainage of storm water within the plaza to inlets and or outfall locations. Profile grades affectively reduce construction costs by more closely following the natural grade within the established right of way, balancing the quantity of excavation and embankment material and reducing the foundation and earthwork cost of bridges.

## 4.7.1 State-of-the-Practice

The survey did not request information on vertical profile grades.

#### 4.7.2 Recommended Guidelines

# Vertical Profile Grade Design Issue and Guideline Development

Construction of a toll plaza at the crest of a profile grade results in sight distance advantages and plaza operations benefit from gravitational forces in slowing vehicles approaching the toll lanes and accelerating vehicles departing the plaza. Consequently, some studies have recommended the use of a  $\pm$  3% grade for the plaza approach and departure area. Unfortunately, when the plaza's mixed flow traffic includes commercial vehicles, a 3% grade will adversely affect the performance of these vehicles, resulting in additional delays through the plaza. A vertical profile grade greater than or equal to  $\pm$  1% and less than or equal to  $\pm$  2% better accommodates the performance of commercial vehicles under the stop and go conditions normally encountered in plaza queue zones. For the toll lanes, the cross slope and the vertical profile grade should be designed concurrently to assure proper drainage. Under no circumstances should the vertical profile grade be less than  $\pm$  0.5% or exceed  $\pm$  2% in a toll lane. This avoids the undesirable need to install trench or slot drains across the toll lane entrance that may clog, causing the possible unsafe condition (to both attendant and user) of ice formation within the lane. The canopy and storm drainage system design must direct collected water away from the toll lanes.

# **Recommended Guidelines**

Guideline	Vertical Profile Grade Guideline 1  Plaza Approach and Departure Profile Grades  In cases of fixed flow traffic, the vertical profile grade approaching and departing the toll plaza should be greater than or equal to ± 1% and less than or equal to ±2%.						
Title							
Text							
Commentary	The upper limit on vertical profile grades may be increased to <u>+3 when</u> the percentage of commercial vehicles is low and the toll plaza is located at the crest of the profile grade.						

Guideline	Vertical Profile Grade Guideline 2							
Title	Toll Lane Profile Grades							
Text	The vertical profile grade in a toll lane should be equal to or greater than $\pm 0.5\%$ and less than or equal to $\pm 2\%$ .							
Commentary	The cross slope and profile grade should be designed in conjunction to avoid storm drainage flows across the entrance to the toll lane. The canopy and storm drainage system design should direct collected water away from the toll lanes and help reduce precipitation within the toll lane.							

Metric							US Customary						
Design	Stopping sight distance (m)						Design	sign Stopping sight distance (ft					)
speed	Downgrades			Upgrades			speed	Downgrades			Upgrades		
(km/h)	3 %	6 %	9 %	3 %	6 %	9 %	(mph)	3 %	6 %	9 %	3 %	6 %	9 %
20	20	20	20	19	18	18	15	80	82	85	75	74	73
30	32	35	35	31	30	29	20	116	120	126	109	107	104
40	50	50	53	45	44	43	25	158	165	173	147	143	140
50	66	70	74	61	59	58	30	205	215	227	200	184	179
60	87	92	97	80	77	75	35	257	271	287	237	229	222
70	110	116	124	100	97	93	40	315	333	354	289	278	269
80	136	144	154	123	118	114	45	378	400	427	344	331	320
90	164	174	187	148	141	136	50	446	474	507	405	388	375
100	194	207	223	174	167	160	55	520	553	593	469	450	433
110	227	243	262	203	194	186	60	598	638	686	538	515	495
120	263	281	304	234	223	214	65	682	728	785	612	584	561
130	302	323	350	267	254	243	70	771	825	891	690	658	631
							75	866	927	1003	772	736	704
							80	965	1035	1121	859	817	782

Exhibit 3-2. Stopping Sight Distance on Grades

# **Decision Sight Distance**

Stopping sight distances are usually sufficient to allow reasonably competent and alert drivers to come to a hurried stop under ordinary circumstances. However, these distances are often inadequate when drivers must make complex or instantaneous decisions, when information is difficult to perceive, or when unexpected or unusual maneuvers are required. Limiting sight distances to those needed for stopping may preclude drivers from performing evasive maneuvers, which often involve less risk and are otherwise preferable to stopping. Even with an appropriate complement of standard traffic control devices in accordance with the MUTCD (6), stopping sight distances may not provide sufficient visibility distances for drivers to corroborate advance warning and to perform the appropriate maneuvers. It is evident that there are many locations where it would be prudent to provide longer sight distances. In these circumstances, decision sight distance provides the greater visibility distance that drivers need.

Decision sight distance is the distance needed for a driver to detect an unexpected or otherwise difficult-to-perceive information source or condition in a roadway environment that may be visually cluttered, recognize the condition or its potential threat, select an appropriate speed and path, and initiate and complete the maneuver safely and efficiently (7). Because decision sight distance offers drivers additional margin for error and affords them sufficient length to maneuver their vehicles at the same or reduced speed, rather than to just stop, its values are substantially greater than stopping sight distance.

Drivers need decision sight distances whenever there is a likelihood for error in either information reception, decision making, or control actions (8). Examples of critical locations where these kinds of errors are likely to occur, and where it is desirable to provide decision sight distance include interchange and intersection locations where unusual or unexpected maneuvers are required, changes in cross section such as toll plazas and lane drops, and areas of concentrated

demand where there is apt to be "visual noise" from competing sources of information, such as roadway elements, traffic, traffic control devices, and advertising signs.

The decision sight distances in Exhibit 3-3 (1) provide values for sight distances that may be appropriate at critical locations, and (2) serve as criteria in evaluating the suitability of the available sight distances at these locations. Because of the additional safety and maneuvering space provided, it is recommended that decision sight distances be provided at critical locations or that critical decision points be moved to locations where sufficient decision sight distance is available. If it is not practical to provide decision sight distance because of horizontal or vertical curvature or if relocation of decision points is not practical, special attention should be given to the use of suitable traffic control devices for providing advance warning of the conditions that are likely to be encountered.

	Met	ric			US Customary						
Design	ecision				Design Decision sight distance (ft)						
speed		Avoida	ance m	aneuve	r	speed	- 3 (				
(km/h)	<u> </u>	В	С	D	E	(mph)	A	В	С	D	E
50	70	155	145	170	195	30	220	490	450	535	620
60	95	195	170	205	235	35	275	590	525	625	720
70	115	235	200	235	275	40	330	690	600	715	825
80	140	280	230	270	315	45	395	800	675	800	930
90	170	325	270	315	360	50	465	910	750	890	1030
100	200	370	315	355	400	55	535	1030	865	980	1135
110	235	420	330	380	430	60	610	1150	990	1125	1280
120	265	470	360	415	470	65	695	1275	1050	1220	1365
130	305	525	390	450	510	70	780	1410	1105	1275	1445
						75	875	1545	1180	1365	1545
Avoidance						80	970	1685	1260	1455	1650

Avoidance Maneuver A: Stop on rural road—t = 3.0 s

Avoidance Maneuver B: Stop on urban road—t = 9.1 s

Avoidance Maneuver C: Speed/path/direction change on rural road—/ varies between 10.2 and 11.2 s

Avoidance Maneuver D: Speed/path/direction change on suburban road—t varies between

12.1 and 12.9 s

Avoidance Maneuver E: Speed/path/direction change on urban road—t varies between 14.0 and 14.5 s

**Exhibit 3-3. Decision Sight Distance** 

Decision sight distance criteria that are applicable to most situations have been developed from empirical data. The decision sight distances vary depending on whether the location is on a rural or urban road and on the type of avoidance maneuver required to negotiate the location properly. Exhibit 3-3 shows decision sight distance values for various situations rounded for design. As can be seen in the exhibit, shorter distances are generally needed for rural roads and for locations where a stop is the appropriate maneuver.

For the avoidance maneuvers identified in Exhibit 3-3, the pre-maneuver time is increased above the brake reaction time for stopping sight distance to allow the driver additional time to detect and recognize the roadway or traffic situation, identify alternative maneuvers, and initiate a

# APPENDIX B BASIC PROJECT PURPOSE AND NEED

# REPLY TO ATTENTION OF Regulatory Division CENAE-R-51

#### DEPARTMENT OF THE ARMY

NEW ENGLAND DISTRICT, CORPS OF ENGINEERS 696 VIRGINIA ROAD CONCORD, MASSACHUSETTS 01742-2751

APR 0 9 2007

Joseph G. Grilli, PE HNTB Corporation 75 State Street Boston, MA 02109

Dear Mr. Grilli:

This is in reference to your client's proposal to replace the southern barrier toll plaza on the Maine Turnpike at York, Maine.

Based on presentations at several monthly interagency meetings with Federal and State regulatory and resource agencies, we have determined that the basic project purpose of the project is to replace/rehabilitate the existing barrier toll plaza on the Maine Turnpike at York, Maine, incorporating High Speed Tolling (HST) and addressing settling/subsidence and facilities deficiencies, safety deficiencies, and existing and projected traffic volumes. We will use this basic project purpose to analyze alternatives to avoid and minimize adverse impacts to waters and wetlands in order to comply with the Section 404(b)(1) Guidelines.

If you have any questions concerning this matter, please contact Jay Clement at 207-623-8367 at our Manchester, Maine Project Office.

Sincerely,

Christine Godfrey

Chief, Regulatory Division

Copies Furnished: Trish Garrigan – EPA Wende Mahaney – USFWS Mark Hasselman – FHWA Linda Kokemuller – Maine DEP December 8, 2006

**HNTB** 

Jay Clement, Senior Project Manager Maine Project Office New England District US Army Corps of Engineers 675 Western Avenue #3 Manchester, Maine 04351

Re:

Maine Turnpike Authority

Southern Toll Plaza Replacement

Request for Sign-off on Project Purpose and Need

Dear Mr. Clement:

On behalf of our client, the Maine Turnpike Authority, we are writing to provide information related to replacement of the southern barrier toll plaza at York and to request the Corps' sign-off on Project Purpose and Need. On October 10, 2006 the Maine Turnpike Authority introduced this project to you and other resource and regulatory agencies at the regularly scheduled MaineDOT interagency meeting. Additional information related to project Purpose and Need is provided herewith.

The purpose of the proposed project is to replace the obsolete barrier toll plaza at York with a new toll plaza, with Highway Speed Tolling (HST), at a suitable location determined with consideration of Turnpike operational parameters, engineering design criteria, physical features including regulated natural resources, cultural resources, community resources, and capital and operational costs.

The need for the project is demonstrated by four areas of deficiencies:

Settlement and facility deficiencies; Safety;

Congestion; and Customer service (highway speed tolling).

The attached Project Needs Briefing Paper provides more detail on these deficiencies.

HNTB is currently conducting a site identification and screening study of potential locations for the replacement toll plaza. We will be submitting to you a report on this study on or about the end of the year.

Jay Clement December 7, 2006 Page 2 of 2

Should you have any questions or comments about this project, please feel free to contact us. We look forward to your review and Project Purpose and Need Sign-off.

Regards,

Joseph G. Grilli, PE Study Manager

encl. Project Needs Briefing Paper

cc: C. Welzel, MTA, w/encl.

December 4, 2006

Prepared for: The Maine Turnpike Authority



Prepared by: HNTB Corporation

**HNTB** 

#### **Background**

The existing York Toll Plaza was constructed in 1969. Due to the age of the facility, numerous maintenance and rehabilitation projects have been required to improve the capacity of the plaza, to maintain aging components and to alleviate the adverse conditions resulting from the poor soils in the area. The initial 11-lane plaza was expanded by four lanes as traffic grew in southern Maine. The plaza was modified in 1997 to incorporate electronic toll collection and in 1999, two dedicated electronic toll lanes were added to the plaza to form the current configuration of 17 lanes. The canopy over the original lanes was extended in 2001 to cover all but the exterior dedicated toll lanes. In 2005, the plaza was included in the conversion to E-ZPass.

Today, as the gateway to Maine, the York Toll Plaza sees 15 million transactions per year. \$34 million in revenue is collected here yearly, which is 39 percent of total Maine Turnpike revenue. Truck traffic accounts for 15 to 20 percent of the plaza's use. Forty percent of total traffic at York utilizes Electronic Toll Collection.

In July 2007, the Maine Turnpike Authority authorized implementation of Highway Speed Tolling (HST) at the new Southern Toll Plaza. This feature allows EZ-Pass customers to maintain highway speeds along the mainline highway lanes, while non-EZ-Pass customers must exit the mainline to pay their toll at a conventional toll plaza. This feature improves customer service, aids in congestion relief, provides operational benefits, and provides environmental benefits in terms of air quality and noise.

#### Need

The need for the project is demonstrated by the deficiencies of the York Toll Plaza, a plaza and plaza approach design that does not meet recently published FHWA guidelines.

The age of the toll plaza, the outmoded conditions of the existing tollbooths, canopy, and tunnel, and poor soil conditions make upgrade of the existing facility, including installation of HST technology, infeasible. Proximity to interchanges, inadequate geometry, and exceeded capacity render the existing facility inadequate. Details of these inadequacies are:

#### **Toll Booths**

The original tollbooth structures were designed in the 1960s and are considered deficient by today's standards from a space, layout, climate control, protection, and systems perspective. The original design did not anticipate the need for additional equipment required for modern technology such as Electronic Toll Collection systems. The current booths have limited space for collector activities. The booths are heated in winter but do not include positive ventilation or air conditioning for warm weather operations. Larger modern booths as installed at other locations on the Maine Turnpike will not fit on the existing toll islands. Also, the newer booths have an additional layer of concrete protection on the upstream and downstream ends of the booths providing improved safety for toll collectors.

#### Canopy & Tunnel

The canopy and tunnel infrastructure at the plaza are in poor condition and in need of replacement. The portions of the tunnel directly under the toll lanes have been repaired, but the tunnel sections under the tollbooths haven't been repaired and remain in poor condition. The extensive costs associated with repairing the tunnel sections under the booths rival the costs for a new tunnel. Similarly, the structural supports for the canopy have reached a point of capacity given the additional roofing, equipment, and signage that have been placed on the canopy structure over time.

#### Soil Conditions

The original toll plaza was built in an area with poor subsurface soil conditions, mainly consisting of compressible clay. With this site condition recognized in the design, the plaza tunnel, booths, and canopy were constructed on H-piles to prevent settlement of the entire structure due to consolidation of the clay soils. However, the roadway approaches to the plaza were not pile supported. As a result, the approaches have and continue to settle as the clay soil consolidates. In an effort to mitigate the ongoing settlement of the roadway approaches, routine shimming of the pavement has been necessary. Even with the shimming work, the plaza has a noticeable slope approaching and leaving the plaza, with the roadways settling away from the pile-supported plaza. This approach settlement has created a range of adverse conditions, from low bed trailer hang-ups at the plaza to excessive settlement of the protective concrete bumpers in front of the booths, both resulting in safety concerns. Vehicles that become hung up on the plaza high point increase potential for vehicle accidents, and settlement of the concrete bumpers reduces the ability of the bumpers to absorb vehicle collisions increasing risk to toll plaza staff and patrons.

#### Proximity to Interchange

The proximity of the Chase's Pond Road interchange (Exit 7) located immediately south of the plaza presents undesirable safety and operational conditions for the plaza from both a traffic weaving and sight distance perspective. The Federal Highway Administration's (FHWA) recently published "State of the Practice and Recommendations on Traffic Control Strategies at Toll Plazas," recommends a one (1) mile separation between toll plaza and interchanges. The interchange southbound off ramp is less than 1,000 feet from the plaza and the northbound on ramp is less than 500 feet from the plaza. The proximity of these interchange ramps to the plaza creates traffic weaving issues, signing difficulty and driver confusion. MaineDOT has classified the York Toll Plaza in the northbound direction as a high crash location (2003-2005 crash data).

#### Vertical Geometry

The FHWA guidelines recommend toll plazas be located on a crest vertical curve. Locating the plaza on a high point will increase sight distance and provide operational benefits as the approach up-grade will aide in slowing vehicles and the departure down-grade will aide in accelerating vehicles.

The existing York Toll Plaza is located at the low point of a hill that begins just north of the plaza. This vertical geometry presents undesirable conditions with traffic departing northbound and approaching southbound. The northbound impact is primarily operational in nature, since the roadway north of the plaza includes a significant grade that impacts acceleration for departing vehicles, especially trucks. There is currently a truck climbing lane in this area to mitigate this

condition. The southbound approach represents a concern from the safety perspective since it is on a downgrade. This creates a condition where vehicles (especially trucks) must brake sooner to compensate for the downgrade in addition to the significant speed reduction required in the plaza area.

#### Horizontal Geometry

Recently published FHWA Guidelines recommend plazas be constructed on horizontal tangents instead of curves. Placement of plazas on tangents results in improved driver sight distance, awareness, and ultimately safety.

The York Toll Plaza was built on a horizontal curve. In addition to driver sight distance concerns, the curved roadway has an operational impact on the plaza, specifically in the southbound direction. Vehicles approaching southbound make a sweeping right turn approaching the plaza. This movement creates a tendency for southbound vehicles to travel through toll lanes on the outside of the curve (interior of the plaza) and reduces utilization of the tollbooths on the inside of the curve. Traffic that is not uniformly distributed in the plaza reduces operational efficiency, with some lanes over utilized and some underutilized. While a certain amount of non-uniform usage is common at plazas, the existing roadway curve exacerbates the skewed distribution.

#### Plaza Capacity

The original York Toll Plaza, along with past expansions, does not accommodate today's peak traffic loads. With the plaza constrained laterally by wetlands the only opportunities for expanding throughput are through the use of Electronic Toll Collection, installation of pass-thru satellite booths and/or tandem booths. While a number of efforts have been employed to increase capacity of the plaza, the current location is severely constrained from an expansion perspective.

One option for increasing capacity through the toll plaza is the use of Highway Speed Tolling. Utilization of this technology in a new Southern Toll Plaza is viable and will prove constructive by providing improved customer service, congestion relief, operational ease, and environmental benefits. However, due to the deficiencies described above, this technology cannot be installed at the existing York Toll Plaza.

#### Summary

In summary, the existing southern toll plaza at York is deficient in many areas and rehabilitation and expansion to accommodate current and future needs is not feasible. Therefore, a replacement southern toll plaza is required.

### APPENDIX C WHAT IS A TOLL PLAZA

#### **Components of a Typical Conventional Toll Plaza**

A conventional toll plaza consists of several main components: a toll booth on a concrete island, toll lanes, a canopy, and a tunnel. These are described below and shown in the following figure. Within these descriptions there are a number of additional items mentioned along with their purpose.

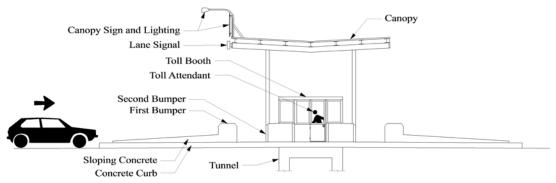
- 1. <u>Toll Lane</u> The toll lane allows the patron to drive through to pay their toll either with cash or E-ZPass. The lane is typically a minimum of 11' wide. There are different types of lanes at a conventional plaza including staffed, coin collection, and dedicated slow speed E-ZPass.
  - <u>Staffed Lanes</u> A staffed lane is attended with Turnpike personnel that collect money and make change.
  - <u>Coin Collection Lanes</u> This lane is not attended. There is a coin machine with a basket that drivers toss correct coin combinations into.
  - <u>Dedicated E-ZPass Lanes</u> This lane is not attended. Only drivers with an E-ZPass transponder are allowed to pass through at speed of 10 mph. Their transponder is read, allowing for proper toll payment, and a signal gives them an indication of acceptance. Drivers are not to stop in these lanes.

To maximize the efficiency of processing patrons, some lanes on the Turnpike have changeable signing that allows for lanes to switch between types. Regardless of lane types, all Turnpike toll plazas have a 10 mph speed limit for the immediate area before and after the plaza.

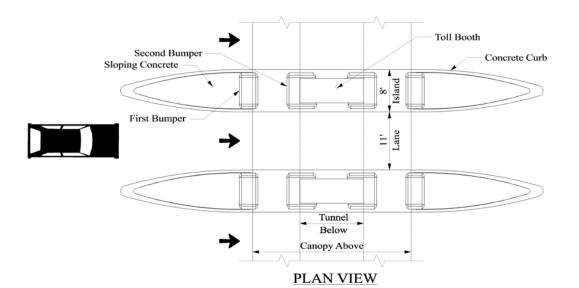
- 2. <u>Concrete Island with Toll Booth</u> A concrete island with curbing is provided to separate the toll booths from the toll lanes. The island functions much the same as a curb and sidewalk does to separate pedestrians from vehicles. The island also provides an area to house 'bumpers' and/or attenuators along with various tolling equipment.
  - <u>Sloping Concrete and the First Bumper</u> The concrete island is shaped to slope up to the first bumper and is intended to redirect the vehicle away from the toll booth back into the toll lane. The first concrete "block" is intended to stop a vehicle that hasn't been redirected, essentially protecting the toll attendant from errant vehicles approaching the toll plaza.
  - <u>Second Bumper</u> The second massive concrete "block" is the second line of defense from errant vehicles and is after the first bumper. This also surrounds the toll booth. If the errant vehicle gets past the first bumper, this bumper is intended to stop the vehicle.
  - <u>Attenuator and Guardrail</u> Installing impact attenuators followed by guardrail before the toll booth is an alternative to a system of bumpers and sloping

concrete that some agencies have adopted. The purpose of the impact attenuator is to slow down an errant vehicle or make it come to a complete stop by absorbing the vehicle's energy. The guardrail is meant to redirect the vehicle back into the lane.

- <u>Toll Booth</u> The toll booth is a weatherproof structure located on the island behind the system of bumpers. Toll collection equipment and heating / ventilation systems are housed in the toll booths. A toll attendant collects cash tolls from inside toll booths serving staffed lanes. Toll booths for coin collection lanes have coin machines for patrons to pay their tolls into.
- <u>Toll Attendant</u> The attendant is the Turnpike employee collecting cash tolls and making change for patrons as needed.
- 3. <u>Canopy</u> The canopy or "roof" covering the toll booths and toll lanes provides protection from the weather. The canopy must be able to support a snow load as well as signing, lighting, lane signals and tolling equipment that is mounted above and below the canopy.
- 4. <u>Tunnel</u> The weatherproof tunnel under the toll booths and travel lanes allows safe passage for Turnpike employees to access the booths and lanes. Tunnel access is provided on certain islands to minimize the number of toll lanes that personnel will have to cross. Personnel also use the tunnel to move the money collected to the toll plaza auxiliary building. Also located in the tunnel are electrical and communication lines along with heating / ventilation system components. Location of these utilities in the tunnel allows for easier access for repair and maintenance.



**ELEVATION VIEW** 



### APPENDIX D ONE-WAY TOLLING FEASIBILITY STUDY

## One-Way Tolling Feasibility Study

Prepared for the:

**Maine Turnpike Authority** 



**EXECUTIVE SUMMARY** 



#### Executive Summary

#### **PURPOSE**

The purpose of this report is to identify and assess the various impacts associated with the conversion of the York Toll Plaza to one-way tolling. The report will also identify the critical issues that the Maine Turnpike Authority (MTA) will need to address both in order to implement one-way tolling on the Maine Turnpike and to construct a new toll facility to replace the existing York Toll Plaza.

#### BACKGROUND

The MTA has considered one-way tolling at York toll plaza since the conversion to a closed barrier toll system in September 1997. One-way tolling essentially involves charging twice the fare in one direction, while making the other direction toll-free.

The concept of one-way tolling in this area came to the forefront in August 2003, when New Hampshire's Governor authorized the New Hampshire DOT to conduct a one-way tolling experiment at the Hampton toll plaza. One-way tolling trials were conducted at the Hampton Toll plaza in the late summer/fall of 2003 and again during the summer of 2004. However, New Hampshire plans no additional trials, nor has it identified permanent plans to convert Hampton Toll Plaza to one-way tolling.

Evaluation of one-way tolling at the York Toll plaza began in 2003 as a result of the Hampton Toll Plaza one-way tolling trials. This current evaluation incorporates the toll rate changes and conversion to the E-ZPass electronic toll collection system that became effective on February 1, 2005.

The MTA is currently planning to replace the existing York Toll Plaza in 2008. This study and its findings will be included in the overall evaluation of locating and constructing a new toll facility.

#### STUDY ASSUMPTIONS

The following are key assumptions used in the one-way tolling feasibility study:

- Vehicles at the York toll plaza are tolled in the NB direction only
- February 1, 2005 toll rates are the basis for calculating toll rates
- Cash fares at York toll are doubled in the NB direction
- Northbound ETC fares are nearly doubled, but continue to be less than or equal to the cash fares
- Commuter rates remain unchanged

#### RESULTS OF HAMPTON ONE-WAY TOLLING TRIAL

The New Hampshire DOT – Division of Turnpikes launched its first trial in one-way tolling in August 2003. On this date, the NHT doubled its northbound toll to \$2.00 while making southbound travel toll-free. The NHDOT continued its one-way tolling policy for 10 weeks, ending the trial in October 2003. A second trial was run in the summer of 2004 for approximately 15 weeks. The key results are as follows:

- Diversion around the northbound toll averaged 4.7%.
- Attraction to the Turnpike in the southbound (toll-free) direction averaged 2.3%.

#### ESTIMATED TRAFFIC IMPACTS

Traffic impacts, both at York Toll Plaza and on local roadways, were estimated for a one-way tolling condition. Traffic impacts included both diversion and attraction. Diversion is an important concern for

two reasons. First, diverting vehicles represent lost revenue to the Authority. And second, diverting vehicles may create problems for local roadways, since they impose additional demands on a network that already experiences peak-hour congestion. Attraction is a measure of the extent to which non-Turnpike users are drawn to the Turnpike when tolls are lowered. By drawing vehicles away from parallel routes and onto the Turnpike, attraction can provide relief to congested local roadways and intersections. Understanding attraction is important, because the benefits of attraction may help to offset the disbenefits associated with diversion.

The key findings of this analysis are summarized in the following bullets:

- HNTB estimates that the rates of diversion and attraction at York toll would be higher than the diversion at Hampton Toll Plaza. Over the course of a year, the average rate of diversion would be approximately 11.7%, while the rate of attraction would be approximately 10.0%. This equates to 5400 vehicles per day of diversion and 4600 vehicles per day of attraction.
- A conversion to one-way tolling could potentially improve operations at the York toll plaza. Currently, the plaza is at (or above) capacity during peak summer periods. If one-way tolling were implemented, then more lanes could be devoted to servicing northbound traffic, while southbound traffic could flow freely without having to stop. In short, one-way tolling would enable the existing plaza to meet the growing demand while avoiding summer backups. However, a new toll plaza is currently planned for 2008, which will alleviate these backups.
- A conversion to one-way tolling would also enable the Authority to construct a smaller, less costly plaza when the existing facility is replaced.
- HNTB estimates that the volume of traffic diverting around one-way tolling would be modest, and would be relatively consistent throughout the year.
- The combined impact of diversion and attraction will have a nearly negligible impact on key roadways and intersections along Routes 1 and 236.

#### ESTIMATED REVENUE IMPACTS

The foremost concern to the Authority is the impact one-way tolling would have on net revenue. It is estimated that an average of 11.7% of all northbound patrons would divert to an alternative route. These diverting patrons represent lost revenue. This raises a fundamental question: Can savings in capital and operating expenses offset this lost revenue? If not, the financial feasibility of one-way tolling will be in jeopardy.

HNTB has developed a revenue model for the Maine Turnpike to assess one-way tolling. This model incorporates current traffic volumes and historical trends as a means of forecasting how changes in rates could affect Turnpike revenue. The model accounts for all classes of vehicles, as well as all of the various payment programs (e.g. commuter, debit, and cash) that the MTA offers to its patrons. As part of this effort, a new set of one-way tolling rate tables were developed for all vehicle types and payment programs. These rates were then input into the revenue model, based on traffic volumes for 2004 the first part of 2005.

In 2005, the York toll plaza is projected to account for approximately **\$34.1 million** in toll revenue. A diversion rate of 11.7% would yield a revenue loss of **\$4.0 million**. This equates to an overall drop in revenue of **4.6%**.

#### ESTIMATED COST SAVINGS AT YORK

Conversion to one-way tolling would likely provide a cost savings that such a conversion might yield. These savings will come in three forms—labor, operations, and capital.

- **Labor Savings**. One significant opportunity for cost savings brought about by one-way tolling is the reduction of toll collection staff. By collecting tolls in one direction only, the number of toll collection staff can be reduced by approximately 30%.
- Operation and Maintenance Cost Savings. In addition to reducing the toll collection staff, a
  conversion to one-way tolling would also reduce operation and maintenance (O&M) costs at
  York toll plaza. These O&M costs include ETC Maintenance Costs, Toll Lane Equipment Replacement Costs, and Toll Plaza Maintenance (Canopy, lighting, signing, etc)
- Toll Plaza Construction Savings. Currently, the Maine Turnpike Authority 20-year plan calls for the replacement of the York Toll Plaza in 2008. The estimated cost to replace the York Toll Plaza as a two-way toll plaza is currently \$33 million. A new one-way toll plaza will cost significantly less, primarily due to its smaller size.

A summary of the estimate costs savings by converting to one-way tolling at York is identified in the table below.

	 -
Labor Savings:	\$ 800
Operations & Maintenance Savings:	\$ 166
Capital Savings:	\$ 1,050

\$

2,016

#### **Yearly Cost Savings Summary (2005\$)**

#### NET REVENUE

Based on these figures above, net revenue would decrease by about \$2.0 million if the MTA were to adopt one-way tolling. In other words, the savings in capital, operating, and maintenance costs would likely cover only about half of the projected loss in revenue.

Reduction in Costs (\$k)

#### **SAFETY IMPACTS**

Currently, the York Toll Plaza is a two-directional barrier toll plaza collecting tolls in both the northbound and southbound directions. All traffic, with the exception of ETC traffic, must stop and pay a toll. Toll collectors can access their booths either by walking across live lanes of traffic, or by using the tunnel that runs from the utility building under the entire toll plaza. Crossing lanes is the foremost safety concern of toll collectors. Visibility is limited, and ETC patrons—sometimes passing through the plaza in excess of 25 miles per hour—can use any lane. Other safety concerns include current crash rates at York toll plaza. York toll plaza is a high crash location (HCL) in both the northbound and southbound directions based on 2001-2003 MaineDOT crash data. An assessment of toll collector and vehicular safety impacts by converting to one-way toll collection concludes that:

- Conversion to one-way tolling at York is likely to have a greater positive effect on toll collector and vehicular safety.
- The existing high crash location at York toll plaza SB will be greatly improved. The majority of crashes occurring at York toll plaza are rear end/driver inattention crashes resulting from vehicles queuing to pay toll.
- Toll collectors will no longer cross non-stopping traffic to access toll lanes. However, the Authority should review this operational issue to ensure that all collectors can use the tunnel from the utility building to the toll lanes.

It should be noted that the construction of the new York Toll facility in 2008 is also anticipated to improve toll collector and vehicle safety through improved facility design, roadway geometrics, and ETC lane location.

#### OTHER POTENTIAL IMPACTS

Other relevant impacts that should be carefully evaluated as part of the one-way tolling feasibility assessment include:

- Toll Equity. One-way tolling at York would create a more uneven directional toll than exists in the current toll system. For example, a northbound Class 1 vehicle cash toll would be \$3.50 while southbound would be free. Directional toll differential has been a previous issue identified under other toll studies.
- **Toll Opportunity**. Doubling the toll at York in one direction may limit the ability to increase toll rates in the future. For example, where a \$0.25 increase previously may have generated sufficient additional revenue as part of a toll increase package, the required increase under one-way tolling would be \$0.50 to generate approximately the same level of revenue.
- Toll Elasticity. The recent toll increase, coupled with the flat growth at the York toll plaza, may suggest that the limit of the current toll elasticity is being reached. A \$3.50 Class 1 toll rate at a single toll location is similar in the northeast only at bridge or tunnel locations (Tobin Bridge, Tappan Zee Bridge, Holland Tunnel, etc.)
- Off-Site Improvement Costs. Specific off-site improvements are not identified as part of this study. Impacts of one-way tolling (due to diversion around the toll) may require costly local roadway enhancements such as intersection improvements (turn lanes, signalization).
- **System Anomaly**. A one-way toll plaza would create an anomaly in the existing toll structure where tolls have been historically charged in both directions at mainline plazas.
- **Air Quality**. Construction of a one-way tolling facility will reduce the number of stops by approximately ½. This is a small but quantifiable air quality benefit and a real public perception benefit

#### FINDINGS AND RECOMMENDATIONS

Based on the findings in this one-way toll feasibility study, HNTB recommends that the Maine Turnpike Authority cease further consideration of a one-way toll at the York Plaza. This recommendation is based on the following findings:

- Loss in Revenue. Implementation of one-way tolling is anticipated to result in a net revenue loss of approximately \$2.0 million dollars per year.
- Local Diversion/Traffic Impacts. The average rate of diversion by implementing one-way tolling is anticipated to be 11.7%. This equates to roughly 5400 vehicles per day shifting to local roads.
- Improved Safety from a new toll facility. One-way tolling would improve both vehicle and toll collector safety, but many of these safety benefits are also anticipated to be realized through the construction of a new toll facility in 2008.
- **Toll Equity/Elasticity**. One-way tolling at York would create a more uneven directional toll than exists in the current toll system and may reach the limit of toll elasticity at York.
- **Toll Opportunity**. Doubling the toll at York in one direction may limit the ability to increase toll rates in the future.

## APPENDIX E ALL ELECTRONIC TOLLING REPORT

## Maine Turnpike Southern Toll Plaza Initial All-Electronic Tolling Feasibility Review

Prepared for

Maine Turnpike Authority



Prepared by:



February 20, 2009

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#### **EXECUTIVE SUMMARY**

The York Toll Plaza was constructed in 1969 and was expected to be removed with the defeasance of the bonds in 1981. Since its construction it has undergone two expansions and has experienced four toll collection systems. The York toll Plaza processes 15.7 million vehicle transactions per year. A total of \$33 million or 41 % of the Turnpike's revenue was collected at York in 2008. Of the 15.7 million vehicles processed at York in 2008, roughly 12% were trucks, approximately half were from out of state and over 57% used E-ZPass.

In 2006, the Maine Turnpike Authority voted and approved the concept that the replacement York Toll Plaza would be built incorporating highway speed tolling for E-ZPass customers at the new plaza. Highway speed tolling (HST) would allow E-ZPass users to pay their tolls electronically while traveling at normal highway speed (55-65 mph). Cash paying customers would exit the mainline to pay their tolls. This decision was made after consideration of the potential benefits of HST such as: improved safety, congestion relief, customer service, and capital cost savings, all weighed against some of the business costs associated with probable revenue leakage.

As part of the alternatives analysis related to the York Toll Plaza project, HNTB was commissioned to review the potential for All-Electronic Tolling (AET), also known as cashless or full open road tolling. AET would eliminate all cash toll payments potentially using two methods. First, E-ZPass users would pay their toll as they would under HST as well as any former cash customers who would convert to E-ZPass as a result of the implementation of AET. Tolls would be collected from non-E-ZPass users through video tolling.

Since 2006, a few agencies in the US have either begun implementing or have set policy that future replacement facilities will be AET. A handful of agencies have begun conversion or have set policies that future installations will incorporate AET. A few more agencies have initiated extensive formal studies to evaluate the applicability of AET. Many agencies are mainly waiting to see the results of these agencies activities before conducting extensive assessments. It should be noted that although some agencies have committed to convert to AET, at the time of this review, no existing cash based agency has completed a total conversion to AET. Furthermore, there is very little standardization of reporting of the business impacts of AET and much reluctance on the part of those agencies involved in AET to release documented and audited results of the business impacts.

While the potential benefits of AET can be documented, the significant risk associated with the uncertainty behind the business costs of AET make the option of AET for the York Toll Plaza replacement not feasible. The following points elaborate on this risk:

- The ability to recover toll revenue from as much as 26 percent of the total traffic at York due to the lack of legislation that would compel payment from out of state patrons weighs significantly in this risk. This inability has perplexed toll agencies for over 10 years and we believe that this issue will not be cured in the next 20 years.
- The traffic mix of the Maine Turnpike is such that a significant number of patrons are non E-ZPass users and from out of state. The extent to which these customers would not migrate to E-ZPass and pre-paid video products is uncertain and these factors greatly influence business costs such as operating costs and revenue losses.
- The resulting toll and fee structure for an AET system could result in actual or perceived unfair distribution of payments between Maine and out of state customers. This results when out of state violators do not pay because there is no significant enforcement capability and the structure is set up or perceived to be set up to offset these losses by paying in-state patrons further compelled to pay because of threat of registration hold.
- Difficulties attributed to the duplicate license plate numbering system and the ability of video systems to recognize the myriad of different plate types present minor operational challenges.
- The current lack of industry data for similar roadways already implementing AET limits the ability to compare potential MTA outcomes makes forecasting difficult to calibrate.
- The uncertainty relative to how customers will respond to the changes in payment methods and the uncertainty relative to revenue recovery potential for violations pose too broad a range of potential outcomes. These include potentially significant risks to net revenue required to operate the roadway.
- The MTA may be limited in its ability to allow for certain types of post payment options typical for AET systems. For example, post payments of video tolls by customers are considered an extension of credit and any restrictions on how the MTA operates under these situations would need to be considered.

Greater certainty around the potential impacts to toll operating costs and revenue impacts resulting from AET would be necessary to determine if the range of risks can potentially be mitigated to an acceptable level or if the risks are insurmountable. Based on the cost analyses conducted, the range of risk to the MTA resulting from uncertainties related to AET over 20 years could be as high as \$400 million. Therefore, given the revenue risk associated with the stated uncertainties, HNTB does not recommend AET for the York Toll Plaza at this point in time, nor do we anticipate, given the significant concerns described herein, that AET would be prudent for York Toll within the next 20 years.

#### INTRODUCTION

In 2006, the Maine Turnpike Authority voted and approved the concept that the replacement York Toll Plaza would incorporate highway speed tolling for E-ZPass customers at the new plaza. Highway speed tolling (HST) would allow E-ZPass users to pay their tolls electronically while traveling at normal highway speed (55-65 mph) by simply passing beneath sensors on the mainline of the highway. Cash paying customers would briefly exit the mainline of the highway to pay their tolls at a more traditional plaza. This decision was made after consideration of the potential benefits such as improved safety, congestion relief, customer service, and capital cost savings, all weighed against potential business costs associated with probable revenue leakage.

As part of the alternatives analysis related to the project, HNTB was commissioned to review the potential for All-Electronic Tolling (AET), also known as cashless or full open road tolling, as an alternative to the currently planned highway speed and cash collection plaza. An AET option would eliminate all cash toll payments at the toll plaza. Turnpike customers originally with E-ZPass would continue to pay as they would under HST as well as any former cash customers who would convert to E-ZPass as a result of the implementation of AET. Tolls would be collected from non-E-ZPass users by capturing an image of their license plate, using their license plate number to either match pre-paid license plate accounts or identify the registered owner's address to send them a bill.

Since 2006, a few agencies in the US have either begun implementing or have set policy that future replacement facilities will be AET. Some of these agencies are start-up or "greenfield" toll roads while others are existing "brownfield facilities with established toll roads and customers. A handful of agencies have begun conversion or have set policies that future installations will incorporate AET. A few more agencies have initiated extensive formal studies to evaluate the applicability of AET. Many agencies are mainly waiting to see the results of these agencies activities before conducting extensive assessments. It should be noted that although some agencies have committed to convert to AET, at the time of this review, no existing cash based agency has completed a total conversion to AET and therefore there is little to no available information to assist other agencies with forecasting the applicability of AET for their own roadways. Furthermore, there is very little standardization of reporting of the business impacts of AET and much reluctance on the part of those agencies involved in AET to release documented and audited results of the business impacts. Considering the lack of information plus the broad range of local factors and the unique characteristics of each facility, a decision regarding AET cannot be based solely on what other agencies may be doing, but must consider the individual agency case in order to appropriately determine feasibility.

#### TOLL TECHNOLOGY BACKGROUND

Electronic toll collection (ETC) technology has been in use on major toll roads since 1988 and has grown significantly due to its convenience for the consumer/customer. Nearly every toll agency that has implemented ETC has shown positive impacts on vehicular throughput and customer service for toll collection. The development and public acceptance of ETC technologies have allowed toll agencies to rely less on cash collection and more on non-stop electronic toll collection. Initially in the 1990's there were some predictions of an eventual national interoperability standard that would unite ETC systems across the country by the turn of the century. In practice, there are several regional groups within the United States that have adopted interoperability requirements so that a single transponder can be used on any of the facilities that are part of that group but there is no national interoperability at this stage. The Federal Highway Administration along with several other coalitions and industry groups continue to pursue the development of a national standard that would tie into an overall vehicle to vehicle and vehicle to infrastructure communication system, but this schedule continues to be uncertain. Instead, regional interoperability has grown and the result has encouraged ETC use to continue to grow steadily while cash payments have declined.

The Maine Turnpike has used electronic toll collection since 1997, when Transpass, the first system in New England, was put into operation. In 2005, the Authority converted their electronic toll collection system to *E-ZPass*, allowing Maine and any customer of the 11 state Inter-Agency Group (IAG) to pay tolls electronically on the Maine Turnpike. This system provides the Maine Turnpike with a far-reaching *E-ZPass* user base and provides interoperability and a regional transponder distribution network that extends throughout the Northeast. The IAG has issued over 17.5 million active *E-ZPass* transponders throughout the northeast.

In addition to transponder based electronic toll collection, several agencies (such as agencies in Texas, Florida and North Carolina) have or are planning to implement some form of "video tolling" as an additional payment option for patrons. Video tolling represents the option for a customer to pay for the toll based on the capture of their license plate by a roadside camera at the toll plaza rather than purchasing a transponder. Video toll accounts are typically designed for less frequent customers who cannot justify the cost of a transponder based on the frequency of their trips to benefit from the lower cost per toll for ETC.

The variety of video toll accounts types typically fall into two categories, "pre-paid" and "post-paid". In the "pre-paid" account option, the customer would sign up for an account, much like an E-ZPass account, but instead of a transponder assigned, the customer provides a license plate number for the account. Pre-paid accounts could include the same options as the current ETC accounts, including debit or commuter plans, but they

can also include features such as period passes that allow unlimited travel within a window of time. However the account is set up, the cost of tolls (or fees associated with the toll) for pre-paid video accounts is typically higher than ETC rates to first cover the cost to review the images and any other appropriated operational costs (such as a percentage of unreadable image costs). Second, some agencies consider pricing the video toll transaction to encourage ETC participation to improve operating efficiencies, weighing frequency of travel with operating costs. "Post-paid" accounts can take on different forms also, including those similar to the pre-paid options, only handled after the travel occurs. For example, the customer could contact the MTA post-travel to pay the toll, set up a debit and/or commuter account, or purchase a period pass covering the timeframe. The primary consideration is "when" the post payment occurs. Options for post payment within a time window (such as 72 hours or one week) after travel via a phone call or website would present one option. The next would be post-payment upon receipt of an invoice for travel. Toll rates or associated fees are typically set to cover costs for each scenario, similar to the pre-paid cost structures.

Most toll plazas designed and constructed within the last 10 years in the United States have incorporated dedicated ETC lanes as part of the toll plazas. These lanes are dedicated solely to ETC patrons and are designed as either slow speed or highway-speed dedicated electronic toll collection. A detailed description of slow speed and highway speed dedicated ETC technology is presented in the HNTB report entitled, "Maine Turnpike Southern Toll Plaza Dedicated Electronic Toll Collection Lane Design Recommendations" dated July 27, 2006. As noted, the MTA is currently planning to incorporate highway speed tolling at the replacement York plaza. This decision was in part based on the referenced report.

#### All-Electronic Tolling (AET)

It is possible that All-Electronic-Toll collection (AET) will be employed on a number of toll highways in the future. The concept of AET, also termed "Full Open Road Tolling", "Full ORT" or "cashless" tolling has been incorporated in the long range plans of a number of toll agencies. AET is a concept where 100% of all tolls are collected electronically without the need for a conventional toll plaza. While the technology to implement cashless, AET toll collection currently exists, the conversion from a cash or cash/ETC-based toll collection system to AET requires the resolution of many difficult issues, most of which are non-technical.

Since the 2006 report, the number of toll agencies studying AET and in the process of opening, planning to open or converting existing systems to AET has increased. The common characteristics among the majority of these installations remains that the facilities are:

Primarily commuter roadways

- Primarily in-state user based
- Primarily ETC driven or ETC will be required of all users
- Heavily congested toll plazas

In addition to the above characteristics, another important factor is whether or not the project is part of an existing toll road ("brownfield" project), or part of a completely new toll road ("greenfield" project). For example, the conversion of existing toll roads in Texas and Florida to AET are all considered brownfield projects. New toll roads such as projects in North Carolina and Virginia are greenfield projects. Brownfield projects are faced with the additional challenges such as established cash payment options, driver expectations, and existing labor agreements and employees. Greenfield projects have the benefit of being designed from the beginning to incorporate AET based on understanding of the customer market, planning for operations and infrastructure, and setting local expectations early. For example, if the Maine Turnpike were considering a new roadway as part of their network and this roadway met the appropriate characteristics, this would likely represent a better candidate for AET than a brownfield portion of the existing system.

The Maine Turnpike currently does not share any of the characteristics common to agencies considering AET . By comparison, the Maine Turnpike is not a commuter roadway and approximately 50% of the vehicles entering the York Toll plaza and the Turnpike are from out of state. ETC penetration on the Maine Turnpike is only 50%. While this value is expected to grow towards the 80% range in the next 20 years full AET applications are expected to be higher still. Congestion levels are not significant with the exception of peak summer weekends in York and isolated ramp plaza locations during certain commuter hours.

The reason behind these common characteristics is risk. AET presents far greater risk in the collection of revenue. This is due to the fact that AET presents no restriction regarding who may use the roadway. As a result, the system is reliant upon video capture of sufficient information to assess the toll. The risks of this system include: correct video capture, availability of information regarding the vehicle and the legal ability to assess the toll and penalties in the instances of non payment. Three of the common characteristics listed above serve to significantly reduce this risk because of the consistent and /or known identity of the users. Even in the instance of the facility being a high commuter roadway with high ETC tag penetration the system can fail. The 407 ETR in Canada was the first full AET roadway. The 407 ETR meets the first two conditions listed with the roadway being the commuter roadway into Toronto and having in excess of 80% toll tag (ETC) utilization and 98.5% of the users being in province with no duplicate plate numbers between plate types. 407 ETR requires "heavy vehicles" (large commercial trucks) to use a transponder while passenger cars and light commercial vehicles have the option to pay by video tolling. Video represents about 20% of the transactions on 407 ETR. Currently,

there is a significant issue regarding toll collection of non toll tag users such that there is a severe revenue shortfall.

With regards to agency efforts to increase ETC percentages, a number of approaches have been tested or implemented by other agencies. In some cases, agencies (by direct action or through required construction) have limited the available cash payment lanes, resulting in delays to cash customers to encourage ETC participation. This approach must be carefully calculated as the resulting backups must be considered for potential safety conflicts with other traffic patterns, such as blocking through traffic on ramps or ramp access onto a facility. These methods of increasing ETC participation have not shown success.

The following page summarizes the toll agencies that have or will likely be utilizing AET. Note that the information available produces mainly high level characterizations of these facilities. In practice, the details behind certain types of data, such as net violations and recovery, are not readily available. Where applicable, HNTB is able to apply some experience with other agencies but only indirectly as an industry observation.

							Open Road Deployments					
		Existing Full ORT Facilities Proposed Full ORT Facilities					ORT/Managed Lane Configuration					
	Facility	407 ETR	Westpark Tollway	Crosstown Expressway Elevated Reversible	Central Texas Turnpike SH 183A & SH 130	NTTA DNT, PGBT, AATT, MCLB	Miami Dade Expressway SRs 924, 878, 874, 836, & 112	E470 NWP, CTE, E-470	Inter County Connector	North Carolina Turnpike Authority	SR-91	SANDAG I-15
	Location	Toronto, Canada	Houston, TX	Tampa, FL	Austin, TX	Dallas, TX	Miami, FL	Denver, CO	Montgomery County, MD		Orange County, CA	San Diego, CA
em	Characteristic											
1 F	Roadway											
		Rt 401 & QEW	US 59, Beltway, SH 6	I-4 and FL 60	I-35	Local commuter routes	Local commuter routes	I-25, I-70	MD 198 MD 28	Multiple projects	SR-91 GP Lanes	I-5
[]		Urban 4 lane	Urban 4 lane	Urban 3 lane reversible	Urban 4 lane	Urban 6 lane	Urban 3-5 lane	Urban	Urban 6 lane	T	Urban 4 lanes	Urban 4 lane
	Open-Closed System	Closed	Open	Closed	Open	Open	Open	Open - Mixed	TBD	Open - Mixed	Closed	Open - mixed
٦	Toll Movements (all - partial)	All	Partial	All	Partial	Partial (have tolled and untolled ramps onto and off of tolled main lanes)	Partial	All	TBD	Partial or All	All	Reversible section (all) and variable access (partial)
I		Double Gantry at every Entrance and Exit		Double Gantry at Toll Zone		Double (old) and Single Gantry (new)	Single Gantry		Under Construction TBD	TBD	One Double Toll Gantry (6 lanes at toll point)	Single Gantry
2 1	Traffic											
7	Total Transactions (Revenue)			\$550K/mo	\$9M (5 months May 07)	383,453,978 (\$223,894,096.65)			T&R Study in Progress			
E	ETC %			100%	85%	79		Approx 70%		Projected 80%	100%	100% on reversible sectio
١	√ideo Toll %			1%	15%	11				Projected 20%		
	Violation Rates		16% initially, then 2% (reported 2005)	unavailable (FTE)								Approx 14%
3 F	Patrons											
C	Dominuter - Casual	Mostly commuter & Light Truck	Commuter	Commuter	Primarily Commuter	Commuter primarily	Commuter and tourist	Commuter and Tourist	Primarily Commuter	Depends on project - mostly commuter	Filliarity Collinates	Primarily Commuter
	nstate - Out of state	Both, incl light, heavy single truck and heavy multiple trailer		Instate	Instate	Instate	Instate	Instate	Yes - Metro DC Area and Tourist	Depends on project - mostly in-state	Instate	Instate
4 F	Products											
7	Type of Transponders	MK IV - TDMA (ASTM v6)	TransCore eGo, ATA Transponders; TxTag, TollTag, EZ Tag	TransCore SunPass Transponder Allegro, ATA, eGo+	TransCore eGo, ATA Transponders; TxTag, TollTag, EZ Tag	TransCore eGo, ATA Transponders; TxTag, TollTag, EZ Tag	TransCore SunPass Transponder Allegro, ATA, eGo+	CA Title 21, TC and SIRIT	IAG	TBD	CA Title 21, TC and SIRIT	CA Title 21, TC and SIRIT
F		Fleet MGT Capability - specifically silent on Rental Car Accounts	Unk	(future)	Unk	Have agreement with Rent A Toll; working with Enterprise directly; working through statewide Interoperability for future PlatePass.	FTE SunPass accounts by some rental cars in place)	Unk	TBD	TBD	Unk	Unk
Ĺ	LPN Lists	Yes	Yes	Yes	Yes	Yes	Yes	Yes	TBD	Yes		
F		Light, Heavy single, Heavy multiple	Axle Based 2-6 axles	Axle Based 2-10 axles	2 Axle only	Axle Based 2 to 6+ axles	Axle Based 2 to 9 axles	Axle Based 2 to 9 axles	TBD		2-axle, HOV 3+, dynamic	2 axle, HOV 2+
C	Discount Plans	ETR Program provides Free Toll mileage and Gas Discounts; Heavy Vehicle Savings Program		none		Video toll have premiums above the transponder toll rate		Express Toll Reward Program - provides discounts and deals from local companies	TBD			
5 E	Enforcement											
		Video with ALPR		Video		Video with ALPR and manual review			TBD		Manual CHP and Video ALPR	Manual CHP and Video ALPR
		Yes including in US Yes - % or Amount Unk Yes - % or Amount Unk		yes unavailable (FTE) unavailable (FTE)		Some					Yes Yes - % or Amount Unk Yes - % or Amount Unk	
1	Instate Pursued?	Yes				Yes, once meet business rules					Yes	
	Revenue Recovered	Yes - % or Amount Unk		unavailable (FTE)			1				Yes - % or Amount Unk	
j	Fee/Fines Recovered	Yes - % or Amount Unk		unavailable (FTE)							Yes - % or Amount Unk	
L	Legal Restrictions etc					2 years from txn date to pursue through citation in JP courts, no limit on collection process						
6 ١	What Led to ORT Decision?											
( c r	capital cost, O&M costs, customer service, congestion relief)	Congestion relief in city center	Improve mobility in region	Relieve congestion (during peak periods - directional)		Improve mobility and Congestion relief; reduce operation and capital costs	Eliminate Cash Operations and reduce congestion	Improve mobility and Congestion relief	Improved mobility and Congestion relief	Overall and capital cost savings	SR 91 congestion relief between Orange County and Riverside County	Congestion relief in region
	Master Plan and/or Decision paper available?											
				Available		Conversion plan to be completed in June/July 2008	Available					

The toll lane level technology involved for AET is very similar if not the same as toll technology used for highway speed dedicated ETC lanes already approved for the replacement York Toll Plaza. The system would include overhead structures to support the placement of antennas and cameras to identify vehicles passing through the toll point. Other sensors would detect and classify vehicles to assign the appropriate toll point and these could be a combination of overhead mounted and pavement surface sensors.

While the benefits and cost considerations for AET are very similar to the decision to incorporate the option of HST, one fundamental difference exists. HST maintains an option for non-ETC customers preferring to use a stop condition form of payment, such as cash. AET is entirely electronic and eliminates the option to stop and pay by cash at the plaza. This distinction provides both benefits and costs worthy of careful consideration:

In conjunction with a decision to incorporate AET at future toll plazas, the Maine Turnpike Authority must also consider the following negative impacts:

- 1. AET will measurably increase operational costs for back office and the customer service center due to initial and ongoing customer education, additional post processing of transactions and increased violation image and notice processing.
- 2. Non-payment events at an AET plaza will likely increase due to patron confusion, technology limitations and increased scofflaws. Other toll agencies who have installed highway speed lanes or AET have typically experienced increases after conversion that lessens over time as a result of familiarization and enforcement. The issue of revenue collection has been discussed previously regarding scofflaws. The issue of collecting from patrons who infrequently use the roadway must also be considered as the cost to collect for one or two trips must be weighed against the available tolls and fees that could be charged.
- 3. Current limitations or lack of interstate agreements to enforce out of state toll violators limit the options for penalizing these violators. Without these agreements or laws, the Turnpike has few options to try to compel these violators to pay.
- 4. Improperly structured AET programs could result in a real or perceived subsidization of revenue by certain customers (for example, in-state patrons paying for out of state violators who do not pay). An AET program would need to be structured to minimize subsidization of tolls by certain groups of paying patrons at different points in the payment stream. For example, rates/fees/penalties associated with violations would need to be appropriately assigned to cover losses in that category due to lost revenue rather than having ETC or video rates set to offset a portion of losses due to violations. Global inefficiencies such as unreadable images would need to be distributed given an appropriate traffic assumptions.

- 5. Privacy concerns may emerge given that AET reduces the anonymous options for driver payments. Currently cash is exchanged with no record of the driver. An AET system may require anonymous account options to satisfy a portion of this concern. However, patrons who do not prepay with an account would be subject to identification via license plate lookup. The actual level of this concern is unknown and would need to be the subject of further understanding of patrons.
- 6. Regardless of the result of capital, operating maintenance and revenue impact costs and savings comparisons, consideration must be given for the potential equity or ethical concerns that could arise from the initial or sustained increases in non-payments expected under AET. The business case of cost savings would have to be weighed against the policy decision to accept that the potential that fewer patrons will ultimately pay the toll. More specifically, a system that allows higher revenue leakage but results in a net positive revenue over previous tolling regimes could still be viewed as inequitable or unethical since a larger portion of patrons are not actually paying the toll.
- 7. The capacity of local judicial processes is a potential concern if the judicial system is not set up to handle the additional cases resulting from AET. Advanced planning and coordination with the appropriate agencies would be necessary to determine costs and considerations needed as part of AET planning and implementation.
- 8. Unbanked customers (those without bank or credit card accounts) that prefer to pay cash at the point of tolling will find the cash option of pre or post paying with cash offsite as a burden.
- 9. AET may result in revenue decreases from increased diversion to local roads (some of which are already congested) as some patrons who perceive a lack of options to pay the toll that suits their preferences, seek alternate routes.
- 10. AET will require additional costs to increase transponder use, develop, market and implement new tolling products, as well as implement a significant public relations campaign to inform the public of the changes initially and ongoing education of future customers. The introduction of video tolling products and the removal of cash payment on the roadway will require significant public communication. Other products may include anonymous accounts to satisfy privacy concerns by some patrons.

- 11. Weather impacts to equipment are magnified with increasing reliance on video technologies. Significant snow or similar conditions may reduce the quality of images resulting in higher volumes of image rejections resulting in direct revenue losses.
- 12. AET may violate restrictions associated with existing bond covenants, trust indentures or similar agreements associated with the financing of the Maine Turnpike. For example, where bonds require toll revenues to meet certain thresholds, a higher amount of revenue loss under AET may require higher toll rates either initially or over a sustained period.
- 13. Consideration for labor agreements and the impact regarding AET implementation.
- 14. In some cases, the location for the construction of an AET plaza may not be conducive for the construction of a cash plus highway speed toll plaza given the different site requirements. If for some reason the plaza needed to be converted to add cash collection in the future, some AET plaza sites may restrict this option.
- 15. The conversion of only one location on the Maine Turnpike to AET while maintaining cash options at others may present confusion among patrons with regards to where payments options are available. Since cash lanes on the Maine Turnpike do not have enforcement cameras, if patrons assuming AET payment options pass through these lanes without stopping to pay, the Maine Turnpike would not realize this revenue.
- 16. Without fare collection staff at toll plazas, the Maine Turnpike will need to consider alternatives to handling wide load permits, which are currently a function served by fare collection staff.

With the challenges understood, the following beneficial impacts associated with AET include:

- 1. An AET toll plaza has the potential for greater safety due to the removal of any decisions required of the patron at the toll point. The goal of AET is a transparent roadway that reduces or eliminates any change to the driver's environment than what is typically encountered on other parts of the facility.
- 2. Under AET, all customers of the facility benefit from the convenience of not having to stop to pay the toll. Customers can either sign up for a transponder or opt

for other products such as pre-paid or post-paid video tolling options that could be offered by the agency.

- 3. AET toll plaza configurations minimize plaza construction capital cost by eliminating the need for toll booths that may require wider right of way and additional infrastructure.
- 4. AET toll plazas typically require less long term maintenance, since an AET plaza includes significantly less infrastructure.
- 5. AET eliminates the cost of fare collection staffing and support at the toll plaza.
- 6. Additional environmental benefits are possible with an AET plaza. By increasing the average speed of vehicles passing through the plaza, the average fuel economy of vehicles will increase. This quantifiable reduction in the use of fuel will not only provide financial benefits to the patrons, but reduce the consumption of nonrenewable resources.

An AET plaza would require patrons to either sign up for an E-ZPass account or pay via a pre-paid or post-paid video toll account. The MTA would need to consider pricing of such options would be matched to the frequency of the trip by the customer and cover administrative costs for each product. Pricing considerations can also go further to influence patrons to utilize more cost efficient products. Infrequent users who cannot justify the cost of a transponder would have the option to pay a video toll at a higher rate than the transponder rate but less than the cost of a transponder. Depending on the magnitude of the rate adjustment, larger portions of infrequent users would find the transponder option more financially practical. It may be expected that this adjustment may be as high three or more times the existing transponder rate in cases where patrons delay payment until an invoice or notice is received. While having the positive impact of driving patrons towards more cost efficient pre-payment options, this would likely have significant negative public acceptance issues.

#### DETAILED COST FACTOR DISCUSSION FOR ALL-ELECTRONIC TOLLING

As noted, the current direction of both industry technology and agency decision-making is to allow for the possibility of migration to AET under the right conditions. Some agencies are implementing AET on current projects or as in the case of the Maine Turnpike, considering this a future possibility in strategic planning activities. In addition to planning for the York Toll Plaza, other barrier toll plaza projects are under consideration in long range planning that will also consider HST and AET options. Each agency is faced with unique user and traffic features which will impact the consideration and viability of AET. The following discussion presents the benefits and costs in the context of the decision process for planning for AET.

#### **Capital Cost Considerations**

Plazas that incorporate staffed and/or cash collection along with considerations for ETC customers either through dedicated or highway speed lanes require greater infrastructure than those plazas that do not. The plazas require a larger right of way for pavement to support the widening for toll booths and traffic splits, as well as utilities, access and buildings to support the plaza staff. By comparison, an AET facility requires basically the same infrastructure as the highway speed tolling lanes of an HST toll plaza. At the center of the proposed HST plaza would contain a set of toll gantries over a section of roadway continuous with the mainline alignment. These gantries and equipment would be very similar to an AET toll point. The overhead structures, pavement footprint and toll equipment are basically the same. The state of the practice in the industry is to construct the highway speed lanes to match the approaching mainline configuration, allowing simpler transition to AET in the future although this may be modified dependent upon ETC utilization.

Based on the condition of the existing plaza, a capital cost estimate has also been performed to determine the amount of investment needed to refurbish the existing toll plaza. The following provides an initial estimate and comparison of the capital costs for each option. Both represent an average estimated cost for a new plaza location.

#### Capital Construction Cost Estimates for Plaza Options

	Existing	risting Highway Speed		AET	
Existing Plaza Demo	n/a	\$	2,500,000	\$	2,500,000
New Construction	\$ 14,300,000	\$	28,900,000	\$	4,400,000
	\$ 14,300,000	\$	31,400,000	\$	6,900,000

While the toll equipment and system for transponder users is essentially the same between the AET and highway speed systems, the development of and related system upgrades in order to support any new products such as pre-paid or post-paid video tolling

would be an additional cost to the AET system for the back office. These additional costs are not captured here.

#### Maintenance Cost Considerations

Because the highway speed plaza involves cash collection lanes as well as the dedicated ETC lanes, the annual maintenance costs will likely be higher. The life cycle costs require significant review as over time part of the cash collection infrastructure may morph into part of the ETC system. Annual maintenance includes additional building, plaza and roadway maintenance. Building maintenance would include items such as custodial, lighting, HVAC and other regular maintenances. Roadway maintenance would include snow and ice control for the additional plaza area as well as annual routine maintenance of pavements, plaza structures and plaza grounds.

In addition to routine maintenance, the non-routine (also known as reserve maintenance or renewal and replacement costs) items such as pavement rehabilitation, plaza area concrete maintenance and booth maintenance require budgeting in the later years of the facility. By contrast, the AET plaza does not require these additional costs because it does not include the cash plaza infrastructure. Both options require maintenance of the toll equipment. The highway speed option contains a larger amount of toll equipment because of the additional cash equipment, where as the AET system would require more maintenance of the backhouse operation, potentially involving more technical staff or expansion of contracted maintenance services.

The following estimates the maintenance requirements for both options. The cost of toll equipment maintenance for AET assumes a highest cost option, which would involve a separate vendor with full time on-site support. In practice, the use of the same vendor as the rest of the system or limited on-site availability could yield lower costs.

#### Estimated Annual Routine Maintenance Costs for York Plaza Options

	Current Plaza	Highway Speed	AET
Cash Plaza Maintenance	\$ 345,000	\$ 345,000	\$ -
Toll Equipment Maintenance	\$ 204,000	\$ 180,000	\$ 187,000
	\$ 549,000	\$ 525,000	\$ 187,000

#### Non-routine Maintenance Cost for Plazas with Cash Collection Infrastructure

Activity	Cost	Frequency
Concrete islands, slab and other surface sealing	\$106,000	Every 5 years
Approach pavement crack sealing	\$12,300	Every 8 years
Canopy roof sealing	\$53,000	Every 15 years
Complete approach pavement overlay	\$2.8 million	Every 15 years
Tunnel and slab rehabilitation	\$740,000	Every 20 years

#### **Operations Cost Considerations**

The cost to operate toll plazas for the purposes of this report includes the cost to staff the plaza and the cost of customer service and violations processing related to the plaza. Since the highway speed plaza sizing and staffing has not been finalized and ultimate impacts to overall MTA staff costs will be an MTA policy decision, this study starts by assuming a percentage reduction in staffing costs based on the most recent reduced number of cash lanes in the highway speed plaza compared to the current plaza. Since the AET plaza requires no on-site cash collection, the AET option is assumed have no on-site fare collection staffing costs. Depending on the capacity of current MTA back office staff, additional technical staff associated with the new toll system may be required offsite. It must be noted that the functions of toll collection are primarily transferred to the customer service and violations processing centers.

Both highway speed tolling and an AET option will increase the load on the customer service and violations processing costs to the MTA. Highway speed tolling is projected to have far less of an effect since a cash option will remain. The challenge with estimating the impact under the AET scenario is projecting the migration of the cash customers. Without any similar industry examples to compare to and without quantifiable information about the attitudes and willingness of MTA cash customers to migrate to certain products, the projection of operating costs carries the potential for significant variation and therefore risk. The risk in the case of the MTA is much higher since the characteristics of the roadways are so different. The other agencies share the benefits of high commuter usage, high ETC penetration rates and high instate constituency. The largest agency contemplating this change is the Port Authority of New York and New Jersey (PANYNJ). The risk for this agency is likely smaller than may be contemplated. The facilities of PANYNJ fit the common characteristics previously discussed with one other benefit. For example, the PANYNJ enjoys up to 80% market share (peak), and over 85% of plates are within jurisdiction. Being a duel state agency, PANYNJ has jurisdiction in both New York and New Jersey. This means they can assess fines for the largest amount of their users, all of the two states mentioned.

In order to estimate the range of this risk for the MTA given the limited information, two scenarios were considered. The first involves using limited MTA traffic pattern information (origin and destination studies or O&D) to estimate how cash patrons might migrate to certain products based on their frequency of use. This first "optimistic" scenario assumes that a significant portion of the transactions (but not patrons) will be handled as E-ZPass or video transactions under an all AET configuration. The second scenario presents a significantly more negative scenario in which all of the cash customers at the plaza migrate to the violation category. In other words, under this "pessimistic" scenario, none of the cash customers at the York plaza choose to sign up for E-ZPass or video tolling (pre-paid or post-payment before invoicing). This presents somewhat of a worst case and places a high end on the risk assessment.

The following represents the four categories of customers likely under AET:

- 1. E-ZPass customer (lowest risk of not collecting)
- 2. Registered video account (mild risk)
- 3. Unregistered video (more risk)
- 4. Violation (maximum risk)

Under the "optimistic" scenario, cash customer migration to ETC or video is based on trip frequency estimated from O&D study information. Current cash customers who use the Turnpike with greater frequency are assumed to migrate to one of these products for cost benefit reasons. The result of an evaluation of O&D data and estimates of patron trip frequency suggests that approximately 600,000-700,000 unique patrons use the Maine Turnpike. Based on trip frequencies of different patrons and based on payment type, it is estimated that approximately 225,000 unique patrons pay using E-ZPass, 350,000 pay with cash, and depending on the frequency of violations, 20,000-80,000 unique patrons violate. The cash users are further broken down in two groups, frequent and infrequent users. Based on the O&D data, it is estimated that roughly two out of three unique patrons travel less than once per week but at most six times per year. Because of their infrequent use, these individuals would represent approximately 10% of the cash transactions on the Turnpike. So for the purposes of estimating the increased volume of violation transactions to be processed by the violations processing center, this study conservatively assumes that 10% of the cash transactions at York (or 2 out of 3 current cash customers, not transactions, but unique customers of the Turnpike, based on estimated frequency of travel) will become violations. So the "optimistic" scenario assumes that 2 out of 3 unique cash customers on the Turnpike would choose to not pay the toll before receiving a violation notice. This would represent an approximate 150% increase in total non-payments at the toll plaza and an overall gross violation rate of 6.4%. This translates into additional staff required for the violations processing center to handle the additional volume of images from the system and process notices.

It is assumed that the majority of the rest of patrons (diversions are addressed later in the report), based on their estimated trip frequencies, will join E-ZPass, prepaid video tolling or post paid video tolling either via paying by phone or website within a certain window of time after traveling or by paying an invoice. These would include the one out of three unique cash patrons noted in the O&D observations above. These represent 90% of the cash transactions at York. Based on estimated trips per account, this additional volume would require additional customer service staff to manage the higher volume of E-ZPass or video accounts.

Under the "pessimistic" scenario, all cash customers (and their corresponding transactions) are assumed to migrate to the violation category. This results in a more straightforward calculation of the operating and revenue cost impacts, because the larger volume is simply applied to the current cost and recovery rates for the Maine Turnpike violations processing center. What is not assessed is the potential for increased violations due to the "their not paying why so I" scenario.

The following summarizes the additional staff estimated for each option to cover the additional costs of ETC, video tolling and violation processing followed by the additional costs for these increases in staffing.

Estimated	l Additiona	l CSC/VPC Staff
Loumance	i ziuuiuona	

	Highway Coad	AET	AET
	Highway Speed	Optimistic	Pessimistic
Customer Service Reps	1	12	2
Image Reviewers	1	3	25
Notice Processors	1	4	48
Clerical Staff	1	2	24
Total Additional Staff	4	21	99

The following summarizes the estimated total annual operating costs for the York plaza under each configuration. This includes the additional staff costs as well as direct costs. Direct costs include costs such as rent, utilities, postage, printing and credit card fees.

York Plaza Annual Operating Costs by Plaza Type

	Current	Highway	AET "Outimistis"	AET
		Speed Option	"Optimistic"	"Pessimistic"
Fare Collection	\$ 3,750,000	\$ 3,150,000	\$ -	\$ -
Base CSC Cost	\$ 507,000	\$ 507,000	\$ 507,000	\$ 507,000
Additional CSC Costs	\$ -	\$ 84,000	\$ 1,210,000	\$ 165,000

Base VPC Costs	\$ 137,000	\$ 137,000	\$ 137,000	\$ 137,000
Additional VPC Costs	\$ -	\$ 255,000	\$ 762,000	\$ 8,378,000
Total Annual Costs	\$ 4,394,000	\$ 4,133,000	\$ 2,616,000	\$ 9,187,000

#### Revenue Impacts

In order to estimate the revenue impacts of AET at the York plaza, an analysis of the current system-wide and York plaza leakage was developed. The current estimate was then used as a baseline for estimating the revenue impacts of highway speed tolling at York and AET (optimistic and pessimistic) at York. Since the analysis is based on the system-wide observations to develop the York portion, an estimate of the total system leakage for a system-wide AET deployment also results.

With the E-ZPass system-wide conversion in 2005 and with recent augmentations to the VPC process, the MTA has a robustly capable enforcement system with revenue recovery methods for the ETC lanes at the York Toll Plaza, in addition to the rest of the ETC and coin lanes throughout the MTA system for both in-state and out of state violators. Additionally, roughly half of the images captured are used to collect revenue from E-ZPass customers who, for a variety of reasons that are mostly due to patron behavior, are not captured via valid transponder transaction. The MTA is also currently pursuing in and out of state violations that meet MTA policy and thresholds.

Revenue leakage is defined for this effort by the transactions that ultimately do not result in a collected toll. A variety of factors can be attributed to revenue leakage and this effort focuses on where the leakage is occurring in the system and what impact the new toll collection methods will have.

#### Potential sources of revenue leakage on the Maine Turnpike

Lane Type	Leakage	Notes
ETC lane	Unreadable image -	Cannot pursue vehicles that cannot be identi-
	system	fied due to equipment error
	Unreadable image -	Cannot pursue vehicles that cannot be identi-
	patron	fied due to patron action
	Rejected image	Some images are rejected based on non-
		revenue vehicles such as state police cars
	Non-pursued trans-	The MTA does not pursue certain transac-
	actions	tions based on cost effectiveness thresholds or
		policies.
	In-state suspended or	In-state violators who do not pay violation
	waived violation	notices are moved to suspension and are not

		collected from. In practice, most of this cate-
		gory is recaptured but due to data limitations,
		this category is conservative included as loss.
	Out of state sus-	Out of state violators who do not pay viola-
	pended or waived	tion notices are moved to suspension and are
	violation	not collected from. This means the driver's
		right to operate in Maine is suspended how-
		ever, this is not enforceable in other states and
		therefore provides minimal leverage.
	Select out of state and	Due to limitations in some direct DMV ac-
	out of country viola-	cess, the MTA has limited options to cost ef-
	tors	fectively pursue some violators. In some of
		these cases, MTA utilizes access to data via
		State Police for these violators. For the pur-
		poses of this analysis, these are considered
		losses due to the lack of data history. In prac-
		tice, the MTA is actively seeking the majority
		of this revenue with some initial returns.
Manual Lane	Non-payments	Revenue not realized in manual lanes.

The current system leakage is estimated at the following based on MTA data and applied average toll rates. Note these are only approximate initial estimates based on average toll rates. Some variation could be expected due to higher volumes of trucks in one category or another, but this does provide an order of magnitude estimate at a minimum.

#### Current Estimated System-wide and York Plaza Revenue Leakage

	System-v	wide	York Plaza
Total net leakage as % of transactions	1.7%	\$1,500,000	\$560,000
Manual lane non-payments	1.1%	\$1,000,000	\$328,000
Non-pursued transactions	0.4%	\$330,000	\$138,000
Unreadable or reject images	0.1%	\$110,000	\$89,000
New Hampshire	<0.01%	<\$10,000	<\$5,000
Pennsylvania	<0.01%	<\$1000	<\$1000
New Brunswick	<0.01%	<\$5,000	<\$1000
In-state suspended or waived	<0.01%	<\$1000	<\$1000
Out of state suspended or waived	<0.01%	<\$1000	<\$1000

As the patrons shift as discussed in the Operations costs section, this also impacts the revenue leakage estimates. The following presents revenue leakage for the highway speed and AET options. Note that system-wide highway speed is not applicable at this stage

given not all locations would facilitate highway speed tolling and therefore the leakage factors would not apply to all locations.

## Highway Speed York Plaza Revenue Leakage for York Plaza

	York Plaza
Total net leakage	\$850,000
Manual lane non-payments	\$312,000
Non-pursued transactions	\$429,000
Unreadable or reject images	\$89,000
New Hampshire	<\$10,000
Pennsylvania	<\$1000
New Brunswick	<\$5,000
In-state suspended or waived	<\$1000
Out of state suspended or waived	<\$5000

# Estimated System-wide and York Plaza Revenue Leakage Under "Optimistic" AET Scenario

	System-wide		York Plaza
Total net leakage as % of transactions	4.2%	\$3,300,000	\$1,500,000
Manual lane non-payments	0%	\$0	\$0
Non-pursued transactions	3.5%	\$2,700,000	\$1,000,000
Unreadable or reject images	0.6%	\$500,000	\$400,000
New Hampshire	0.04%	\$46,000	\$25,000
Pennsylvania	<0.01%	<\$5000	<\$5000
New Brunswick	<0.02%	\$18,000	\$10,000
In-state suspended or waived	<0.01%	<\$5000	<\$5000
Out of state suspended or waived	0.05%	\$55,000	\$23,000

#### Estimated System-wide and York Plaza Revenue Leakage Under "Pessimistic" AET Scenario

	System-	wide	York Plaza
Total net leakage as % of transactions	45.6%	\$36,000,000	\$17,100,000
Manual lane non-payments	0%	\$0	\$0
Non-pursued transactions	38.8%	\$30,200,000	\$13,000,000
Unreadable or reject images	5.6%	\$4,300,000	\$3,400,000
New Hampshire	0.4%	\$520,000	\$277,000
Pennsylvania	0.04%	\$43,000	\$21,000
New Brunswick	0.17%	\$202,000	\$105,000
In-state suspended or waived	0.1%	\$61,000	\$19,000
Out of state suspended or waived	0.5%	\$620,000	\$254,000

#### Comparison of York Plaza Total Revenue Leakage under Each Scenario

	Current	Highway Speed	AET	AET
			"Optimistic"	"Pessimistic"
Total Leakage	\$560,000	\$850,000	\$1,500,000	\$17,100,000

In addition to the revenue impacts due to leakage, the estimates should also recognize a level of diversion from the toll plaza under the AET scenario. There were no significant estimates of diversion for this scenario, but as a point of reference, if 2.5% of the current cash customers at the York plaza choose to divert under AET, this would represent about \$400,000 in lost revenue. In addition, privacy concerns, technology aversion, and preference to pay cash are factors that must be considered as they will impact the outcome of diversion.

While leakage and diversion negatively impact revenue, the collection of tolls, fees and penalties under the violation process are also recognized. The following estimates the revenue recovery by the violations processing center.

#### York Plaza Total Annual VPC Revenue Recovery

	Current	Highway Spe	ed AET	AET
			"Optimistic"	"Pessimistic"
Annual Reco	very \$12,000	\$38,000	\$200,000	\$2,300,000

An AET plaza would require these patrons to either sign up for an E-ZPass account or pay via a pre-paid or post-paid video toll account. From an operating cost recovery perspective, the MTA would need to consider pricing of such options would be matched to the frequency of the trip by the customer and cover operating costs for each product. Pricing considerations can also go further to influence patrons to utilize more cost efficient products. So infrequent users who cannot justify the cost of a transponder would have the option to pay a video toll at a higher rate than the transponder rate but less than the cost of a transponder based on the infrequency of use. Depending on the magnitude of the rate adjustment, larger portions of infrequent users would find the transponder option more financially practical.

Note that specific toll revenue projections or revised rate structures are not part of the scope of this report. This report does assume, as a starting point of reference, that there will be some balance of cost recovery with the increased cost to process the customer options above. In other words (and subject to further discussion), pre and post paid video billing is assumed (for initial estimates) to be structured such that the net operating cost to the MTA is the same as processing ETC customers. So for the one in three cash customers identified as "frequent" users, the net cost to handle them will require the same staffing and direct costs as handling current ETC accounts. This introduces further discussions that will be needed relative to overall pricing of toll products, how each recovers costs to operate and how the pricing structure might be set to direct customers towards more cost efficient products (namely transponder based accounts).

The following summarizes the entire cost analysis for the options at the York plaza.

Total 20-Year Cost Summary for York Plaza (\$2008)\*

Current	\$ 132 million
Highway Speed	\$ 152 million
AET "Optimistic"	\$ 94 million
AET "Pessimistic"	\$ 494 million

<sup>\*</sup>Capital costs assume 20-year bonds at 4.75%. O&M costs factored in on annual or scheduled as needed basis. No cost inflation, changes in traffic volume, ETC penetration, violation rates assumed as this stage.

#### **Other Considerations**

In addition to the business costs, the Authority will also need to consider the other less tangible impacts that would result from the implementation of AET:

- 1. Regardless of business case, consideration may be needed for the potential equity or ethical concerns that could arise from the initial or sustained increases in non-payments anticipated under AET. For example, the current toll plaza does not collect approximately \$0.6 million due to revenue leakage. Under the "optimistic" AET scenario, this would potentially increase to \$1.5 million in uncollected tolls. The Maine Turnpike would be accepting an additional loss of approximately \$1 million annually to realize the one time savings of at least \$20 million in capital costs and maintenance and operating cost savings of up to \$2.1 million annually. Under the "pessimistic" AET scenario a substantial amount of the MTA revenue would be at risk. The business case of cost savings would have to be weighed against the policy decision to accept that fewer patrons will initially and ultimately pay the toll regardless of recovery efforts.
- 2. Consideration for any restrictions associated with existing bond covenants, trust indentures or similar agreements associated with the financing of the Maine Turnpike.
- 3. Consideration for current labor agreements and the impact to the timing of an AET implementation
- 4. Possible environmental credits for reducing emissions at toll plazas.
- 5. Safety benefits due to reduce conflict potential on the roadway.

#### CONCLUSION AND RECOMMENDATIONS

The reality of the circumstance is that it is very unlikely that the optimistic or the pessimistic scenario will occur. It is more likely that revenue leakage will be somewhere in the middle. This value however is significant and poses a grave threat to the Maine Turnpike.

While there may be theoretical benefits of converting a cash & ETC facility to AET, the significant uncertainty behind the business costs associated with AET coupled with the unique and quantified characteristics of the Maine Turnpike make the consideration of AET for the York Toll Plaza replacement not a feasible option at this point in time or in the 20 year planning horizon. The lack of industry data for similar roadways, the uncertainty relative to how customers will respond to the changes in payment methods and the uncertainty relative to revenue recovery potential for violations pose too broad a range of potential outcomes. These include significant risks to net revenue required to operate the roadway. Greater certainty around the potential impacts to toll operating costs and revenue impacts would be necessary to reduce the range of risks to an acceptable level for the further consideration of AET. Therefore, given the lack of comparable industry information to date and the revenue risk associated with uncertainties with patron behavior, HNTB does not recommend AET for the York Toll Plaza at this time, nor do we anticipate, given the significant risk described herein, that AET would not be prudent for York Toll within the next 20 years.

# APPENDIX F DEDICATED ELECTRONIC TOLL COLLECTION LANE DESIGN RECOMMENDATIONS

# Maine Turnpike Southern Toll Plaza Dedicated Electronic Toll Collection Lane Design Recommendations

Prepared for

Maine Turnpike Authority



Prepared by:



July 27, 2006

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#### **EXECUTIVE SUMMARY**

The Maine Turnpike Authority (MTA) is examining the options for resolving the need to address an aging existing York toll plaza. The current toll plaza was constructed in the 1970s and is well beyond the design life of the type of facility that was constructed. The current location not only suffers from aging and outdated facilities, the plaza also has deficiencies relative to layout and site conditions that need to be addressed. Technology has advanced significantly since the initial construction and efforts to retrofit the plaza have only provided temporary solutions to date. The York toll plaza is the busiest plaza on the Maine Turnpike, annually serving around 15 million transactions and collecting approximately \$34 million. These numbers represent 19% of all Maine Turnpike transactions but more importantly, over 39% of the total Maine Turnpike revenue. Initial estimates of the replacement cost of the plaza range from \$30-35 million (2005 dollars) with a design life of over 40 years. In short, the York Toll Plaza is a critical and valuable component of the Maine Turnpike and careful consideration must be made for any adjustments to how traffic and revenue is handled at this southern terminus of the toll collection system.

A fundamental decision prior to the detailed design of the project is the decision to incorporate either: (a) purely slow speed dedicated electronic toll collection (ETC) lanes, or (b) highway speed dedicated ETC lanes. The current York plaza, as well as many other MTA toll plazas, utilizes slow speed (10 mph) dedicated ETC lanes. The industry trend in the design of many new or replacement toll plazas incorporate highway speed (65 mph or similar) dedicated ETC lanes into the plaza design to take advantage of significant benefits associated with these designs.

#### The benefits associated with the highway speed dedicated lanes specifically include:

- A highway speed toll plaza has the potential for safety improvements due to the separation of non-stop from stopping traffic and reduction of exposure for workers in the plaza area.
- Highway speed configurations can help to **relieve congestion**. Operational efficiencies from highway speed lanes present opportunity to more cost effectively manage traffic congestion at tolling points.
- **Customer convenience increases** with highway speed options. All ETC customers have the opportunity to travel at the posted highway speed through the plaza rather than the current 10 mph speed limit.

- Highway speed lanes have the potential to **attract ETC customers** through the expanded benefits offered by the new option. A high ETC customer base leads to a larger population of users making the most of the benefits of ETC and improves operations for the road operator.
- The benefits of highway speed lanes have the potential to **divert cars from local** roadways.
- Highway speed toll plaza configurations are potentially more cost effective. Preliminary cost estimates show that the cost of more complex toll equipment and infrastructure for a highway speed plaza is more than offset by the savings of not building additional manual toll lanes to handle the same throughput capacity as the highway speed toll lanes.
- The **trend in the industry** is to construct highway speed facilities. It is more cost effective and less disruptive to customers to build a new plaza with highway speed toll lanes than to renovate a plaza in the future to accommodate highway speed toll collection lanes.
- A highway speed toll plaza has the potential to provide **benefits to the environment** due to increased fuel efficiency associated with maintaining a constant speed, reduced noise impacts and reduced emissions.

However, in conjunction with a decision to incorporate highway speed lanes at future toll plazas, the Maine Turnpike Authority must also consider the following potential increases to business costs:

- Highway speed lanes will potentially increase operational costs for back office and the customer service center due to initial and ongoing customer education, additional post processing of transactions and increased violation processing.
- Non-payment events at the plaza will likely increase due to patron confusion, technology limitations and increased scofflaws. Other toll agencies who have installed highway speed lanes have typically experienced increases after conversion that lessens over time as a result of familiarization and enforcement.

In light of these potential costs and benefits, HNTB recommends that the Maine Turnpike Authority incorporate highway speed dedicated ETC lanes into the design of the future mainline toll plazas. The projected benefits outweigh the modest increase in business costs associated with highway speed tolling.

In order to mitigate the potential increase in business costs related to highway speed toll collection, the following are recommended:

- Upon the introduction of highway speed toll lanes, the Authority should consider the required capacity to handle increased demands on back office operations related to highway speed operations.
- The Maine Turnpike Authority should conduct a specific review of the current violation enforcement practices and continue to evaluate potential options to further maximize revenue recovery.
- Future plaza design should include development and implementation of a clear and comprehensive signing plan and geometric layout to minimize patron confusion.
- Highway speed system specifications for future plazas should be comprehensive to insure the highest available accuracies of equipment.
- The Maine Turnpike Authority should consider a specific public awareness campaign relative to the use of highway speed lanes as designs are developed.

#### INTRODUCTION

The Maine Turnpike Authority (MTA) is examining the options for resolving the need to address an aging existing York toll plaza. The current toll plaza was constructed in the 1970s and is well beyond the typical design life for this type of facility. The current location not only suffers from aging and outdated facilities, the plaza also has deficiencies relative to layout and site conditions that need to be addressed. Technology has advanced significantly since the initial construction and efforts to retrofit the plaza have only provided temporary solutions to date. The York toll plaza is the busiest plaza on the Maine Turnpike, serving almost 15 million transactions annually and collecting almost \$34 million. These numbers represent 19% of all transactions but more importantly, over 39% of the total Maine Turnpike revenue. Initial estimates of the replacement cost of the plaza range from \$30-35 million (2005 dollars) with a design life of over 40 years. In short, the York Toll Plaza is a critical component of the Maine Turnpike and careful consideration must be made for any adjustments to how traffic and revenue is handled at the southern terminus of the toll collection system.

A fundamental decision prior to the detailed design of a solution is the decision to incorporate either: (a) purely slow speed dedicated electronic toll collection (ETC) lanes, or (b) highway speed dedicated ETC lanes. The current York plaza, as well as many other MTA toll plazas, utilizes slow speed (10 mph) dedicated ETC lanes. The industry trend in the design of many new or replacement toll plazas incorporate highway speed (65 mph or similar) dedicated ETC lanes into the plaza design to take advantages such as safety improvements, customer benefits, and operational efficiencies. This report will present these factors and provide a recommendation on the use of highway speed dedicated lanes. This document is only part of the beginning of the comprehensive process to evaluate options and recommendations. Further detailed evaluations and related activities as required will follow; including, but not limited to: location and need analyses, environmental permitting, and public involvement, as well as detailed design and cost estimates.

#### TOLL TECHNOLOGY BACKGROUND

Attended toll lanes are labor intensive and inconvenient for customers. Consequently, electronic toll collection (ETC) technology has been in use on major toll roads since 1988. Nearly every toll agency that has implemented ETC has shown positive impacts on vehicular throughput and customer service for toll collection. The development and public acceptance of ETC technologies have allowed toll agencies to rely less on cash collection and rely more on non-stop electronic toll collection. There are several regional groups within the United States that have adopted interoperability requirements so that a single transponder can be used on any of the facilities that are part of that group. Interoperabil-

ity has encouraged ETC use to continue to grow steadily while cash payments have declined. Some facilities are now completely ETC.

The Maine Turnpike has used electronic toll collection since 1997, when Transpass was put into operation. 1n 2005, the Authority converted their electronic toll collection system to *E-ZPass*, allowing Maine and any customer of the 11 state Inter-Agency Group (IAG) to pay tolls electronically on the Maine Turnpike. This system provides the Maine Turnpike with a far-reaching *E-ZPass* user base and provides interoperability and a regional transponder distribution network that extends throughout the Northeast. The IAG has issued over 16 million *E-ZPass* transponders throughout the northeast.

Most toll plazas designed and constructed within the last 10 years in the United States have incorporated dedicated ETC lanes as part of the toll plaza. These lanes are dedicated solely to ETC patrons and are designed as either slow speed or highway-speed dedicated electronic toll collection. The following is a brief description of both methods:

#### Slow Speed Dedicated Electronic Toll Collection (10 mph)

The Maine Turnpike currently uses slow speed dedicated ETC lanes at numerous plazas, including the York toll plaza. Typically at toll facilities across the country, vehicles speeds within a plaza area are limited to 5 to 15 mph for safety reasons and depending on local laws. Toll lanes dedicated solely to electronic toll transactions are located within the plaza, and users of these lanes are expected to also decelerate to the posted speed. These vehicles then must accelerate while merging with the other attended toll lanes back to the typical roadway section. These lanes provide the advantage of being reserved for electronic toll ONLY thereby improving throughput.

#### Highway Speed Electronic Toll Collection (65 mph)

Highway speed electronic toll collection allows a vehicle to operate at the posted highway speed through the toll plaza area. This not only increases customer convenience, but it also provides for more efficient operation of the toll plaza. This method of toll collection requires physical separation from the attended lanes since the operating speeds of the attended lanes and the highway speed electronic toll collection are dramatically different. The separation should extend an adequate distance from the plaza area to allow the users of the attended lanes to accelerate close to the posted speed of the highway prior to merging with the highway speed lanes.

Many toll agencies have implemented highway-speed ETC lanes. These implementations have involved reconfiguring existing toll plazas, reconstructing existing plazas, or designing and constructing new facilities.

The following list summarizes the facilities that have incorporated highway speed ETC lanes over the past 10 years.

- San Joaquin Hills Transportation Corri- President George Bush Turnpike dor (Southern California)
- ➤ Eastern Transportation Corridor (South- ➤ Orlando Orange County Expressway ern California)
- Foothill Corridor (Southern California)
- > Pennsylvania Turnpike
- Oklahoma Turnpike
- ➤ Dallas North Tollway (Dallas)
- Sam Houston Toll Road (Houston)
- ➤ Hardy Toll Road (Houston)
- US 183A (Austin, TX)
- Port Authority New York and New Jersey

- (Dallas)
- **Facilities**
- ➤ Delaware DOT Facilities
- ➤ Atlantic City Expressway
- New Jersey Turnpike
- ➤ Garden State Parkway
- ➤ Georgia 400
- ➤ Florida Turnpike Facilities
- ➤ Illinois Tollway Facilities

These facilities did not necessarily have significant ETC participation rates to justify the selection of highway speed ETC lanes. Several of the facilities had ETC participation rates of less than 50%, but the customer service benefits outweighed the perceived need for high ETC usage. The customer response has been overwhelmingly positive on all facilities that have implemented highway speed ETC lanes. According to New Jersey Turnpike Authority data, about 95% of users prefer highway speed lanes to slow speed dedicated lanes. In addition, the capacity increase and (in some cases) the resulting reduced size of the toll plaza provided additional benefits to the agencies.

Many toll agencies have incorporated full Open Road Tolling (ORT) into their longrange plans. ORT is a concept where tolls are collected 100% electronically without the need for a conventional toll plaza. Technology exists today to implement cashless, ORT toll collection; however, the conversion from a cash or cash/ETC-based toll collection system to full ORT requires the resolution of many difficult issues, most of which are nontechnical. Only 2 ORT facilities operate in North America: WestPark in Houston and 407ETR in Toronto. These are commuter-based toll facilities and were designed and opened as ORT toll roads.

#### DEDICATED LANE COMPARISON

The following is a summary of the two types of options reviewed for the design of dedicated lanes at a new toll plaza as well as a discussion of the advantages and disadvantages of each. From a cost perspective, initial review of conceptual costs estimates that the overall plaza construction costs would be similar. Slow speed plazas may require more staffed booths to achieve the same throughput as highway speed facilities. The additional cost of booths is generally roughly equivalent to the cost of additional equipment and pavement required for a highway speed facility.

#### **Slow Speed Dedicated ETC Lanes**

This toll system is currently utilized at the York Toll Plaza. Dedicated lanes on the outside of the toll plazas are separated from the adjacent toll lane by a curbed concrete island. In addition, two interior toll lanes can be signed as dedicated electronic toll lanes as conditions warrant.



#### Benefits:

- □ All vehicles approaching the toll plaza maintain the same alignment until reaching the toll plaza approach zone, reducing the need for patron decision making.
- □ Requires similar footprint per lane as existing toll plaza configuration.
- □ Limited merge distance required since all vehicles operate at similar speeds
- □ Similarity to existing conventional toll plazas leads to patron familiarity

#### *Limitations and Considerations:*

□ Electronic toll vehicles must slow as they enter the toll plaza area. While this is an improvement over the stop condition, slowing down to 10 mph is less ideal from a customer and operations perspective when compared to a highway speed lane.

- ☐ Insufficient deceleration by low speed dedicated lane toll users can create an unsafe situation in which the ETC vehicles approaching the toll plaza area at a relatively high rate of speed while all other vehicles are stopping
- □ Vehicles must access the dedicated toll lanes via the toll plaza approach area. Excessive vehicle queue in the approach area impacts access and efficiency of dedicated toll lanes.
- □ Current state of the leading industry technology allows highway speed tolling.

#### Highway Speed Dedicated ETC Lanes

Highway speed dedicated toll lanes are currently not used on the Maine Turnpike. Highway speed dedicated lanes would be designed to physically separate the majority of ETC traffic from the cash customers, resulting in operational, safety and customer satisfaction improvements. Given the higher speeds of a portion of the traffic passing through, considerations for plaza layout and approach roadways are required to safely transition the vehicles between these significantly different transaction conditions.



Regardless of configuration, highway speed dedicated lanes provide the following advantages and disadvantages:

#### Benefits:

- □ Separation of non-stop and stopped vehicles reduces potential conflicts within the plaza booth area
- □ Significantly reduces the number of non-stop vehicles in the cash collection area where toll collectors and other employees may be crossing
- □ Safe higher speeds lead to more efficient operation and reduced congestion.
- □ Increases throughput capacity of the plaza, potentially reducing the number of booths required
- Provides ETC customers with specific at-speed lanes with no queuing or speed reduction. This provides the best possible level of service for ETC customers.
- □ Provides increased incentive to participate in ETC program through the added convenience of the highway speed tolling.
- □ Potentially diverts additional users to the roadway from local roads as compared to conventional plazas due to increased customer convenience.
- □ Reduces fuel consumption, vehicle emissions and noise due to higher average speeds through the plaza and reduced braking and acceleration.

#### *Limitations and Considerations:*

- □ Will likely increase the non-payment rate through the plaza
- □ Less communication with the patron regarding tag status
- ☐ Increased cost of toll and violation detection equipment
- ☐ May eliminate the ability to implement reversible lanes

#### BENEFIT AND COST DISCUSSION OF HIGHWAY SPEED DEDICATED LANES

As noted, the current direction of both technology and agency decision-making is towards the use of highway speed tolling. While each facility presents unique user and traffic features, the overriding commonalities of increased customer service, improved operational efficiencies, and enhanced safety have generally compelled agencies to implement highway speed tolling. The following discussion develops the benefits and costs in the context of the decision for the layout of the future southern toll plaza.

#### Benefits of Highway Speed Tolling

The current York toll plaza serves as a gateway to the State of Maine for travelers on Interstate 95. These travelers include a combination of commuters, local trips and out of state visitors. The plaza clearly shows peak traffic volumes in the traditional recreation and vacation periods, further demonstrating the emphasis on use of the plaza as an entry point for tourism. Improvements to the operation of the York toll plaza will ensure that it does not function as a barrier to tourism. Any efforts to improve the quality of service to customers traveling through the plaza therefore have the potential to enhance a key component of the State's economy. Highway speed tolling clearly reduces or eliminates the need for ETC patrons to adjust their driving behavior when passing through a plaza. The customer is allowed to continue through at highway speeds rather than the conventional plaza speed of say 10 mph. Patrons are not required to slow down or negotiate slowing or stopped traffic. The more "transparent" the system, the less impact is to the patron and the quality of service increases.

In addition to the added convenience for ETC customers, cash paying customers will also see benefits of the new configuration. Since a large portion of traffic will have the option to utilize the highway speed lanes, fewer vehicles will enter the slow speed portion of the plaza. Customers who continue to choose to pay cash or use slow speed lanes for ETC will still encounter fewer vehicles in the payment area. This provides fewer conflicts as noted in relation to the safety benefits, but also reduces the number of decisions required of the driver. Also, the slow speed area of the plaza will have fewer lanes with ETC only modes, reducing the potential that a cash customer mistakenly enters a slow speed dedicated lane. Signage and lane types will be similar to previous plaza designs, adding consistency and familiarity to the plaza that will additionally benefit cash customers.

While often difficult to forecast and quantify, the potential also exists for increased incentive to participate in an ETC program given the higher level of service to customers. Also, the increased convenience may also persuade drivers to use the Maine Turnpike as opposed to alternative local routes.

The cost of toll equipment that allows the identification of vehicles at high speed and the capture of images of violating vehicles is higher than the cost of conventional slow speed lane equipment. This is primarily due to the more complex sensors, computer and camera equipment required. Furthermore, the cost of additional pavement and other physical infrastructure to separate highway speed traffic from slow or stopped traffic also presents additional capital costs. However, operational efficiencies can be realized given the increased throughput capacities of highway speed lanes that reduce the overall number of slow speed lanes required. An initial analysis of the mix of Maine Turnpike traffic as it relates to the projected sizing of both highway speed and conventional toll plazas shows that a conventional plaza will require more slow speed lanes than a plaza incorporating highway speed lanes. Cost estimates of the various options shows that the additional costs of highway speed toll equipment and infrastructure is more than offset by the cost of the additional toll structures for a conventional plaza. Current cost estimates show that regardless of configuration, the new plaza would cost between \$30-35 million (2005 dollars), with conventional plazas typically on the higher end of the range. Moreover, as overall traffic continues to migrate towards the use of ETC, the efficiency of the highway speed plaza increases over time, further presenting opportunity for operational savings in the long term.

One clear advantage of the highway speed toll plaza configuration over the conventional slow speed condition are the environmental benefits realized from highway speed tolling.. By increasing the average speed of overall vehicles passing through the plaza (since a greater number of vehicles will be able to continue at highway speeds) the average fuel economy of vehicles will increase. This quantifiable reduction in the use of fuel will not only provide financial benefits to the patrons, but reduce the consumption of non-renewable resources. Fewer vehicles decelerating and accelerating has the potential to reduce overall noise impacts at the plaza and reduces the emissions in the area due to lower residence times of vehicles in the plazas (since many will pass through quicker). Reducing air emissions has the potential to improve the air quality for plaza workers, passing vehicle cabin air intakes, surrounding communities and environments over a conventional plaza.

Finally, while specific safety studies and toll plaza design configuration standards have been limited, there is an overall trend in the industry to consider the potential safety implications of toll plaza design. High profile accidents at toll plazas have created renewed industry emphasis focusing on aspects of toll plazas that contribute to or reduce conflicts. Similar to the separation of local road traffic from highway speed through traffic in roadway networks in general (such as interstate bypasses around developed areas), there is increasing emphasis on the physical separation of toll plaza traffic that can continue at speed via electronic toll collection from the vehicles who are required to stop and pay cash. This concept of separation also moves traffic away from plaza areas with pedestrian activities (toll collectors and workers) in the lanes. Fewer vehicles in these lanes result in

fewer potential conflicts, reducing worker exposure. These potential safety benefits are key factors when considering basic toll plaza configurations.

#### Business Cost Considerations of Highway Speed Tolling

A potential cost of the incorporation of highway speed lanes in the center of a toll plaza relates to the inability of the plaza to incorporate reversible toll collection lanes in the center of the plaza. For facilities that experience significant differences in peak flow volumes by direction, the use of reversible lanes provides operational efficiencies with fewer booths. Recent trends in the peak flows at the current York toll plaza have shown directional peak flows approaching equalization in both directions. Initial analysis has shown that in peak conditions the future plaza would benefit from having at most a single lane, if any at all, that would be reversible. In short, the reversible lane option does not provide significant operational efficiencies, particularly when compared to the improved throughput of a highway speed lane.

Toll agencies who have incorporated highway speed lanes have realized varying levels of increases in non-payment events at these newly configured toll plazas. These increases have a variety of reasons, mainly centered on the lack of patron recognition of the new plaza configuration, limitations of the toll tag reading technology and increases in scoff-laws. Regardless of the reason for the increase of non-payment events at these types of plazas, the technology for capturing images of vehicles who do not register a payment is sound and proven to accurately capture license plates of vehicles in the highway speed tolling environment. Regardless of whether the patron mistakenly entered the highway speed lane, the patron's toll tag was not read or the patron was emboldened by the opportunity to violate at highway speeds, the Maine Turnpike can specify a new system which will reasonably identify the license plates of vehicles involved in non-payment events to maximize revenue recovery potential.

While the current industry trend has been towards the use of highway speed lanes at new or renovated toll plazas, if incorporated in Maine, the concept would be new to many patrons. As other agencies have experienced, the addition of a new toll plaza configuration will require additional design considerations to mitigate confusion; including, but not limited to specific signing and geometric layout considerations. The introduction of a new toll plaza configuration is also typically accompanied by significant public relations campaigns to educate patrons.

Since highway speed lanes typically do not provide feedback to individual patrons passing through the toll zone, accommodations for those who wish to receive feedback from a patron fare display (as currently used in Maine Turnpike plazas) or similar device could still be achieved by allowing those patrons to use their tags in the slow speed lanes. While

this population of users tends to be very low, agencies have recognized that this is a factor that is easily considered by accepting ETC in all lanes.

For those ETC customers who forget to mount their toll tag or have a tag the system fails to read for some reason, the MTA will be able to continue to use its automated processes to accurately charge these existing customers. For those additional actual violation events, the MTA will need to continue to be diligent in the pursuit of violators as the current laws allow and continue to evaluate options for violation recovery through continuous improvement of the current violation enforcement system and policy as appropriate and available. Through the optimization of the violation enforcement process and the maximization of opportunities for revenue recovery, the Maine Turnpike has the potential to reduce the impact of these additional violations to levels to lowest possible level.

As part of the initial broad assessment and one of the many design options under consideration, one compromise between the desire to incorporate highway speed lanes and the need to minimize preliminary revenue impacts would be to design a 'convertible' plaza. The design would be initially constructed as a conventional plaza with consideration for conversion to highway speed lanes in the future at a time when the revenue impacts would be further reduced. Initial estimates of the cost of a convertible plaza from a capital perspective alone would result in an additional approximate \$4 million (2005 dollars) in conversion costs in the future, not to mention additional disruptions to traffic due to additional construction activity in a relatively short period of time following the initial construction of the plaza. In the spirit of improving the gateway to Maine and given the magnitude of the additional capital costs, this concept, while worth noting, was not deemed appropriate.

#### RECOMMENDATIONS

HNTB therefore makes the following recommendations:

- The Maine Turnpike Authority should incorporate highway speed dedicated ETC lanes into the design of future mainline toll plazas. The projected benefits outweigh the modest increase in business costs associated with highway speed tolling.
- In order to mitigate the potential revenue impacts related to highway speed toll collection, the following is recommended:
  - Operational considerations. Upon the introduction of highway speed toll lanes, the Authority will need to consider the required capacity to handle increased demands on back office operations related highway speed operations.

- Enforcement process evaluation. In order to offset potential increases in revenue loss due to increased violations associated with the introduction of highway speed lanes, the Authority should conduct further assessment of the current violation enforcement practice and policy to determine if any modifications would be warranted based on the operational costs, public response and potential legislative requirements that may accompany such modifications.
- Signing. Development and implementation of a clear and comprehensive signing plan to guide patrons in advance of the toll plaza will help reduce confusion.
- o *Geometrics*. Design the entrance to the highway speed portion of the toll plaza as a "split" rather than an "exit", with an identical division for both the highway speed lanes and the conventional toll plaza. This should reduce confusion among patrons.
- Ocomprehensive specification and system testing. Limiting the errors introduced by technology can be in part mitigated by comprehensive specification of the highway speed system and rigorous testing to ensure the requirements are met. While no technology delivers a 100% accurate system, these efforts have the potential to minimize loss due to technology.
- o *Public awareness*. Inform the public of the conversion through a proactive public relations campaign. This will not only further reduce confusion, but it can help build public support for the improved facility as well.

# APPENDIX G CRASH DATA

## **Crash Summary Report**

#### Report Selections and Input Parameters

#### **REPORT SELECTIONS**

#### **REPORT PARAMETERS**

Study Period: Year 2004, Start Month 1 through Year 2006 End Month: 12

Input Data: Route 0095S First Node: 58357 Last Node: 58356

Exclude First Node: No; Exclude Last Node: No

Start Offset: 0; End Offset: 0

#### **REPORT DESCRIPTION**

I-95 SB York

# Crash Summary I

			No	odes										
Node	Route - MP	Node Description	U/R	Total		Injury	y Cra	shes		Percent	Annual M C	Crash	Critical	CRF
				Crashes	K	Α	В	С	PD	Injury	Ent-Veh	Rate	Rate	
58357	0095S - 293.72	Non-Int I 95 SB	1	0	0	0	0	0	0	0.0 State	8.439 wide Crash Rate	0.00 0.03	0.10	0.00
57693	0095S - 295.23	Non-Int I 95 SB	1	1	0	0	0	0	1	0.0 State	0.000 wide Crash Rate	0.00 e: 0.03	0.00	0.00
58871	0095S - 295.48	Int of I 95 SB, RAMP B OFF TO YORK CONNECTOR	1	0	0	0	0	0	0	0.0 State	8.439 wide Crash Rate	0.00 e: 0.03	0.10	0.00
58869	0095S - 295.89	Int of I 95 SB, RAMP A FROM YORK CONNECTOR	1	1	0	0	0	0	1	0.0 State	10.541 wide Crash Rate	0.03 0.03	0.09	0.00
58356	0095S - 296.30	BRG 6228, I 95 SB under ST RTE 91	2	0	0	0	0	0	0	0.0 State	10.541 wide Crash Rate	0.00 e: 0.12	0.26	0.00
Study \	Years: 3.00	NODE TOTA	ALS:	2	0	0	0	0	2	0.0	37.960	0.02	0.11	0.16

# Crash Summary I

							Section	ns									
Start	End	Element	Offset	Route - MP	Section	J/R	Total		Inju	ry Cra	ashes		Percent	Annual	Crash	Critical	CRF
Node	Node		Begin - End		Length		Crashes	K	Α	В	С	PD	Injury	HMVM	Rate	Rate	
57693 Non-Int I 95	58357 5 SB	239222	0 - 1.51	0095S - 293.72 INT 95 SB	1.51	1	27	0	0	4	2	21	22.2 State	0.12743 wide Crash Ra	70.63 te: 63.57	95.48	0.00
57693 Non-Int I 95	58871 SB	239223	0 - 0.25	0095S - 295.23 INT 95 SB	0.25	1	11	0	0	0	1	10	9.1 State	0.02110 wide Crash Ra	173.80 te: 63.57	137.31	1.27
58356 BRG 6228,	58869 I 95 SB ur	239734 der ST RTE 91	0 - 0.41	0095S - 295.48 INT 95 SB	0.41	1	8	0	0	2	1	5	37.5 State	0.04322 wide Crash Ra	61.70 te: 63.57	116.75	0.00
58869 Int of I 95 S CONNECT	*	240305 A FROM YORK	0 - 0.41	0095S - 295.48 INT 95 SB	0.41	1	1	0	1	0	0	0	100.0 State	0.03208 wide Crash Ra	10.39 te: 63.57	124.58	0.00
Study Ye	ears: 3	.00		Section Totals:	2.58		47	0	1	6	4	36	23.4	0.22383	69.99	87.89	0.80
				Grand Totals:	2.58		49	0	1	6	4	38	22.4	0.22383	72.97	92.75	0.79

# Crash Summary

rart End Element Offset Route - MP Total Injury Crashes Crash Report Crash Date Crash Injury ode Node Begin - End Crashes K A B C PD Mile Point Degree	Section Details														
Node Begin - End Crashes K A B C PD Mile Point Degree 7693 58357 239222 0 - 1.51 0095S - 293.72 27 0 0 0 4 2 2 21 2004-24842 09/06/2004 293.93 B 2005-20265 07/20/2005 294.23 B 2005-20265 07/20/2005 294.23 PD 2005-24540 08/28/2005 294.53 PD 2005-24540 08/28/2005 294.53 PD 2005-24540 08/28/2005 294.53 PD 2005-24540 08/28/2005 294.73 B 2005-24044 08/28/2005 294.73 B 2005-24044 08/28/2005 294.73 PD 2005-24041 02/10/2005 294.93 PD 2005-24041 02/10/2005 295.03 PD 2005-24041 02/10/2005 295.03 PD 2005-24041 02/10/2005 295.13 PD 2005-24041 02/10/2005 295.13 PD 2005-24041 02/10/2005 295.13 PD 2005-24041 02/10/2004 295.13 PD 2005-24041 02/10/2004 295.13 PD 2006-100 01/10/10/2006 295.13 PD 2006-100 01/10/10/2006 295.13 PD 2006-100 01/10/10/2006 295.13 PD 2006-240692 12/10/2005 295.13 PD 2006-240692 12/10/2005 295.13 PD 2006-240692 12/10/2005 295.13 PD 2005-240692 12/10/2005 295.23 PD 2005-240692 12/10/2005 295.23 PD 2006-240692 12/10/2006 295.23 PD 2006-2667 02/10/2006 295.23 PD	Start								Crash Data	Crach	lniury				
2005-20265   07/20/2005   294.23   B   2004-21958   08/12/2004   294.23   PD   2005-24540   08/28/2005   294.53   PD   2004-23420   08/28/2004   294.53   PD   2005-16743   06/11/2005   294.73   B   2005-24044   08/28/2005   294.73   PD   2005-4101   02/10/2005   294.73   PD   2004-8351   02/06/2004   294.98   C   2004-8351   02/06/2004   294.98   C   2004-8351   02/06/2004   294.98   C   2004-83734   08/30/2004   295.03   PD   2004-6629   02/20/2004   295.03   PD   2004-6629   02/20/2004   295.13   PD   2004-37138   11/01/2004   295.13   PD   2004-37148   12/26/2004   295.13   PD   2004-37141   12/26/2004   295.13   PD   2005-26918   09/23/2005   295.13   PD   2006-22362   09/13/2006   295.23   PD   2006-6139   03/07/2006   295.23   PD   2006-6139   03/07/2006   295.23   PD   2006-61367   03/07/2006   295.23   PD   2006-61367   03/07/2006   295.23   PD   2006-2867   03/07/2006   295.23   PD	Node		Liement		Noute - MF		K	-	-			Crasii Kepuit	Crasii Dale		
2004-21958   08/12/2004   294.23   PD	57693	58357	239222	0 - 1.51	0095S - 293.72	27	0	0	4	2	21	2004-24842	09/06/2004	293.93	В
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2006-16565 07/08/2006 295.23 PD 2006-6139 03/07/2006 295.23 PD 2006-2867 02/02/2006 295.23 PD												2005-40692	12/09/2005	295.13	PD
2006-6139 03/07/2006 295.23 PD 2006-2867 02/02/2006 295.23 PD												2006-22352	09/13/2006	295.23	PD
2006-2867 02/02/2006 295.23 PD												2006-16565	07/08/2006	295.23	PD
												2006-6139	03/07/2006	295.23	PD
2006-1590 01/23/2006 295.23 PD												2006-2867	02/02/2006	295.23	PD
												2006-1590	01/23/2006	295.23	PD

# Crash Summary

Start	End	Element	Offset	Route - MP	Total		ion De Iniu	ry Cra	shes		Crash Report	Crash Date	Crash	Injury
Node	Node	Licinom	Begin - End	rtoute wii	Crashes	K	A	В	С	PD	Ordon Nopon	Oragin Bate	Mile Point	Degree
57693	58871	239223	0 - 0.25	0095S - 295.23	11	0	0	0	1	10	2005-34027	12/04/2005	295.33	С
											2005-868	01/07/2005	295.33	PD
											2005-19028	06/28/2005	295.33	PD
											2005-26383	09/17/2005	295.33	PD
											2006-354	01/11/2006	295.33	PD
											2005-1859	01/23/2005	295.33	PD
											2004-30304	10/25/2004	295.33	PD
											2006-14254	06/15/2006	295.33	PD
											2004-36553	12/20/2004	295.33	PD
											2006-18246	07/26/2006	295.33	PD
											2006-19418	08/04/2006	295.33	PD
8869	58871	240305	0 - 0.41	0095S - 295.48	1	0	1	0	0	0	2006-32072	12/15/2006	295.59	Α
8356	58869	239734	0 - 0.41	0095S - 295.89	8	0	0	2	1	5	2005-26200	09/18/2005	296	В
											2004-31589	11/13/2004	296	PD
											2004-26504	09/22/2004	296.20	В
											2004-34743	12/07/2004	296.20	С
											2004-18523	07/07/2004	296.20	PD
											2004-18470	07/07/2004	296.20	PD
											2004-15669	05/30/2004	296.20	PD
											2004-37770	12/26/2004	296.20	PD

Totals:

# Maine Department Of Transportation - Traffic Engineering, Crash Records Section Crash Summary II - Characteristics

Crashes by Day and Hour																										
						AM					ŀ	Hour o	of Day						PM							
Day Of Week	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	Un	Tot
SUNDAY	0	0	0	0	2	0	1	1	0	1	0	0	2	2	1	1	0	0	0	1	0	0	0	0	0	12
MONDAY	0	0	0	0	0	0	0	1	0	0	0	1	1	0	0	2	2	0	0	0	0	0	0	0	0	7
TUESDAY	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	2	2	0	0	0	0	0	0	0	0	5
WEDNESDAY	0	0	0	0	0	0	0	0	1	0	1	1	0	0	1	1	2	0	0	0	0	0	0	0	0	7
THURSDAY	0	0	0	0	0	0	1	1	1	0	0	0	0	1	0	1	1	0	0	0	0	0	0	1	0	7
FRIDAY	0	0	0	0	0	1	0	0	0	1	0	0	0	2	1	1	0	0	0	0	1	0	0	0	0	7
SATURDAY	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	1	0	0	1	0	0	0	0	0	0	4
Totals	0	0	0	0	2	1	3	3	2	2	1	3	3	5	4	9	7	0	1	1	1	0	0	1	0	49

Cı	rashes	by Ye	ar and Month		V	'ehicle Co	unts by Type	
Month	2004	2005	2006	Total	Unit Type	Total	Unit Type	Total
JANUARY	0	2	3	5	1-2 Door	9	32-3 Axle Tractor with Tandem Axle Semi	9
JANUART	U	2	3	3	2-4 Door	33	33-3 Axle Tractor with Tridem Axle Semi	2
FEBRUARY	2	1	1	4	3-Convertible	0	35-3 Axle Tractor with Single Axle Semi & 2	0
MARCH	0	0	1	1	4-Station Wagon	3	Axle Trailer	•
A DDII	0	4	0	1	5-Van	8	36-3 Axle Tractor with Tandem Axle Semi & 2 Axle Trailer	0
APRIL	0	1	U	1	6-Pickup Truck	6	37-5 Axle Semi; Split Trailer Tandem	0
MAY	1	0	1	2	7-SUV	13	•	0
JUNE	0	2	2	4	10-Truck Tractor Only (Bobtail)	0	38-6 Axle Semi; Split Trailer Tandem with Center Axle	U
	-	_			12-School Bus	0	39-6 Axle: Standard Trailer Tandem with Center	. 0
JULY	4	1	2	7	13-Motor Home	0	Axle	Ū
AUGUST	4	2	1	7	14-Motorcycle	0	40-4 Axle Single Unit	0
SEPTEMBER	2	3	1	6	15-Moped	0	42-4 Axle Tractor with Tandem Axle Semi	0
	_				16-Motor Bike	0	50-Any Other Axle Configuration	0
OCTOBER	1	0	0	1	17-Bicycle	0	60-Other Unit	0
NOVEMBER	2	0	0	2	18-Snowmobile	0	70-ATV	0
DECEMBER	6	2	1	9	20-2 Axle Single Unit with Dual Tires	1	81-2 Axle Bus	0
			<u> </u>	<del></del>	21-2 Axle Tractor with Single Axle Semi	0	82-3 Axle Bus	0
Total	22	14	13	49	22-2 Axle Tractor with Tandem Axle Semi	0	98-Farm Vehicles / Tractors	0
					25-2 Axle Tractor with Single Axle Semi & 2 Axle Trailer	0	99-Unknown	0
					30-3 Axle Single Unit	1	Total	85
					31-3 Axle Tractor with Single Axle Semi	0		

Crashes by Apparen	t Cont	ributin	ng Fac	tor An	d Driv	er	
Apparent Contributing Factor	Dr 1	Dr 2	Dr 3	Dr 4	Dr 5	Other	Tota
No Improper Action	14	16	3	1	0	0	34
Failure to Yield Right of Way	5	2	0	0	0	0	7
Illegal Unsafe Speed	14	3	0	0	0	0	17
Following Too Close	1	3	0	0	0	0	4
Disregard Traffic Control Device	0	0	0	0	0	0	0
Driving Left of Center Not Passing	0	0	0	0	0	0	0
Improper Passing, Overtaking	1	1	0	0	0	0	2
Improper Unsafe Lane Change	4	1	0	0	0	0	5
Improper Parking Start, Stop	0	0	0	0	0	0	0
Improper Turn	0	0	0	0	0	0	0
Unsafe Backing	0	0	0	0	0	0	0
No Signal or Improper Signal	0	0	0	0	0	0	0
Impeding Traffic	0	0	0	0	0	0	0
Driver Inattention, Distraction	8	6	0	0	0	0	14
Driver Inexperience	0	0	0	0	0	0	0
Pedestrian Violation Error	0	0	0	0	0	0	0
Physical Impairment	0	0	0	0	0	0	0
Vision Obscured, Windshield Glass	0	0	0	0	0	0	0
Vision Obscured, Sun, Headlights	0	0	0	0	0	0	0
Other Vision Obscurement	0	0	0	0	0	0	0
Other Human Violation Factor	0	0	0	0	0	0	0
Hit and Run	0	0	0	0	0	0	0
Defective Brakes	0	0	0	0	0	0	0
Defective Tire, Tire Failure	0	0	0	0	0	0	0
Defective Lights	0	0	0	0	0	0	0
Defective Suspension	0	0	0	0	0	0	0
Defective Steering	0	0	0	0	0	0	0
Other Vehicle Defect or Factor	2	0	0	0	0	0	2
Unknown	0	0	0	0	0	0	0
Total	49	32	3	1	0	0	85

Crashes by Ap	paren	t Phys	sical C	onditi	on An	d Drive	er
Apparent Physical Condition	Dr 1	Dr 2	Dr 3	Dr 4	Dr 5	Other	Total
Normal	47	31	3	1	0	0	82
Under the Influence	0	1	0	0	0	0	1
Had Been Drinking	1	0	0	0	0	0	1
Had Been Using Drugs	0	0	0	0	0	0	0
Asleep	1	0	0	0	0	0	1
Fatigued	0	0	0	0	0	0	0
ill	0	0	0	0	0	0	0
Handicapped	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0
Total	49	32	3	1	0	0	85

Driver Age by Unit Type												
Age	Driver	Bicycle	SnowMobile	Pedestrian	ATV	Total						
09-Under	0	0	0	0	0	0						
10-14	0	0	0	0	0	0						
15-19	4	0	0	0	0	4						
20-24	11	0	0	0	0	11						
25-29	12	0	0	0	0	12						
30-39	20	0	0	0	0	20						
40-49	15	0	0	0	0	15						
50-59	17	0	0	0	0	17						
60-69	3	0	0	0	0	3						
70-79	1	0	0	0	0	1						
80-Over	2	0	0	0	0	2						
Unknown	0	0	0	0	0	0						
Total	85	0	0	0	0	85						

Fixed Object Struck	
Fixed Object Struck	Total
1-Construction, Barricades Equipment, etc.	0
2-Traffic Signal	0
3-R.R. Crossing Device	0
4-Light Pole	0
5-Utility Pole (Tel. Electrical)	0
6-Sign Structure Post	0
7-Mail Boxes or Posts	0
8-Other Poles, posts or supports	1
9-Fire Hydrant/Parking Meter	0
10-Tree or Shrubbery	0
11-Crash Cushion	2
12-Median Safety Barrier	6
13-Bridge Piers (including protective guard rails)	1
14-Other Guardrails	3
15-Fencing (not median barrier)	0
16-Culvert Headwall	0
17-Embankment, Ditch, Curb	3
18-Building, Wall	1
19-Rock Outcrops or Ledge	0
20-Other	5
Total	22

Traffic Control Devices	
Traffic Control Device	Total
1-Traffic Signals (Stop & Go)	0
2-Traffic Flashing	0
3-Overhead Flashers	4
4-Stop Signs - All Approaches	0
5-Stop Signs - Other	0
6-Yield Sign	1
7-Curve Warning Sign	0
8-Officer, Flagman, School Patrol	0
9-School Bus Stop Arm	0
10-School Zone Sign	0
11-R.R. Crossing Device	0
12-No Passing Zone	0
13-None	34
14-Other	10
Total	49

Road Character	
Road Character	Total
1-Level Straight	26
2-Level Curved	0
3-On Grade Straight	18
4-On Grade Curved	4
5-Top of Hill Straight	1
6-Top of Hill Curved	0
7-Bottom of Hill Straight	0
8-Bottom of Hill Curved	0
9-Other	0
Total	49

Injury Data								
Severity Code	Injury Crashes	Number Of Injuries						
K	0	0						
Α	1	1						
В	6	6						
С	4	7						
PD	38	0						
Total	49	14						

Light	
Light	Total
1-Dawn (Morning)	3
2-Daylight	38
3-Dusk (Evening)	3
4-Dark (Street Lights On)	2
5-Dark (No Street Lights)	3
6-Dark (Street Lights Off)	0
7-Other	0
Total	49

## Crash Summary II - Characteristics

Crashes by Crash Type and Type of Location										
Crash Type	Straight Road	Curved Road	Three Leg Intersection	Four Leg Intersection	Five Leg Intersection	Driveways	Bridges	Interchanges	Other	Total
Object in Road	4	0	0	0	0	0	0	1	0	5
Rear End / Sideswipe	20	4	0	0	0	0	0	2	3	29
Head-on / Sideswipe	0	0	0	0	0	0	0	0	0	0
Intersection Movement	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	0	0	0
Train	0	0	0	0	0	0	0	0	0	0
Ran Off Road	8	0	0	0	0	0	0	0	0	8
All Other Animal	0	0	0	0	0	0	0	0	0	0
Bike	0	0	0	0	0	0	0	0	0	0
Other	5	0	0	0	0	0	0	0	0	5
Jackknife	0	0	0	0	0	0	0	0	0	0
Rollover	0	0	0	0	0	0	0	0	0	0
Fire	0	0	0	0	0	0	0	0	0	0
Submersion	0	0	0	0	0	0	0	0	0	0
Rock Thrown	0	0	0	0	0	0	0	0	0	0
Bear	0	0	0	0	0	0	0	0	0	0
Deer	0	0	0	0	0	0	0	0	0	0
Moose	2	0	0	0	0	0	0	0	0	2
Total	39	4	0	0	0	0	0	3	3	49

# Maine Department Of Transportation - Traffic Engineering, Crash Records Section Crash Summary II - Characteristics

			Crachae	w Woather I	ight Condit	ion and Pa	ad Surface				
			Crasnes	y Weather, L	ignt Condit	ion and Ro	ad Surrace				
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	lce, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Blowing Sand or Dust											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Clear											
Dark (No Street Lights)	0	1	0	0	0	0	0	0	0	0	1
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	24	0	0	0	0	0	0	0	0	24
Dusk (Evening)	0	2	0	0	0	0	0	0	0	0	2
Other	0	0	0	0	0	0	0	0	0	0	0
Cloudy											
Dark (No Street Lights)	0	2	0	0	0	0	0	0	0	0	2
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	1	0	0	0	0	0	0	0	0	1
Daylight	0	3	0	0	0	0	0	0	0	0	3
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Fog, Smog, Smoke											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0

# Maine Department Of Transportation - Traffic Engineering, Crash Records Section Crash Summary II - Characteristics

				10/			10 1				
			Crashes b	y Weather, L	ight Condit	ion and Ro	ad Surface				
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	Ice, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Other											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Rain											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	2	2
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Severe Cross Winds											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Sleet, Hail, Freezing Rain											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	1	0	1
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0

			Crashes b	y Weather, L	ight Condit	ion and Ro	ad Surface				
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	Ice, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Snow											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	1	1	0	2
Dawn (Morning)	0	0	0	0	0	0	0	0	2	0	2
Daylight	0	0	1	1	0	0	0	4	2	0	8
Dusk (Evening)	0	0	0	0	0	0	0	0	1	0	1
Other	0	0	0	0	0	0	0	0	0	0	0
TOTAL	0	33	1	1	0	0	0	5	7	2	49

### **Crash Summary Report**

#### Report Selections and Input Parameters

#### REPORT SELECTIONS

✓ Crash Summary I ✓ Section Detail ✓ Crash Summary II

#### **REPORT PARAMETERS**

Study Period: Year 2004, Start Month 1 through Year 2006 End Month: 12

Input Data: Route 0095X First Node: 58311 Last Node: 58312

Exclude First Node: No; Exclude Last Node: No

Start Offset: 0; End Offset: 0

#### **REPORT DESCRIPTION**

I-95 NB

# 

			No	odes										
Node	Route - MP	Node Description	U/R	Total		Injur	y Cra	shes		Percent	Annual M C	rash	Critical	CRF
				Crashes	K	Α	В	С	PD	Injury	Ent-Veh F	Rate	Rate	
58311	0095X - 6.18	BRG 6228, I 95 NB under BERWICK RD	2	0	0	0	0	0	0	0.0 Statev	10.337 wide Crash Rate:	0.00 0.12	0.26	0.00
58866	0095X - 6.44	Int of I 95 NB, RAMP OFF TO YORK CONNECTOR	1	2	0	0	0	1	1	50.0 Statev	10.337 wide Crash Rate:	0.06	0.09	0.00
58868	0095X - 7.10	Int of I 95 NB, RAMP ON FROM YORK CONNECTOR	1	0	0	0	0	0	0	0.0 Statev	8.315 wide Crash Rate:	0.00	0.10	0.00
57692	0095X - 7.19	Non-Int I 95 NB	1	2	0	0	0	0	2		8.315 wide Crash Rate:	0.08	0.10	0.00
58312	0095X - 9.43	BRG 1311, I 95 NB over CAPE NEDDICK RIVER	1	0	0	0	0	0	0	0.0 Statev	0.000 wide Crash Rate:	0.00	0.00	0.00
Study \	ears: 3.00	NODE TOTA	LS:	4	0	0	0	1	3	25.0	37.304	0.04	0.11	0.33

# Crash Summary I

							Section	ns									
Start	End	Element	Offset	Route - MP	Section	U/R	Total		Inju	ry Cr	ashes		Percent	Annual	Crash	Critical	CRF
Node	Node		Begin - End		Length		Crashes	K	Α	В	С	PD	Injury	HMVM	Rate	Rate	
57692 Non-Int I 95	58312 5 NB	239220	0 - 2.24	0095X - 4.95 INT 95 NB	2.24	1	17	1	1	0	1	14	17.6 State	0.18625 wide Crash Ra	30.43 tte: 63.57	90.15	0.00
58311 BRG 6228,	58866 I 95 NB ur	239686 nder BERWICH	0 - 0.26 KRD	0095X - 6.18 INT 95 NB	0.26	1	13	0	0	1	2	10	23.1 State	0.02688 wide Crash Ra	161.24 ite: 63.57	129.70	1.24
58866 Int of I 95 N CONNECT		240301 OFF TO YORK	0 - 0.66	0095X - 6.44 INT 95 NB	0.66	1	13	0	0	1	0	12	7.7 State	0.05083 wide Crash Ra	85.25 ite: 63.57	112.89	0.00
57692 Non-Int I 95	58868 5 NB	239221	0 - 0.09	0095X - 7.10 INT 95 NB	0.09	1	14	0	0	1	3	10	28.6 State	0.00748 wide Crash Ra	623.62 ite: 63.57	178.38	3.50
Study Ye	ears: 3	.00		Section Totals:	3.25		57	1	1	3	6	46	19.3	0.27144	70.00	85.72	0.82
				Grand Totals:	3.25		61	1	1	3	7	49	19.7	0.27144	74.91	90.50	0.83

## Crash Summary

						Sect	ion De	etails						
Start	End	Element	Offset	Route - MP	Total		Inju	iry Cra	ashes		Crash Report	Crash Date	Crash	Injury
Node	Node		Begin - End		Crashes	K	Α	В	С	PD			Mile Point	Degree
58311	58866	239686	0 - 0.26	0095X - 6.18	13	0	0	1	2	10	2004-11873	04/25/2004	6.28	В
											2004-35997	12/15/2004	6.28	С
											2004-13592	05/16/2004	6.28	PD
											2006-7531	03/10/2006	6.28	PD
											2005-32949	11/19/2005	6.28	PD
											2005-32193	11/21/2005	6.28	PD
											2004-7641	02/06/2004	6.28	PD
											2006-22933	09/20/2006	6.28	PD
											2005-7127	03/04/2005	6.28	PD
											2004-21899	07/02/2004	6.28	PD
											2006-11814	05/20/2006	6.28	PD
											2006-22931	09/19/2006	6.38	С
											2004-12449	05/03/2004	6.38	PD
58866	58868	240301	0 - 0.66	0095X - 6.44	13	0	0	1	0	12	2006-21747	09/04/2006	6.44	PD
											2006-21169	09/02/2006	6.44	PD
											2004-26928	06/23/2004	6.54	В
											2004-15701	05/29/2004	6.54	PD
											2006-6133	03/03/2006	6.54	PD
											2006-10538	04/24/2006	6.54	PD
											2006-28986	11/12/2006	6.54	PD
											2006-32074	12/20/2006	6.54	PD
											2004-21431	08/01/2004	6.54	PD
											2006-11168	05/15/2006	6.64	PD
											2006-12582	05/28/2006	6.74	PD
											2005-22723	08/13/2005	6.74	PD
											2004-24901	08/07/2004	6.94	PD

# Crash Summary

						Sect	ion De	etails						
Start Node	End Node	Element	Offset	Route - MP	Total Crashes	K	-	ry Cra		DD	Crash Report	Crash Date	Crash Mile Point	Injury
NOUE -	Noue		Begin - End		Crasiles	n.	Α	В	С	PD			Wille I Ollit	Degree
7692	58868	239221	0 - 0.09	0095X - 7.10	14	0	0	1	3	10	2004-35994	12/07/2004	7.10	В
											2004-37768	12/30/2004	7.10	С
											2004-24110	08/27/2004	7.10	С
											2005-12590	04/25/2005	7.10	С
											2006-22932	09/19/2006	7.10	PD
											2006-25856	10/20/2006	7.10	PD
											2005-10262	03/31/2005	7.10	PD
											2005-14065	04/15/2005	7.10	PD
											2004-35115	12/11/2004	7.10	PD
											2004-13828	05/21/2004	7.10	PD
											2004-22472	08/17/2004	7.10	PD
											2004-24986	08/21/2004	7.10	PD
											2004-24902	08/06/2004	7.10	PD
											2006-28174	11/11/2006	7.19	PD
692	58312	239220	0 - 2.24	0095X - 7.19	17	1	1	0	1	14	2006-24012	10/01/2006	7.29	С
											2004-18319	06/24/2004	7.29	PD
											2006-21601	08/27/2006	7.39	PD
											2006-8591	04/06/2006	7.39	PD
											2006-20626	08/27/2006	7.49	PD
											2005-9502	03/17/2005	7.59	PD
											2006-170	01/05/2006	7.69	PD
											2006-32903	12/30/2006	8.19	PD
											2006-12960	06/03/2006	8.19	PD
											2006-15195	06/25/2006	8.19	PD
											2004-17015	06/16/2004	8.29	Α
											2005-1123	01/08/2005	8.29	PD
											2006-2651	01/30/2006	8.39	PD
											2005-1892	01/24/2005	8.69	PD
											2004-828	01/07/2004	8.89	PD
											2005-27852	10/09/2005	9.19	K
											2004-15702	06/10/2004	9.29	PD

Totals: 57 1 1 3 6 46

# Maine Department Of Transportation - Traffic Engineering, Crash Records Section Crash Summary II - Characteristics

										Cr	ashes	s by E	ay an	d Ho	ur											
						AM					I	Hour	of Day						РМ							
Day Of Week	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	Un	Tot
SUNDAY	1	0	2	1	0	0	0	0	0	0	0	0	1	1	1	0	0	0	2	1	0	0	0	0	0	10
MONDAY	0	1	0	0	0	1	1	0	0	0	0	0	2	1	0	1	0	0	0	0	1	0	0	0	0	8
TUESDAY	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	2	0	0	0	0	0	0	0	0	0	4
WEDNESDAY	0	0	0	0	0	0	2	0	1	1	0	0	0	0	0	0	2	0	1	0	0	0	0	0	0	7
THURSDAY	0	0	1	0	0	2	0	0	0	0	0	0	1	1	0	0	1	0	1	1	0	0	0	0	0	8
FRIDAY	0	0	0	0	0	0	0	1	0	0	0	1	0	1	0	5	2	1	0	0	0	0	0	1	0	12
SATURDAY	0	0	1	1	0	0	0	0	0	0	0	1	2	0	3	0	0	2	0	1	0	0	0	1	0	12
Totals	1	1	4	2	0	4	3	1	1	1	1	2	6	4	4	8	5	3	4	3	1	0	0	2	0	61

Cı	rashes	by Ye	ar and Month		Ve	ehicle Co	unts by Type	
Month	2004	2005	2006	Total	Unit Type	Total	Unit Type	Total
JANUARY	1	2	2	5	1-2 Door	10	32-3 Axle Tractor with Tandem Axle Semi	9
	•			3	2-4 Door	31	33-3 Axle Tractor with Tridem Axle Semi	1
FEBRUARY	1	0	0	1	3-Convertible	0	35-3 Axle Tractor with Single Axle Semi & 2	0
MARCH	0	3	2	5	4-Station Wagon	5	Axle Trailer	0
APRIL	1	2	2	5	5-Van	12	36-3 Axle Tractor with Tandem Axle Semi & 2 Axle Trailer	0
	•				6-Pickup Truck	12	37-5 Axle Semi; Split Trailer Tandem	0
MAY	5	0	3	8	7-SUV	12	38-6 Axle Semi; Split Trailer Tandem with	0
JUNE	4	0	3	7	10-Truck Tractor Only (Bobtail)	0	Center Axle	Ü
JULY	1	0	0	1	12-School Bus	0	39-6 Axle; Standard Trailer Tandem with Center	0
	•			1	13-Motor Home	0	Axle	
AUGUST	6	2	2	10	14-Motorcycle	0	40-4 Axle Single Unit	0
SEPTEMBER	0	1	5	6	15-Moped	0	42-4 Axle Tractor with Tandem Axle Semi	0
OCTOBER	0	4	2	2	16-Motor Bike	0	50-Any Other Axle Configuration	0
OCTOBER	U	ı	2	3	17-Bicycle	0	60-Other Unit	1
NOVEMBER	0	2	2	4	18-Snowmobile	0	70-ATV	0
DECEMBER	4	0	2	6	20-2 Axle Single Unit with Dual Tires	7	81-2 Axle Bus	0
					21-2 Axle Tractor with Single Axle Semi	0	82-3 Axle Bus	0
Total	23	13	25	61	22-2 Axle Tractor with Tandem Axle Semi	1	98-Farm Vehicles / Tractors	0
					25-2 Axle Tractor with Single Axle Semi & 2 Axle Trailer	1	99-Unknown	0
					30-3 Axle Single Unit	1	Total	103
					31-3 Axle Tractor with Single Axle Semi	0		

No Improper Action	No Improper Action
No Improper Action	No Improper Action
No Improper Action 31 18 1 0 0 0 50 Failure to Yield Right of Way 3 5 0 0 0 0 0 8 Illegal Unsafe Speed 12 1 0 0 0 0 13 Following Too Close 0 5 0 0 0 0 0 5 Disregard Traffic Control Device 0 0 0 0 0 0 0 0 0 Driving Left of Center Not Passing 0 0 0 0 0 0 0 0 0 Improper Passing, Overtaking 0 1 0 0 0 0 0 0 1 Improper Unsafe Lane Change 5 1 0 0 0 0 0 0 1 Improper Parking Start, Stop 0 0 0 0 0 0 0 0 0 0 Improper Turn 0 0 0 0 0 0 0 0 0 0 0 Impading Traffic 0 0 0 0 0 0 0 0 0 0 0 Impeding Traffic 0 0 0 0 0 0 0 0 0 0 0 0 0 Impeding Traffic 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Impeding Traffic 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	No Improper Action
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No Improper Action 31 18 1 0 0 50	No Improper Action 31 18 1 0 0 0
Apparent Contributing Factor Dr 1 Dr 2 Dr 3 Dr 4 Dr 5 Other Tota	Apparent Contributing Factor Dr 1 Dr 2 Dr 3 Dr 4 Dr 5 Other T

Crashes by Ap	paren	t Phys	sical C	onditi	on An	d Drive	er
Apparent Physical Condition	Dr 1	Dr 2	Dr 3	Dr 4	Dr 5	Other	Total
Normal	58	40	1	0	0	0	99
Under the Influence	1	1	0	0	0	0	2
Had Been Drinking	0	0	0	0	0	0	0
Had Been Using Drugs	0	0	0	0	0	0	0
Asleep	1	0	0	0	0	0	1
Fatigued	0	0	0	0	0	0	0
ill	0	0	0	0	0	0	0
Handicapped	0	0	0	0	0	0	0
Other	1	0	0	0	0	0	1
Total	61	41	1	0	0	0	103

		Drive	er Age by Ur	nit Type		
Age	Driver	Bicycle	SnowMobile	Pedestrian	ATV	Total
09-Under	0	0	0	0	0	0
10-14	0	0	0	0	0	0
15-19	8	0	0	0	0	8
20-24	13	0	0	0	0	13
25-29	8	0	0	0	0	8
30-39	22	0	0	0	0	22
40-49	24	0	0	0	0	24
50-59	16	0	0	0	0	16
60-69	8	0	0	0	0	8
70-79	3	0	0	0	0	3
80-Over	0	0	0	0	0	0
Unknown	1	0	0	0	0	1
Total	103	0	0	0	0	103

Fixed Object Struck	
Fixed Object Struck	Total
1-Construction, Barricades Equipment, etc.	0
2-Traffic Signal	0
3-R.R. Crossing Device	0
4-Light Pole	0
5-Utility Pole (Tel. Electrical)	0
6-Sign Structure Post	0
7-Mail Boxes or Posts	0
8-Other Poles, posts or supports	1
9-Fire Hydrant/Parking Meter	0
10-Tree or Shrubbery	3
11-Crash Cushion	0
12-Median Safety Barrier	8
13-Bridge Piers (including protective guard rails)	0
14-Other Guardrails	0
15-Fencing (not median barrier)	0
16-Culvert Headwall	0
17-Embankment, Ditch, Curb	0
18-Building, Wall	0
19-Rock Outcrops or Ledge	0
20-Other	2
Total	14

Traffic Control Devices	
Traffic Control Device	Total
1-Traffic Signals (Stop & Go)	2
2-Traffic Flashing	0
3-Overhead Flashers	3
4-Stop Signs - All Approaches	0
5-Stop Signs - Other	2
6-Yield Sign	3
7-Curve Warning Sign	0
8-Officer, Flagman, School Patrol	0
9-School Bus Stop Arm	0
10-School Zone Sign	0
11-R.R. Crossing Device	0
12-No Passing Zone	0
13-None	38
14-Other	13
Total	61

Road Character	
Road Character	Total
1-Level Straight	41
2-Level Curved	1
3-On Grade Straight	14
4-On Grade Curved	5
5-Top of Hill Straight	0
6-Top of Hill Curved	0
7-Bottom of Hill Straight	0
8-Bottom of Hill Curved	0
9-Other	0
Total	61

Inj	ury Data	
Severity Code	Injury Crashes	Number Of Injuries
K	1	1
Α	1	1
В	3	11
С	7	7
PD	49	0
Total	61	20

Light	
Light	Total
1-Dawn (Morning)	4
2-Daylight	36
3-Dusk (Evening)	2
4-Dark (Street Lights On)	9
5-Dark (No Street Lights)	10
6-Dark (Street Lights Off)	0
7-Other	0
Total	61

			Crashes by	y Crash Ty <sub>l</sub>	pe and Type	of Locatio	n			
Crash Type	Straight Road	Curved Road	Three Leg Intersection	Four Leg Intersection	Five Leg Intersection	Driveways	Bridges	Interchanges	Other	Total
Object in Road	8	0	0	0	0	0	0	0	0	8
Rear End / Sideswipe	30	3	2	0	0	0	0	1	1	37
Head-on / Sideswipe	0	0	0	0	0	0	0	0	0	0
Intersection Movement	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	0	0	0
Train	0	0	0	0	0	0	0	0	0	0
Ran Off Road	4	2	0	0	0	0	0	0	0	6
All Other Animal	0	0	0	0	0	0	0	0	0	0
Bike	0	0	0	0	0	0	0	0	0	0
Other	3	0	0	0	0	0	0	0	0	3
Jackknife	0	0	0	0	0	0	0	0	0	0
Rollover	0	0	0	0	0	0	0	0	0	0
Fire	0	0	0	0	0	0	0	0	0	0
Submersion	0	0	0	0	0	0	0	0	0	0
Rock Thrown	0	0	0	0	0	0	0	0	0	0
Bear	0	0	0	0	0	0	0	0	0	0
Deer	5	0	0	0	0	0	0	0	0	5
Moose	2	0	0	0	0	0	0	0	0	2
Total	52	5	2	0	0	0	0	1	1	61

Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	lce, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Blowing Sand or Dust											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Clear											
Dark (No Street Lights)	0	2	0	0	0	0	0	0	0	0	2
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	6	0	0	0	0	0	0	0	0	6
Dawn (Morning)	0	2	0	0	0	0	0	0	0	1	3
Daylight	0	21	0	0	0	0	0	0	0	0	21
Dusk (Evening)	0	2	0	0	0	0	0	0	0	0	2
Other	0	0	0	0	0	0	0	0	0	0	0
Cloudy											
Dark (No Street Lights)	0	2	0	0	0	0	0	0	0	0	2
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	1	0	0	0	0	0	0	0	0	1
Daylight	0	6	0	0	0	0	0	0	0	1	7
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Fog, Smog, Smoke											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0

Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	lce, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Other											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Rain											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	4	4
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	3	3
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	4	4
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Severe Cross Winds											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Sleet, Hail, Freezing Rain											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	1	0	0	0	0	0	0	0	1
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0

			Crashes b	y Weather, L	_ight Condit	ion and Ro	ad Surface				
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	Ice, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Snow											
Dark (No Street Lights)	0	0	0	2	0	0	0	0	0	0	2
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	1	0	0	0	1	0	1	3
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
TOTAL	0	42	1	3	0	0	0	1	0	14	61

## **Crash Summary Report**

			Re	eport Selections and In	put Para	meters			
REPORT	SELECTIONS								
✓ Crash	Summary I	✓ Section De	etail	✓ Crash Summary II		1320 Included	t	1320 & Driver Report Include	d
	DESCRIPTION Ork Toll area								
	PARAMETERS  5, Start Month 1 throu	gh Year 2007	End Month: 12						
Route:	0095S	Start Node: End Node:		Start Offset: End Offset:	•	_	_	e First Node e Last Node	

# Crash Summary I

			No	odes										
Node	Route - MP	Node Description	U/R	Total		Injur	y Cra	shes		Percent	Annual M C	Crash	Critical	CRF
				Crashes	K	Α	В	С	PD	Injury	Ent-Veh	Rate	Rate	
58357	0095S - 294.52	Non-Int I 95 SB	1	0	0	0	0	0	0	0.0 State	7.760 wide Crash Rate	0.00 e: 0.03	0.10	0.00
57693	0095S - 296.03	Non-Int I 95 SB	1	6	0	0	0	2	4	33.3 State	7.760 wide Crash Rate	0.26 0.03	0.10	2.55
58871	0095S - 296.14	Int of I 95 SB, RAMP B OFF TO YORK CONNECTOR	1	0	0	0	0	0	0	0.0 State	7.760 wide Crash Rate	0.00 e: 0.03	0.10	0.00
58869	0095S - 296.78	Int of I 95 SB, RAMP A FROM YORK CONNECTOR	1	0	0	0	0	0	0	0.0 State	9.600 wide Crash Rate	0.00 e: 0.03	0.10	0.00
58356	0095S - 297.02	BRG 6228, I 95 SB under ST RTE 91	2	0	0	0	0	0	0	0.0 State	9.603 wide Crash Rate	0.00 e: 0.13	0.29	0.00
Study \	Years: 3.00	NODE TOTA	ALS:	6	0	0	0	2	4	33.3	42.483	0.05	0.10	0.47

# Crash Summary I

							Section	ns									
Start	End	Element	Offset	Route - MP	Section	U/R	Total		Inju	ry Cr	ashes		Percent	Annual	Crash	Critical	CRF
Node	Node		Begin - End		Length		Crashes	K	Α	В	С	PD	Injury	HMVM	Rate	Rate	
57693 Non-Int I 95	58357 5 SB	239222	0 - 1.51	0095S - 294.52 INT 95 SB	1.51	1	15	0	0	3	0	12	20.0 State	0.11717 wide Crash Ra	42.67 te: 59.02	90.98	0.00
57693 Non-Int I 98	58871 5 SB	2522897	0 - 0.11	0095S - 296.03 INT 95 SB	0.11	1	10	0	0	0	2	8	20.0 State	0.00854 wide Crash Ra	390.51 te: 59.02	163.16	2.39
58869 Int of I 95 S CONNECT	*	2522901 A FROM YORK	0 - 0.64	0095S - 296.14 INT 95 SB	0.64	1	1	0	1	0	0	0	100.0 State	0.04639 wide Crash Ra	7.18 te: 59.02	108.47	0.00
58356 BRG 6228,	58869 I 95 SB ur	2522903 nder ST RTE 9	0 - 0.24	0095S - 296.54 INT 95 SB	0.24	1	1	0	0	0	0	1	0.0 State	0.02305 wide Crash Ra	14.46 te: 59.02	127.05	0.00
Study Ye	ears: 3	3.00		Section Totals:	2.50		27	0	1	3	2	21	22.2	0.19515	46.12	84.03	0.55
				Grand Totals:	2.50		33	0	1	3	4	25	24.2	0.19515	56.37	89.64	0.63

## **Crash Summary**

						Sect	ion D	etails						
Start	End	Element	Offset	Route - MP	Total		lnjι	iry Cra	ashes		Crash Report	Crash Date	Crash	Injury
Node	Node		Begin - End		Crashes	K	Α	В	С	PD			Mile Point	Degree
57693	58357	239222	0 - 1.51	0095S - 294.52	15	0	0	3	0	12	2005-20265	07/20/2005	295.03	В
											2007-6429	03/16/2007	295.27	PD
											2006-1590	01/23/2006	295.33	PD
											2005-24540	08/28/2005	295.33	PD
											2005-16743	06/11/2005	295.53	В
											2005-24044	08/28/2005	295.53	PD
											2007-7738	04/01/2007	295.63	PD
											2005-4101	02/10/2005	295.73	PD
											2006-12693	05/25/2006	295.83	В
											2007-6418	03/17/2007	295.83	PD
											2006-16830	06/29/2006	295.83	PD
											2006-100	01/01/2006	295.93	PD
											2005-40692	12/09/2005	295.93	PD
											2005-26918	09/23/2005	295.93	PD
											2006-2867	02/02/2006	296.02	PD
57693	58871	2522897	0 - 0.11	0095S - 296.03	10	0	0	0	2	8	2007-13496	06/02/2007	296.04	С
											2005-34027	12/04/2005	296.13	С
											2005-868	01/07/2005	296.13	PD
											2006-354	01/11/2006	296.13	PD
											2006-19418	08/04/2006	296.13	PD
											2006-18246	07/26/2006	296.13	PD
											2005-19028	06/28/2005	296.13	PD
											2005-26383	09/17/2005	296.13	PD
											2006-14254	06/15/2006	296.13	PD
											2005-1859	01/23/2005	296.13	PD
58869	58871	2522901	0 - 0.64	0095S - 296.14	1	0	1	0	0	0	2006-32072	12/15/2006	296.48	Α
58356	58869	2522903	0 - 0.24	0095S - 296.78	1	0	0	0	0	1	2007-20241	07/08/2007	296.90	PD
					27		1	2	2	21				

Totals: 27 0 1 3 2 21

# Maine Department Of Transportation - Traffic Engineering, Crash Records Section Crash Summary II - Characteristics

										Cr	ashes	s by D	ay an	d Ho	ur											
						AM					ŀ	Hour o	of Day						PM							
Day Of Week	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	Un	Tot
SUNDAY	0	0	0	0	1	0	1	0	0	1	0	1	0	1	1	0	0	0	0	0	1	0	0	0	0	7
MONDAY	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	2
TUESDAY	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	0	2
WEDNESDAY	0	0	0	0	0	0	0	0	1	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	3
THURSDAY	0	0	0	0	0	0	0	1	0	0	0	1	0	1	0	1	1	0	0	0	0	0	0	1	0	6
FRIDAY	0	0	0	0	0	1	0	0	0	1	0	0	0	2	1	3	0	0	0	1	0	0	0	0	0	9
SATURDAY	0	0	0	0	0	0	0	1	0	0	0	1	0	0	1	0	0	0	1	0	0	0	0	0	0	4
Totals	0	0	0	0	1	1	1	3	1	2	1	4	0	4	4	4	3	0	1	1	1	0	0	1	0	33

С	rashes	by Ye	ar and Month		Ve	ehicle Co	unts by Type	
Month	2005	2006	2007	Total	Unit Type	Total	Unit Type	Total
JANUARY	2	3	0	5	1-2 Door	7	32-3 Axle Tractor with Tandem Axle Semi	6
	2	3			2-4 Door	23	33-3 Axle Tractor with Tridem Axle Semi	2
FEBRUARY	1	1	0	2	3-Convertible	0	35-3 Axle Tractor with Single Axle Semi & 2	0
MARCH	0	0	3	3	4-Station Wagon	3	Axle Trailer	0
APRIL	1	0	1	2	5-Van	6	36-3 Axle Tractor with Tandem Axle Semi & 2 Axle Trailer	0
	-	U	1	2	6-Pickup Truck	8	37-5 Axle Semi; Split Trailer Tandem	0
MAY	0	1	0	1	7-SUV	5	38-6 Axle Semi; Split Trailer Tandem with	0
JUNE	2	2	1	5	10-Truck Tractor Only (Bobtail)	0	Center Axle	Ü
JULY	1	4	3	E	12-School Bus	0	39-6 Axle; Standard Trailer Tandem with Center	0
	•	1	3	5	13-Motor Home	0	Axle	
AUGUST	2	1	1	4	14-Motorcycle	0	40-4 Axle Single Unit	0
SEPTEMBER	2	0	1	3	15-Moped	0	42-4 Axle Tractor with Tandem Axle Semi	0
OCTOBER	0	0	0	0	16-Motor Bike	0	50-Any Other Axle Configuration	0
				0	17-Bicycle	0	60-Other Unit	0
NOVEMBER	0	0	0	0	18-Snowmobile	0	70-ATV	0
DECEMBER	2	1	0	3	20-2 Axle Single Unit with Dual Tires	1	81-2 Axle Bus	0
Total	40	40			21-2 Axle Tractor with Single Axle Semi	0	82-3 Axle Bus	0
Total	13	10	10	33	22-2 Axle Tractor with Tandem Axle Semi	0	98-Farm Vehicles / Tractors	0
					25-2 Axle Tractor with Single Axle Semi & 2 Axle Trailer	0	99-Unknown	1
					30-3 Axle Single Unit	0	Total	62
					31-3 Axle Tractor with Single Axle Semi	0		

	33	24	4	1	0	0	6:
Jnknown	0	0	0	0	0	0	C
Other Vehicle Defect or Factor	0	0	0	0	0	0	C
Defective Steering	0	0	0	0	0	0	C
Defective Suspension	0	0	0	0	0	0	(
Defective Lights	0	0	0	0	0	0	(
Defective Tire, Tire Failure	0	0	0	0	0	0	(
Defective Brakes	1	0	0	0	0	0	
lit and Run	0	0	0	0	0	0	(
Other Human Violation Factor	0	0	0	0	0	0	(
Other Vision Obscurement	0	0	0	0	0	0	(
/ision Obscured, Sun, Headlights	0	0	0	0	0	0	(
/ision Obscured, Windshield Glass	0	0	0	0	0	0	(
Physical Impairment	0	0	0	0	0	0	(
Pedestrian Violation Error	0	0	0	0	0	0	
Driver Inexperience	1	1	0	0	0	0	
Driver Inattention, Distraction	8	5	0	0	0	0	1
mpeding Traffic	0	0	0	0	0	0	
No Signal or Improper Signal	0	0	0	0	0	0	
Jnsafe Backing	0	0	0	0	0	0	
mproper Turn	0	0	0	0	0	0	
mproper Parking Start, Stop	0	0	0	0	0	0	
mproper Unsafe Lane Change	2	0	0	0	0	0	
mproper Passing, Overtaking	0	1	0	0	0	0	
Driving Left of Center Not Passing	0	0	0	0	0	0	
Disregard Traffic Control Device	0	0	0	0	0	0	
Following Too Close	2	1	0	0	0	0	
llegal Unsafe Speed	7	1	0	0	0	0	
No Improper Action Failure to Yield Right of Way	8 4	13 2	3 1	1 0	0	0 0	2
	0	40	2	4	0	0	
Apparent Contributing Factor	Dr 1	Dr 2	Dr 3	Dr 4	Dr 5	Other	T

Crashes by Ap	paren	t Phys	sical C	onditi	on An	d Drive	er
Apparent Physical Condition	Dr 1	Dr 2	Dr 3	Dr 4	Dr 5	Other	Total
Normal	32	23	3	1	0	0	59
Under the Influence	0	1	0	0	0	0	1
Had Been Drinking	1	0	0	0	0	0	1
Had Been Using Drugs	0	0	0	0	0	0	0
Asleep	0	0	0	0	0	0	0
Fatigued	0	0	0	0	0	0	0
ill	0	0	0	0	0	0	0
Handicapped	0	0	0	0	0	0	0
Other	0	0	1	0	0	0	1
Total	33	24	4	1	0	0	62

		Drive	er Age by Ur	nit Type		
Age	Driver	Bicycle	SnowMobile	Pedestrian	ATV	Total
09-Under	0	0	0	0	0	0
10-14	0	0	0	0	0	0
15-19	2	0	0	0	0	2
20-24	6	0	0	0	0	6
25-29	4	0	0	0	0	4
30-39	14	0	0	0	0	14
40-49	15	0	0	0	0	15
50-59	16	0	0	0	0	16
60-69	2	0	0	0	0	2
70-79	1	0	0	0	0	1
80-Over	1	0	0	0	0	1
Unknown	1	0	0	0	0	1
Total	62	0	0	0	0	62

Fixed Object Struck	
Fixed Object Struck	Total
1-Construction, Barricades Equipment, etc.	0
2-Traffic Signal	0
3-R.R. Crossing Device	0
4-Light Pole	0
5-Utility Pole (Tel. Electrical)	0
6-Sign Structure Post	0
7-Mail Boxes or Posts	0
8-Other Poles, posts or supports	1
9-Fire Hydrant/Parking Meter	0
10-Tree or Shrubbery	0
11-Crash Cushion	1
12-Median Safety Barrier	3
13-Bridge Piers (including protective guard rails)	0
14-Other Guardrails	3
15-Fencing (not median barrier)	0
16-Culvert Headwall	0
17-Embankment, Ditch, Curb	3
18-Building, Wall	1
19-Rock Outcrops or Ledge	0
20-Other	1
21-Gate or Cable	0
22-Pressure Ridge	0
Total	13

Traffic Control Devices	
Traffic Control Device	Total
1-Traffic Signals (Stop & Go)	0
2-Traffic Flashing	1
3-Overhead Flashers	3
4-Stop Signs - All Approaches	0
5-Stop Signs - Other	1
6-Yield Sign	0
7-Curve Warning Sign	0
8-Officer, Flagman, School Patrol	0
9-School Bus Stop Arm	0
10-School Zone Sign	0
11-R.R. Crossing Device	0
12-No Passing Zone	0
13-None	18
14-Other	10
Total	33

Road Character	
Road Character	Total
1-Level Straight	17
2-Level Curved	0
3-On Grade Straight	12
4-On Grade Curved	4
5-Top of Hill Straight	0
6-Top of Hill Curved	0
7-Bottom of Hill Straight	0
8-Bottom of Hill Curved	0
9-Other	0
Total	33

Inj	ury Data	
Severity Code	Injury Crashes	Number Of Injuries
K	0	0
Α	1	1
В	3	3
С	4	9
PD	25	0
Total	33	13

Light	
Light	Total
1-Dawn (Morning)	2
2-Daylight	26
3-Dusk (Evening)	3
4-Dark (Street Lights On)	1
5-Dark (No Street Lights)	1
6-Dark (Street Lights Off)	0
7-Other	0
Total	33

			Crashes by		pe and Type	e of Locatio	n			
Crash Type	Straight Road	Curved Road	Three Leg Intersection	Four Leg Intersection	Five Leg Intersection	Driveways	Bridges	Interchanges	Other	Total
Object in Road	1	0	0	0	0	0	0	0	0	1
Rear End / Sideswipe	12	3	0	0	0	0	0	1	8	24
Head-on / Sideswipe	0	0	0	0	0	0	0	0	0	0
Intersection Movement	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	0	0	0
Train	0	0	0	0	0	0	0	0	0	0
Ran Off Road	5	0	0	0	0	0	0	0	0	5
All Other Animal	0	0	0	0	0	0	0	0	0	0
Bike	0	0	0	0	0	0	0	0	0	0
Other	1	1	0	0	0	0	0	0	0	2
Jackknife	0	0	0	0	0	0	0	0	0	0
Rollover	0	0	0	0	0	0	0	0	0	0
Fire	0	0	0	0	0	0	0	0	0	0
Submersion	0	0	0	0	0	0	0	0	0	0
Rock Thrown	0	0	0	0	0	0	0	0	0	0
Bear	0	0	0	0	0	0	0	0	0	0
Deer	0	0	0	0	0	0	0	0	0	0
Moose	1	0	0	0	0	0	0	0	0	1
Total	20	4	0	0	0	0	0	1	8	33

				• • • • • • • • • • • • • • • • • • •							
			Crashes b	y Weather, L	ight Condit	ion and Ro	ad Surface				
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	Ice, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Blowing Sand or Dust											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Clear											
Dark (No Street Lights)	0	1	0	0	0	0	0	0	0	0	1
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	17	0	0	0	0	0	0	0	0	17
Dusk (Evening)	0	1	0	0	0	0	0	0	0	0	1
Other	0	0	0	0	0	0	0	0	0	0	0
Cloudy											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	1	0	0	0	0	0	0	0	0	1
Daylight	0	3	0	0	0	0	0	0	0	0	3
Dusk (Evening)	0	1	0	0	0	0	0	0	0	0	1
Other	0	0	0	0	0	0	0	0	0	0	0
Fog, Smog, Smoke											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0

Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	lce, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Other											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Rain											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Severe Cross Winds											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Sleet, Hail, Freezing Rain											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	1	0	1
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0

			Crashes b	y Weather, L	ight Condit	ion and Ro	ad Surface				
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	Ice, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Snow											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	1	0	0	1
Dawn (Morning)	0	0	0	0	0	0	0	0	1	0	1
Daylight	0	0	0	0	0	0	0	2	3	0	5
Dusk (Evening)	0	0	0	0	0	0	0	0	1	0	1
Other	0	0	0	0	0	0	0	0	0	0	0
TOTAL	0	24	0	0	0	0	0	3	6	0	33

## **Crash Summary Report**

		Report Selections and Inp	arameters	ı
REPORT SELECTIONS				
✓ Crash Summary I	✓ Section Det	tail	☐ 1320 Included ☐ 1320 & Driver Report Included	
REPORT DESCRIPTION I 95 NB York Toll area				
REPORT PARAMETERS Year 2005, Start Month 1 thro	ugh Year 2007 E	End Month: 12		
Route: 0095X	Start Node: 5 End Node: 5		☐ Exclude First Node ☐ Exclude Last Node	

			No	des										
Node	Route - MP	Node Description	U/R	Total		Injur	y Cra	shes		Percent	Annual M C	rash	Critical	CRF
				Crashes	K	Α	В	С	PD	Injury	Ent-Veh R	Rate	Rate	
58311	0095X - 6.18	BRG 6228, I 95 NB under BERWICK RD	2	0	0	0	0	0	0	0.0 State	9.698 wide Crash Rate:	0.00 0.13	0.29	0.00
58866	0095X - 6.44	Int of I 95 NB, RAMP OFF TO YORK CONNECTOR	1	2	0	0	0	1	1	50.0 State	9.698 wide Crash Rate:	0.07	0.10	0.00
58868	0095X - 7.10	Int of I 95 NB, RAMP ON FROM YORK CONNECTOR	1	0	0	0	0	0	0	0.0 State	8.311 wide Crash Rate:	0.00	0.10	0.00
57692	0095X - 7.19	Non-Int I 95 NB	1	8	0	0	0	1	7	12.5 State	8.311 wide Crash Rate:	0.32	0.10	3.23
58312	0095X - 9.43	BRG 1311, I 95 NB over CAPE NEDDICK RIVER	1	0	0	0	0	0	0	0.0 State	8.311 wide Crash Rate:	0.00	0.10	0.00
Study \	ears: 3.00	NODE TOTA	LS:	10	0	0	0	2	8	20.0	44.329	0.08	0.10	0.76

# Crash Summary I

							Section	ns									
Start	End	Element	Offset	Route - MP	Section	J/R	Total		Inju	ry Cr	ashes		Percent	Annual	Crash	Critical	CRF
Node	Node		Begin - End		Length		Crashes	K	Α	В	С	PD	Injury	HMVM	Rate	Rate	
57692 Non-Int I 98	58312 5 NB	239220	0 - 2.24	0095X - 4.95 INT 95 NB	2.24	1	19	1	0	1	1	16	15.8 State	0.18617 wide Crash Ra	34.02 ite: 59.02	84.61	0.00
58311 BRG 6228,	58866 I 95 NB ur	239686 nder BERWICI	0 - 0.26	0095X - 6.18 INT 95 NB	0.26	1	8	0	0	0	1	7	12.5 State	0.02521 wide Crash Ra	105.76 ite: 59.02	124.36	0.00
58866 Int of I 95 N CONNECT	*	240301 OFF TO YORK	0 - 0.66	0095X - 6.44 INT 95 NB	0.66	1	9	0	0	0	0	9	0.0 State	0.05158 wide Crash Ra	58.17 te: 59.02	106.10	0.00
57692 Non-Int I 98	58868 5 NB	239221	0 - 0.09	0095X - 7.10 INT 95 NB	0.09	1	5	0	0	0	1	4	20.0 State	0.00748 wide Crash Ra	222.82 ite: 59.02	168.85	1.32
Study Ye	ears: 3	.00		Section Totals:	3.25		41	1	0	1	3	36	12.2	0.27044	50.54	80.37	0.63
				Grand Totals:	3.25		51	1	0	1	5	44	13.7	0.27044	62.86	85.84	0.73

# Crash Summary

						Sect	ion De	etails						
Start	End	Element	Offset	Route - MP	Total		-	iry Cra			Crash Report	Crash Date	Crash	Injury
Node	Node		Begin - End		Crashes	K	Α	В	С	PD			Mile Point	Degree
58311	58866	239686	0 - 0.26	0095X - 6.18	8	0	0	0	1	7	2006-11814	05/20/2006	6.28	PD
											2006-7531	03/10/2006	6.28	PD
											2005-32193	11/21/2005	6.28	PD
											2006-22933	09/20/2006	6.28	PD
											2005-7127	03/04/2005	6.28	PD
											2005-32949	11/19/2005	6.28	PD
											2006-22931	09/19/2006	6.38	С
											2007-34547	12/20/2007	6.41	PD
58866	58868	240301	0 - 0.66	0095X - 6.44	9	0	0	0	0	9	2006-28986	11/12/2006	6.54	PD
											2006-32074	12/20/2006	6.54	PD
											2006-10538	04/24/2006	6.54	PD
											2006-6133	03/03/2006	6.54	PD
											2006-11168	05/15/2006	6.64	PD
											2005-22723	08/13/2005	6.74	PD
											2006-12582	05/28/2006	6.74	PD
											2007-13529	06/13/2007	6.74	PD
											2007-4001	02/20/2007	6.94	PD
57692	58868	239221	0 - 0.09	0095X - 7.10	5	0	0	0	1	4	2006-14467	06/14/2006	7.12	С
											2006-22932	09/19/2006	7.13	PD
											2005-10262	03/31/2005	7.15	PD
											2006-21169	09/02/2006	7.18	PD
											2006-21747	09/04/2006	7.18	PD

# Crash Summary

						Sect	ion De	etails						
Start	End	Element	Offset	Route - MP	Total		-	ry Cra			Crash Report	Crash Date	Crash	Injury
Node	Node		Begin - End		Crashes	K	Α	В	С	PD			Mile Point	Degree
57692	58312	239220	0 - 2.24	0095X - 7.19	19	1	0	1	1	16	2006-21601	08/27/2006	7.39	PD
											2006-8591	04/06/2006	7.39	PD
											2006-20626	08/27/2006	7.49	PD
											2007-14323	06/17/2007	7.59	PD
											2005-9502	03/17/2005	7.59	PD
											2006-170	01/05/2006	7.69	PD
											2006-15195	06/25/2006	8.19	PD
											2007-30804	12/03/2007	8.19	PD
											2006-12960	06/03/2006	8.19	PD
											2006-32903	12/30/2006	8.19	PD
											2005-1123	01/08/2005	8.29	PD
											2006-2651	01/30/2006	8.39	PD
											2005-1892	01/24/2005	8.69	PD
											2005-27852	10/09/2005	9.19	K
											2006-24012	10/01/2006	9.23	С
											2007-33622	12/20/2007	9.33	PD
											2007-33627	12/20/2007	9.33	PD
											2007-30799	11/21/2007	9.41	В
											2007-32755	12/10/2007	9.41	PD

Totals: 41 1 0 1 3 36

# Maine Department Of Transportation - Traffic Engineering, Crash Records Section Crash Summary II - Characteristics

										Cr	ashes	s by D	ay an	d Ho	ur											
						AM			Hour of Day									PM								
Day Of Week	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	Un	Tot
SUNDAY	0	0	1	1	0	0	0	0	0	0	0	0	1	1	0	0	0	1	2	1	0	0	0	0	0	8
MONDAY	0	1	0	0	0	1	2	0	1	0	0	0	2	1	0	0	0	0	0	0	1	0	0	0	0	9
TUESDAY	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	1	0	0	0	0	0	0	0	0	4
WEDNESDAY	0	1	0	0	0	0	1	0	0	0	0	0	1	1	0	1	2	0	0	0	0	0	0	0	0	7
THURSDAY	0	0	0	0	0	1	0	0	0	0	2	0	1	2	0	0	0	0	1	1	0	0	0	0	0	8
FRIDAY	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	3	1	1	0	0	0	0	0	0	0	6
SATURDAY	0	0	0	1	0	0	0	0	0	0	0	1	1	0	2	1	0	1	0	1	0	0	0	1	0	9
Totals	0	2	1	2	0	2	3	1	1	0	2	1	6	5	2	8	4	3	3	3	1	0	0	1	0	51

С	rashes	by Ye	ar and Mon	th	Ve	ehicle Co	unts by Type	
Month	2005	2006	2007	Total	Unit Type	Total	Unit Type	Total
JANUARY	2	2	0	4	1-2 Door	10	32-3 Axle Tractor with Tandem Axle Semi	9
JANUART	_	2	U	4	2-4 Door	24	33-3 Axle Tractor with Tridem Axle Semi	0
FEBRUARY	0	0	1	1	3-Convertible	0	35-3 Axle Tractor with Single Axle Semi & 2	0
MARCH	3	3	0	6	4-Station Wagon	1	Axle Trailer	•
APRIL	2	2	0	4	5-Van	10	36-3 Axle Tractor with Tandem Axle Semi & 2 Axle Trailer	0
	2		U	4	6-Pickup Truck	7	37-5 Axle Semi; Split Trailer Tandem	0
MAY	0	3	0	3	7-SUV	9	38-6 Axle Semi; Split Trailer Tandem with	0
JUNE	0	4	2	6	10-Truck Tractor Only (Bobtail)	0	Center Axle	U
		4		4	12-School Bus	0	39-6 Axle; Standard Trailer Tandem with Center	0
JULY	0	1	0	1	13-Motor Home	0	Axle	
AUGUST	2	2	0	4	14-Motorcycle	0	40-4 Axle Single Unit	0
SEPTEMBER	1	6	0	7	15-Moped	0	42-4 Axle Tractor with Tandem Axle Semi	0
	4		0		16-Motor Bike	0	50-Any Other Axle Configuration	0
OCTOBER	1	2	0	3	17-Bicycle	0	60-Other Unit	3
NOVEMBER	2	2	1	5	18-Snowmobile	0	70-ATV	0
DECEMBER	0	2	5	7	20-2 Axle Single Unit with Dual Tires	7	81-2 Axle Bus	0
					21-2 Axle Tractor with Single Axle Semi	0	82-3 Axle Bus	0
Total	13	29	9	51	22-2 Axle Tractor with Tandem Axle Semi	0	98-Farm Vehicles / Tractors	0
					25-2 Axle Tractor with Single Axle Semi & 2 Axle Trailer	0	99-Unknown	0
					30-3 Axle Single Unit	2	Total	83
					31-3 Axle Tractor with Single Axle Semi	1		

Crashes by Apparen	t Cont	ributir	ng Fac	tor An	d Driv	er	
Apparent Contributing Factor	Dr 1	Dr 2	Dr 3	Dr 4	Dr 5	Other	Tota
No Improper Action	20	15	2	0	0	0	37
Failure to Yield Right of Way	1	3	0	0	0	0	4
Illegal Unsafe Speed	14	0	0	0	0	0	14
Following Too Close	0	3	0	0	0	0	3
Disregard Traffic Control Device	0	0	0	0	0	0	0
Driving Left of Center Not Passing	0	0	0	0	0	0	0
Improper Passing, Overtaking	1	0	0	0	0	0	1
Improper Unsafe Lane Change	5	1	0	0	0	0	6
Improper Parking Start, Stop	0	0	0	0	0	0	0
Improper Turn	0	0	0	0	0	0	0
Unsafe Backing	0	0	0	0	0	0	0
No Signal or Improper Signal	0	0	0	0	0	0	0
Impeding Traffic	0	0	0	0	0	0	0
Driver Inattention, Distraction	3	7	0	0	0	0	10
Driver Inexperience	0	0	0	0	0	0	0
Pedestrian Violation Error	0	0	0	0	0	0	0
Physical Impairment	2	0	0	0	0	0	2
Vision Obscured, Windshield Glass	0	0	0	0	0	0	0
Vision Obscured, Sun, Headlights	0	0	0	0	0	0	0
Other Vision Obscurement	0	1	0	0	0	0	1
Other Human Violation Factor	2	0	0	0	0	0	2
Hit and Run	0	0	0	0	0	0	0
Defective Brakes	0	0	0	0	0	0	0
Defective Tire, Tire Failure	2	0	0	0	0	0	2
Defective Lights	0	0	0	0	0	0	0
Defective Suspension	0	0	0	0	0	0	0
Defective Steering	0	0	0	0	0	0	0
Other Vehicle Defect or Factor	0	0	0	0	0	0	0
Unknown	1	0	0	0	0	0	1
Total	51	30	2	0	0	0	83

Crashes by Ap	paren	t Phys	sical C	onditi	on An	d Drive	er
Apparent Physical Condition	Dr 1	Dr 2	Dr 3	Dr 4	Dr 5	Other	Total
Normal	46	29	1	0	0	0	76
Under the Influence	2	0	0	0	0	0	2
Had Been Drinking	0	0	0	0	0	0	0
Had Been Using Drugs	0	0	0	0	0	0	0
Asleep	2	0	0	0	0	0	2
Fatigued	0	0	0	0	0	0	0
ill	0	0	0	0	0	0	0
Handicapped	0	0	0	0	0	0	0
Other	1	1	0	0	0	0	2
Total	51	30	1	0	0	0	82

		Drive	er Age by Ur	it Type		
Age	Driver	Bicycle	SnowMobile	Pedestrian	ATV	Total
09-Under	1	0	0	0	0	1
10-14	0	0	0	0	0	0
15-19	7	0	0	0	0	7
20-24	11	0	0	0	0	11
25-29	8	0	0	0	0	8
30-39	13	0	0	0	0	13
40-49	20	0	0	0	0	20
50-59	9	0	0	0	0	9
60-69	8	0	0	0	0	8
70-79	3	0	0	0	0	3
80-Over	1	0	0	0	0	1
Unknown	2	0	0	0	0	2
Total	83	0	0	0	0	83

Fixed Object Struck	
Fixed Object Struck	Total
1-Construction, Barricades Equipment, etc.	0
2-Traffic Signal	0
3-R.R. Crossing Device	0
4-Light Pole	1
5-Utility Pole (Tel. Electrical)	0
6-Sign Structure Post	1
7-Mail Boxes or Posts	0
8-Other Poles, posts or supports	0
9-Fire Hydrant/Parking Meter	0
10-Tree or Shrubbery	2
11-Crash Cushion	0
12-Median Safety Barrier	10
13-Bridge Piers (including protective guard rails)	0
14-Other Guardrails	0
15-Fencing (not median barrier)	0
16-Culvert Headwall	0
17-Embankment, Ditch, Curb	0
18-Building, Wall	0
19-Rock Outcrops or Ledge	0
20-Other	3
21-Gate or Cable	0
22-Pressure Ridge	0
Total	17

Traffic Control Devices	
Traffic Control Device	Total
1-Traffic Signals (Stop & Go)	1
2-Traffic Flashing	0
3-Overhead Flashers	4
4-Stop Signs - All Approaches	0
5-Stop Signs - Other	1
6-Yield Sign	2
7-Curve Warning Sign	0
8-Officer, Flagman, School Patrol	0
9-School Bus Stop Arm	0
10-School Zone Sign	0
11-R.R. Crossing Device	0
12-No Passing Zone	0
13-None	32
14-Other	11
Total	51

Road Character	
Road Character	Total
1-Level Straight	30
2-Level Curved	2
3-On Grade Straight	14
4-On Grade Curved	5
5-Top of Hill Straight	0
6-Top of Hill Curved	0
7-Bottom of Hill Straight	0
8-Bottom of Hill Curved	0
9-Other	0
Total	51

Injury Data								
Severity Code	Injury Crashes	Number Of Injuries						
K	1	1						
Α	0	0						
В	1	1						
С	5	6						
PD	44	0						
Total	51	8						

Light	
Light	Total
1-Dawn (Morning)	2
2-Daylight	35
3-Dusk (Evening)	1
4-Dark (Street Lights On)	7
5-Dark (No Street Lights)	6
6-Dark (Street Lights Off)	0
7-Other	0
Total	51

			Crashes by	y Crash Ty <sub>l</sub>	pe and Type	e of Locatio	n			
Crash Type	Straight Road	Curved Road	Three Leg Intersection	Four Leg Intersection	Five Leg Intersection	Driveways	Bridges	Interchanges	Other	Total
Object in Road	8	1	0	0	0	0	0	0	1	10
Rear End / Sideswipe	17	2	2	0	0	0	0	0	7	28
Head-on / Sideswipe	0	0	0	0	0	0	0	0	0	0
Intersection Movement	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	0	0	0
Train	0	0	0	0	0	0	0	0	0	0
Ran Off Road	3	2	0	0	0	0	0	0	0	5
All Other Animal	0	0	0	0	0	0	0	0	0	0
Bike	0	0	0	0	0	0	0	0	0	0
Other	2	0	0	0	0	0	0	0	1	3
Jackknife	0	0	0	0	0	0	0	0	0	0
Rollover	0	0	0	0	0	0	0	0	0	0
Fire	0	0	0	0	0	0	0	0	0	0
Submersion	0	0	0	0	0	0	0	0	0	0
Rock Thrown	0	0	0	0	0	0	0	0	0	0
Bear	0	0	0	0	0	0	0	0	0	0
Deer	5	0	0	0	0	0	0	0	0	5
Moose	0	0	0	0	0	0	0	0	0	0
Total	35	5	2	0	0	0	0	0	9	51

			Crashes b	y Weather, L	ignt Condit	ion and Ro	ad Surrace				
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	Ice, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Blowing Sand or Dust											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Clear											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	6	0	0	0	0	0	0	0	0	6
Dawn (Morning)	0	1	0	0	0	0	0	0	0	1	2
Daylight	0	19	0	0	0	0	0	0	0	0	19
Dusk (Evening)	0	1	0	0	0	0	0	0	0	0	1
Other	0	0	0	0	0	0	0	0	0	0	0
Cloudy											
Dark (No Street Lights)	0	1	0	0	0	0	0	0	0	0	1
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	6	0	0	0	0	0	0	0	1	7
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Fog, Smog, Smoke											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0

# Maine Department Of Transportation - Traffic Engineering, Crash Records Section Crash Summary II - Characteristics

Crashes by Weather, Light Condition and Road Surface											
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	Ice, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Other											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Rain											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	4	4
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	1	1
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	2	2
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Severe Cross Winds											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Sleet, Hail, Freezing Rain											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	1	0	0	0	0	0	1	0	2
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0

			Crashes b	y Weather, L	ight Condit	ion and Ro	ad Surface				
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	Ice, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Snow											
Dark (No Street Lights)	0	0	0	1	0	0	0	0	0	0	1
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	1	0	0	0	1	2	1	5
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
TOTAL	0	34	1	2	0	0	0	1	3	10	51

# Crash Summary Report

# Report Selections and Input Parameters

REPORT SELECTIONS  Crash Summary I		✓ Section Detail	☑Crash Summary II
REPORT DESCRIPTION I 95 SB York Toll area			
REPORT PARAMETERS Year 2006, Start Month 1 through	gh Year 2008 End Month: 12	Start Offset: 0	□Exclude First Node

End Offset: 0

☐ Exclude Last Node

End Node: 58356

# 

			No	odes										
Node	Route - MP	Node Description	U/R	Total		Injury	y Cra	shes		Percent	Annual M C	Crash	Critical	CRF
				Crashes	K	Α	В	С	PD	Injury	Ent-Veh I	Rate	Rate	
58357	0095S - 294.68	Non-Int I 95 SB	1	0	0	0	0	0	0	0.0 State	7.760 wide Crash Rate	0.00 0.04	0.13	0.00
57693	0095S - 296.19	Non-Int I 95 SB	1	9	0	0	1	2	6	33.3 State	7.760 wide Crash Rate	0.39 0.04	0.13	3.09
58871	0095S - 296.30	Int of I 95 SB, RAMP B OFF TO YORK CONNECTOR	1	0	0	0	0	0	0	0.0 State	7.760 wide Crash Rate	0.00 e: 0.04	0.13	0.00
58869	0095S - 296.94	Int of I 95 SB, RAMP A FROM YORK CONNECTOR	1	0	0	0	0	0	0	0.0 State	9.600 wide Crash Rate	0.00 e: 0.04	0.12	0.00
58356	0095S - 297.18	BRG 6228, I 95 SB under ST RTE 91	2	0	0	0	0	0	0	0.0 State	9.603 wide Crash Rate	0.00 e: 0.13	0.29	0.00
Study \	Years: 3.00	NODE TOTAL	ALS:	9	0	0	1	2	6	33.3	42.483	0.07	0.11	0.63

# Crash Summary I

							Section	ns									
Start	End	Element	Offset	Route - MP	Section		Total		Inju	ry Cr	ashes		Percent	Annual	Crash	Critical	CRF
Node	Node		Begin - End		Length		Crashes	K	Α	В	С	PD	Injury	HMVM	Rate	Rate	
57693 Non-Int I 98	58357 5 SB	239222	0 - 1.51	0095S - 294.68 INT 95 SB	1.51	1	17	0	0	1	3	13	23.5 State	0.11717 wide Crash Ra	48.36 te: 59.31	91.35	0.00
57693 Non-Int I 98	58871 5 SB	2522897	0 - 0.11	0095S - 296.19 INT 95 SB	0.11	1	5	0	0	0	1	4	20.0 State	0.00854 wide Crash Ra	195.25 te: 59.31	163.76	1.19
58869 Int of I 95 S CONNECT	*	2522901 A FROM YORK	0 - 0.64	0095S - 296.30 INT 95 SB	0.64	1	4	0	1	0	1	2	50.0 State	0.04639 wide Crash Ra	28.74 te: 59.31	108.89	0.00
58356 BRG 6228,	58869 , I 95 SB ui	2522903 nder ST RTE 9	0 - 0.24	0095S - 296.70 INT 95 SB	0.24	1	1	0	0	0	0	1	0.0 State	0.02305 wide Crash Ra	14.46 te: 59.31	127.52	0.00
Study Ye	ears: 3	3.00		Section Totals:	2.50		27	0	1	1	5	20	25.9	0.19515	46.12	84.38	0.55
				Grand Totals:	2.50		36	0	1	2	7	26	27.8	0.19515	61.49	90.14	0.68

# Crash Summary

						Sect	ion D	etails						
Start	End	Element	Offset	Route - MP	Total		-	iry Cra			Crash Report	Crash Date	Crash	Injury
Node	Node		Begin - End		Crashes	K	Α	В	С	PD			Mile Point	Degree
57693	58357	239222	0 - 1.51	0095S - 294.68	17	0	0	1	3	13	2008-31224	12/11/2008	294.69	С
											2008-26005	10/27/2008	294.69	PD
											2008-20619	07/06/2008	294.88	PD
											2008-18009	07/25/2008	295.39	С
											2007-6429	03/16/2007	295.43	PD
											2006-1590	01/23/2006	295.49	PD
											2008-28711	11/14/2008	295.69	PD
											2007-7738	04/01/2007	295.79	PD
											2006-12693	05/25/2006	295.99	В
											2007-6418	03/17/2007	295.99	PD
											2008-34047	12/31/2008	295.99	PD
											2006-16830	06/29/2006	295.99	PD
											2008-13446	05/27/2008	296.09	С
											2006-100	01/01/2006	296.09	PD
											2008-24422	10/07/2008	296.18	PD
											2008-19908	08/13/2008	296.18	PD
											2006-2867	02/02/2006	296.18	PD
57693	58871	2522897	0 - 0.11	0095S - 296.19	5	0	0	0	1	4	2007-13496	06/02/2007	296.20	С
											2006-14254	06/15/2006	296.29	PD
											2006-354	01/11/2006	296.29	PD
											2006-18246	07/26/2006	296.29	PD
											2006-19418	08/04/2006	296.29	PD
58869	58871	2522901	0 - 0.64	0095S - 296.30	4	0	1	0	1	2	2008-21924	09/01/2008	296.54	PD
											2006-32072	12/15/2006	296.64	Α
											2008-24533	10/14/2008	296.74	С
											2008-16354	07/06/2008	296.84	PD
58356	58869	2522903	0 - 0.24	0095S - 296.94	1	0	0	0	0	1	2007-20241	07/08/2007	297.06	PD
				 Totals:	27	0	1	1	5	20				

# Maine Department Of Transportation - Traffic Engineering, Crash Records Section Crash Summary II - Characteristics

										Cr	ashes	s by D	ay an	d Ho	ur											
						AM					ŀ	Hour o	of Day						PM							
Day Of Week	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	Un	Tot
SUNDAY	0	0	0	0	0	1	1	0	0	0	0	1	1	0	1	0	0	1	0	0	1	0	0	0	0	7
MONDAY	1	0	0	0	0	0	0	1	0	0	1	0	0	0	0	1	1	0	0	0	0	0	0	0	0	5
TUESDAY	0	0	0	0	0	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	0	0	0	0	0	3
WEDNESDAY	0	0	0	0	0	0	0	0	1	0	2	0	0	1	0	0	0	1	0	0	0	0	0	0	0	5
THURSDAY	0	0	0	0	0	0	0	1	0	0	0	1	0	1	0	2	0	0	0	0	0	0	0	1	0	6
FRIDAY	0	0	0	0	0	1	0	0	0	0	0	0	0	1	1	2	2	0	0	1	0	0	0	0	0	8
SATURDAY	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	2
Totals	1	0	0	0	0	2	1	3	1	0	4	3	2	4	2	5	3	2	0	1	1	0	0	1	0	36

С	rashes	by Ye	ear and Month		Ve	hicle Co	unts by Type	
Month	2006	2007	2008	Total	Unit Type	Total	Unit Type	Total
JANUARY	3	0	1	4	1-2 Door	7	32-3 Axle Tractor with Tandem Axle Semi	6
	_		•	7	2-4 Door	27	33-3 Axle Tractor with Tridem Axle Semi	4
FEBRUARY	1	0	0	1	3-Convertible	0	35-3 Axle Tractor with Single Axle Semi & 2	0
MARCH	0	3	1	4	4-Station Wagon	3	Axle Trailer	0
APRIL	0	1	0	4	5-Van	6	36-3 Axle Tractor with Tandem Axle Semi & 2 Axle Trailer	0
	U	ı	U	I	6-Pickup Truck	11	37-5 Axle Semi; Split Trailer Tandem	0
MAY	1	0	1	2	7-SUV	5	38-6 Axle Semi; Split Trailer Tandem with	0
JUNE	2	1	0	3	10-Truck Tractor Only (Bobtail)	0	Center Axle	U
		•	0	7	12-School Bus	0	39-6 Axle: Standard Trailer Tandem with Center	0
JULY	1	3	3	1	13-Motor Home	0	Axle	
AUGUST	1	1	1	3	14-Motorcycle	0	40-4 Axle Single Unit	0
SEPTEMBER	0	1	1	2	15-Moped	0	42-4 Axle Tractor with Tandem Axle Semi	0
				_	16-Motor Bike	0	50-Any Other Axle Configuration	0
OCTOBER	0	0	3	3	17-Bicycle	0	60-Other Unit	0
NOVEMBER	0	0	2	2	18-Snowmobile	0	70-ATV	0
DECEMBER	1	0	3	4	20-2 Axle Single Unit with Dual Tires	1	81-2 Axle Bus	0
					21-2 Axle Tractor with Single Axle Semi	0	82-3 Axle Bus	0
Total	10	10	16	36	22-2 Axle Tractor with Tandem Axle Semi	0	98-Farm Vehicles / Tractors	0
					25-2 Axle Tractor with Single Axle Semi & 2 Axle Trailer	0	99-Unknown	1
					30-3 Axle Single Unit	0	Total	71
					31-3 Axle Tractor with Single Axle Semi	0		

Crashes by Apparen	t Cont	ributin	ıg Fac	tor An	d Driv	er	
Apparent Contributing Factor	Dr 1	Dr 2	Dr 3	Dr 4	Dr 5	Other	Tota
No Improper Action	6	22	3	1	0	0	32
Failure to Yield Right of Way	5	2	1	0	0	0	8
Illegal Unsafe Speed	9	0	1	0	0	0	10
Following Too Close	2	0	0	0	0	0	2
Disregard Traffic Control Device	1	0	0	0	0	0	1
Driving Left of Center Not Passing	0	0	0	0	0	0	0
Improper Passing, Overtaking	0	0	0	0	0	0	0
Improper Unsafe Lane Change	3	0	0	0	0	0	3
Improper Parking Start, Stop	1	0	0	0	0	0	1
Improper Turn	0	0	0	0	0	0	0
Unsafe Backing	0	0	0	0	0	0	0
No Signal or Improper Signal	0	0	0	0	0	0	0
Impeding Traffic	0	0	0	0	0	0	0
Driver Inattention, Distraction	6	3	0	0	0	0	9
Driver Inexperience	1	1	0	0	0	0	2
Pedestrian Violation Error	0	0	0	0	0	0	0
Physical Impairment	0	0	0	0	0	0	0
Vision Obscured, Windshield Glass	0	0	0	0	0	0	0
Vision Obscured, Sun, Headlights	0	0	0	0	0	0	0
Other Vision Obscurement	0	0	0	0	0	0	0
Other Human Violation Factor	0	0	0	0	0	0	0
Hit and Run	0	0	0	0	0	0	0
Defective Brakes	1	0	0	0	0	0	1
Defective Tire, Tire Failure	0	0	0	0	0	0	0
Defective Lights	0	0	0	0	0	0	0
Defective Suspension	0	0	0	0	0	0	0
Defective Steering	0	0	0	0	0	0	0
Other Vehicle Defect or Factor	0	0	0	0	0	0	0
Unknown	1	1	0	0	0	0	2
Total	36	29	5	1	0	0	71

Crashes by Ap	paren	t Phys	sical C	onditi	on An	d Drive	er
Apparent Physical Condition	Dr 1	Dr 2	Dr 3	Dr 4	Dr 5	Other	Total
Normal	34	29	4	1	0	0	68
Under the Influence	0	0	0	0	0	0	0
Had Been Drinking	2	0	0	0	0	0	2
Had Been Using Drugs	0	0	0	0	0	0	0
Asleep	0	0	0	0	0	0	0
Fatigued	0	0	0	0	0	0	0
ill	0	0	0	0	0	0	0
Handicapped	0	0	0	0	0	0	0
Other	0	0	1	0	0	0	1
Total	36	29	5	1	0	0	71

		Drive	er Age by Ur	nit Type		
Age	Driver	Bicycle	SnowMobile	Pedestrian	ATV	Total
09-Under	0	0	0	0	0	0
10-14	0	0	0	0	0	0
15-19	2	0	0	0	0	2
20-24	5	0	0	0	0	5
25-29	9	0	0	0	0	9
30-39	17	0	0	0	0	17
40-49	12	0	0	0	0	12
50-59	15	0	0	0	0	15
60-69	5	0	0	0	0	5
70-79	3	0	0	0	0	3
80-Over	2	0	0	0	0	2
Unknown	1	0	0	0	0	1
Total	71	0	0	0	0	71

Fixed Object Struck	
Fixed Object Struck	Total
1-Construction, Barricades Equipment, etc.	1
2-Traffic Signal	0
3-R.R. Crossing Device	0
4-Light Pole	0
5-Utility Pole (Tel. Electrical)	0
6-Sign Structure Post	0
7-Mail Boxes or Posts	0
8-Other Poles, posts or supports	1
9-Fire Hydrant/Parking Meter	0
10-Tree or Shrubbery	0
11-Crash Cushion	1
12-Median Safety Barrier	1
13-Bridge Piers (including protective guard rails)	0
14-Other Guardrails	3
15-Fencing (not median barrier)	0
16-Culvert Headwall	0
17-Embankment, Ditch, Curb	2
18-Building, Wall	0
19-Rock Outcrops or Ledge	0
20-Other	1
21-Gate or Cable	0
22-Pressure Ridge	0
Total	10

Traffic Control Devices	
Traffic Control Device	Total
1-Traffic Signals (Stop & Go)	0
2-Traffic Flashing	2
3-Overhead Flashers	2
4-Stop Signs - All Approaches	0
5-Stop Signs - Other	1
6-Yield Sign	0
7-Curve Warning Sign	0
8-Officer, Flagman, School Patrol	0
9-School Bus Stop Arm	0
10-School Zone Sign	0
11-R.R. Crossing Device	0
12-No Passing Zone	0
13-None	20
14-Other	11
Total	36

Road Character	
Road Character	Total
1-Level Straight	22
2-Level Curved	0
3-On Grade Straight	9
4-On Grade Curved	4
5-Top of Hill Straight	1
6-Top of Hill Curved	0
7-Bottom of Hill Straight	0
8-Bottom of Hill Curved	0
9-Other	0
Total	36

Inju	ury Data	
Severity Code	Injury Crashes	Number Of Injuries
K	0	0
Α	1	1
В	2	3
С	7	10
PD	26	0
Total	36	14

Light	
Light	Total
1-Dawn (Morning)	2
2-Daylight	28
3-Dusk (Evening)	1
4-Dark (Street Lights On)	2
5-Dark (No Street Lights)	3
6-Dark (Street Lights Off)	0
7-Other	0
Total	36

Crashes by Crash Type and Type of Location												
Crash Type	Straight Road	Curved Road	Three Leg Intersection	Four Leg Intersection	Five Leg Intersection	Driveways	Bridges	Interchanges	Other	Total		
Object in Road	3	0	0	0	0	0	0	0	0	3		
Rear End / Sideswipe	12	2	0	0	0	0	0	0	10	24		
Head-on / Sideswipe	0	0	0	0	0	0	0	0	0	0		
Intersection Movement	0	0	0	0	0	0	0	0	0	0		
Pedestrians	0	0	0	0	0	0	0	0	0	0		
Train	0	0	0	0	0	0	0	0	0	0		
Ran Off Road	2	1	0	0	0	0	0	0	0	3		
All Other Animal	0	0	0	0	0	0	0	0	0	0		
Bike	0	0	0	0	0	0	0	0	0	0		
Other	2	1	0	0	0	0	0	0	1	4		
Jackknife	0	0	0	0	0	0	0	0	0	0		
Rollover	0	0	0	0	0	0	0	0	0	0		
Fire	0	0	0	0	0	0	0	0	0	0		
Submersion	0	0	0	0	0	0	0	0	0	0		
Rock Thrown	0	0	0	0	0	0	0	0	0	0		
Bear	0	0	0	0	0	0	0	0	0	0		
Deer	1	0	0	0	0	0	0	0	0	1		
Moose	1	0	0	0	0	0	0	0	0	1		
Total	21	4	0	0	0	0	0	0	11	36		

			Crashes b	y Weather, L	ight Condit	ion and Ro	ad Surface				
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	Ice, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Blowing Sand or Dust											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Clear											
Dark (No Street Lights)	0	1	0	0	0	0	0	0	0	0	1
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	1	0	0	0	0	0	0	0	0	1
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	19	0	0	0	0	0	0	0	1	20
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Cloudy											
Dark (No Street Lights)	0	1	0	0	0	0	0	0	0	0	1
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	1	0	0	0	0	0	0	0	0	1
Daylight	0	3	0	0	0	0	0	0	0	0	3
Dusk (Evening)	0	1	0	0	0	0	0	0	0	0	1
Other	0	0	0	0	0	0	0	0	0	0	0
Fog, Smog, Smoke											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0

# Maine Department Of Transportation - Traffic Engineering, Crash Records Section Crash Summary II - Characteristics

			Crashes b	y Weather, L	ight Condit	ion and Ro	ad Surface				
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	Ice, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Other											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Rain											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	1	1
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Severe Cross Winds											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Sleet, Hail, Freezing Rain											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	1	0	0	0	0	0	1	0	2
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0

			Crashes b	y Weather, L	ight Condit	ion and Ro	ad Surface				
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	Ice, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Snow											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	1	0	1
Dawn (Morning)	0	0	0	0	0	0	0	0	1	0	1
Daylight	0	0	0	0	0	0	0	1	2	0	3
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
TOTAL	0	27	1	0	0	0	0	1	5	2	36

# Crash Summary Report

## Report Selections and Input Parameters

REPORT SELECTIONS  Crash Summary I		✓ Section Detail	✓ Crash Summary II
REPORT DESCRIPTION I 95 NB York Toll area			
REPORT PARAMETERS Year 2006, Start Month 1 thro			
Route: 0095X	Start Node: 58311	Start Offset: 0	□ Exclude First Node

End Offset: 0

☐ Exclude Last Node

End Node: 58312

# 

			No	des									
Node	Route - MP	Node Description	U/R	Total		Injur	y Cra	shes		Percent Annual M C	rash	Critical	CRF
				Crashes	K	Α	В	С	PD	Injury Ent-Veh F	Rate	Rate	
58311	0095X - 6.18	BRG 6228, I 95 NB under BERWICK RD	2	0	0	0	0	0	0	0.0 9.698 Statewide Crash Rate:	0.00	0.29	0.00
58866	0095X - 6.44	Int of I 95 NB, RAMP OFF TO YORK CONNECTOR	1	1	0	0	0	0	1	0.0 9.698 Statewide Crash Rate:	0.03	0.12	0.00
58868	0095X - 7.10	Int of I 95 NB, RAMP ON FROM YORK CONNECTOR	1	0	0	0	0	0	0	0.0 8.311 Statewide Crash Rate:	0.00	0.12	0.00
57692	0095X - 7.19	Non-Int I 95 NB	1	10	0	0	0	0	10	0.0 8.311 Statewide Crash Rate:	0.40 0.04	0.12	3.26
58312	0095X - 9.43	BRG 1311, I 95 NB over CAPE NEDDICK RIVER	1	0	0	0	0	0	0	0.0 8.311 Statewide Crash Rate:	0.00	0.12	0.00
Study Y	/ears: 3.00	NODE TOTA	LS:	11	0	0	0	0	11	0.0 44.329	0.08	0.11	0.75

# Crash Summary I

							Section	ns									
Start	End	Element	Offset	Route - MP	Section	U/R	Total		Inju	ry Cr	ashes		Percent	Annual	Crash	Critical	CRF
Node	Node		Begin - End		Length		Crashes	K	Α	В	С	PD	Injury	HMVM	Rate	Rate	
57692 Non-Int I 95	58312 5 NB	239220	0 - 2.24	0095X - 4.95 INT 95 NB	2.24	1	23	0	0	3	1	19	17.4 State	0.18617 wide Crash Ra	41.18 te: 59.31	84.96	0.00
58311 BRG 6228,	58866 I 95 NB ur	239686 oder BERWICI	0 - 0.26	0095X - 6.18 INT 95 NB	0.26	1	6	0	0	0	1	5	16.7 State	0.02521 wide Crash Ra	79.32 te: 59.31	124.83	0.00
58866 Int of I 95 N CONNECT	*	240301 OFF TO YORK	0 - 0.66	0095X - 6.44 INT 95 NB	0.66	1	10	0	0	0	1	9	10.0 State	0.05158 wide Crash Ra	64.63 te: 59.31	106.51	0.00
57692 Non-Int I 95	58868 5 NB	239221	0 - 0.09	0095X - 7.10 INT 95 NB	0.09	1	7	0	0	0	1	6	14.3 State	0.00748 wide Crash Ra	311.95 te: 59.31	169.46	1.84
Study Ye	ears: 3	.00		Section Totals:	3.25		46	0	0	3	4	39	15.2	0.27044	56.70	80.72	0.70
				Grand Totals:	3.25		57	0	0	3	4	50	12.3	0.27044	70.26	86.32	0.81

# Crash Summary

						Sect	ion D							
Start	End	Element	Offset	Route - MP	Total		-	iry Cra			Crash Report	Crash Date	Crash	Injury
Node	Node		Begin - End		Crashes	K	Α	В	С	PD			Mile Point	Degree
58311	58866	239686	0 - 0.26	0095X - 6.18	6	0	0	0	1	5	2006-11814	05/20/2006	6.28	PD
											2006-7531	03/10/2006	6.28	PD
											2006-22933	09/20/2006	6.28	PD
											2008-21365	08/29/2008	6.31	PD
											2006-22931	09/19/2006	6.38	С
											2007-34547	12/20/2007	6.41	PD
58866	58868	240301	0 - 0.66	0095X - 6.44	10	0	0	0	1	9	2008-16353	07/03/2008	6.54	С
											2006-32074	12/20/2006	6.54	PD
											2006-28986	11/12/2006	6.54	PD
											2006-10538	04/24/2006	6.54	PD
											2006-6133	03/03/2006	6.54	PD
											2006-11168	05/15/2006	6.64	PD
											2007-13529	06/13/2007	6.74	PD
											2006-12582	05/28/2006	6.74	PD
											2007-4001	02/20/2007	6.94	PD
											2008-24854	10/11/2008	6.94	PD
57692	58868	239221	0 - 0.09	0095X - 7.10	7	0	0	0	1	6	2008-17372	07/10/2008	7.11	PD
											2008-18683	08/01/2008	7.11	PD
											2008-4032	02/20/2008	7.11	PD
											2006-14467	06/14/2006	7.12	С
											2006-22932	09/19/2006	7.13	PD
											2006-21747	09/04/2006	7.18	PD
											2006-21169	09/02/2006	7.18	PD

# Crash Summary

						Sect	ion De	etails						
Start	End	Element	Offset	Route - MP	Total		Inju	ıry Cra			Crash Report	Crash Date	Crash	Injury
Node	Node		Begin - End		Crashes	K	Α	В	С	PD			Mile Point	Degree
57692	58312	239220	0 - 2.24	0095X - 7.19	23	0	0	3	1	19	2006-24012	10/01/2006	7.29	С
											2008-32990	12/21/2008	7.29	PD
											2006-21601	08/27/2006	7.39	PD
											2006-8591	04/06/2006	7.39	PD
											2006-20626	08/27/2006	7.49	PD
											2008-2617	02/07/2008	7.49	PD
											2007-14323	06/17/2007	7.59	PD
											2008-4284	02/22/2008	7.69	PD
											2006-170	01/05/2006	7.69	PD
											2008-3883	02/17/2008	8.19	В
											2006-32903	12/30/2006	8.19	PD
											2007-30804	12/03/2007	8.19	PD
											2006-15195	06/25/2006	8.19	PD
											2006-12960	06/03/2006	8.19	PD
											2008-25100	10/20/2008	8.31	PD
											2006-2651	01/30/2006	8.39	PD
											2008-15534	06/15/2008	8.93	В
											2008-2376	02/05/2008	9.23	PD
											2008-8378	03/28/2008	9.33	PD
											2007-33627	12/20/2007	9.33	PD
											2007-33622	12/20/2007	9.33	PD
											2007-30799	11/21/2007	9.41	В
											2007-32755	12/10/2007	9.41	PD
				Totals	: 46	0	0	3	4	39				

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# Maine Department Of Transportation - Traffic Engineering, Crash Records Section Crash Summary II - Characteristics

										Cr	ashes	s by D	ay an	d Ho	ur											
						AM					ŀ	Hour o	of Day						PM							
Day Of Week	12	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	Un	Tot
SUNDAY	1	0	0	1	0	0	0	0	0	0	0	0	1	1	0	0	1	1	3	1	0	0	1	0	0	11
MONDAY	0	1	0	0	0	0	1	0	1	0	0	0	2	0	0	0	0	1	0	0	1	0	0	0	0	7
TUESDAY	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	3	1	0	0	0	0	0	0	0	0	5
WEDNESDAY	0	1	0	1	0	0	1	0	0	0	0	1	1	1	0	1	1	0	0	0	0	0	0	0	0	8
THURSDAY	0	0	0	1	0	1	0	0	0	0	2	0	1	2	0	1	0	0	1	0	0	0	0	0	0	9
FRIDAY	0	0	0	0	0	0	0	1	0	0	1	1	1	0	0	3	0	0	1	1	0	0	0	0	0	9
SATURDAY	0	0	0	2	1	0	0	0	0	0	0	0	1	0	1	1	0	1	0	0	0	0	0	1	0	8
Totals	1	2	0	5	1	1	2	1	2	0	3	2	7	4	1	9	3	3	5	2	1	0	1	1	0	57

С	rashes	by Ye	ear and Month		Ve	hicle Co	unts by Type	
Month	2006	2007	2008	Total	Unit Type	Total	Unit Type	Total
JANUARY	2	0	0	2	1-2 Door	6	32-3 Axle Tractor with Tandem Axle Semi	13
		U	•		2-4 Door	26	33-3 Axle Tractor with Tridem Axle Semi	0
FEBRUARY	0	1	5	6	3-Convertible	0	35-3 Axle Tractor with Single Axle Semi & 2	0
MARCH	3	0	1	4	4-Station Wagon	2	Axle Trailer 36-3 Axle Tractor with Tandem Axle Semi & 2	0
APRIL	2	0	0	2	5-Van	8	Axle Trailer	U
			-		6-Pickup Truck	10	37-5 Axle Semi; Split Trailer Tandem	0
MAY	3	0	0	3	7-SUV	10	38-6 Axle Semi; Split Trailer Tandem with	0
JUNE	4	2	2	8	10-Truck Tractor Only (Bobtail)	0	Center Axle	•
JULY	1	0	3	4	12-School Bus	0	39-6 Axle; Standard Trailer Tandem with Center	0
	•				13-Motor Home	0	Axle	
AUGUST	2	0	3	5	14-Motorcycle	0	40-4 Axle Single Unit	0
SEPTEMBER	6	0	0	6	15-Moped	0	42-4 Axle Tractor with Tandem Axle Semi	0
OCTOBER	2	0	2	4	16-Motor Bike	0	50-Any Other Axle Configuration	0
	_				17-Bicycle	0	60-Other Unit	2
NOVEMBER	2	1	1	4	18-Snowmobile	0	70-ATV	0
DECEMBER	2	5	2	9	20-2 Axle Single Unit with Dual Tires	8	81-2 Axle Bus	0
Total	29		19	57	21-2 Axle Tractor with Single Axle Semi	0	82-3 Axle Bus	0
Total	29	9	19	5/	22-2 Axle Tractor with Tandem Axle Semi	0	98-Farm Vehicles / Tractors	0
					25-2 Axle Tractor with Single Axle Semi & 2 Axle Trailer	0	99-Unknown	0
					30-3 Axle Single Unit	2	Total	88
					31-3 Axle Tractor with Single Axle Semi	1		

Crashes by Apparen	t Cont	ributin	ig Fac	tor An	d Driv	er	
Apparent Contributing Factor	Dr 1	Dr 2	Dr 3	Dr 4	Dr 5	Other	Tota
No Improper Action	14	20	2	0	0	0	36
Failure to Yield Right of Way	0	3	0	0	0	0	3
Illegal Unsafe Speed	14	0	0	0	0	0	14
Following Too Close	5	1	0	0	0	0	6
Disregard Traffic Control Device	0	0	0	0	0	0	0
Driving Left of Center Not Passing	0	0	0	0	0	0	0
Improper Passing, Overtaking	1	0	0	0	0	0	1
Improper Unsafe Lane Change	7	0	0	0	0	0	7
Improper Parking Start, Stop	0	0	0	0	0	0	0
Improper Turn	0	0	0	0	0	0	0
Unsafe Backing	0	0	0	0	0	0	0
No Signal or Improper Signal	0	0	0	0	0	0	0
Impeding Traffic	0	0	0	0	0	0	0
Driver Inattention, Distraction	5	4	0	0	0	0	9
Driver Inexperience	1	0	0	0	0	0	1
Pedestrian Violation Error	0	0	0	0	0	0	0
Physical Impairment	2	0	0	0	0	0	2
Vision Obscured, Windshield Glass	0	0	0	0	0	0	0
Vision Obscured, Sun, Headlights	0	0	0	0	0	0	0
Other Vision Obscurement	0	1	0	0	0	0	1
Other Human Violation Factor	4	0	0	0	0	0	4
Hit and Run	0	0	0	0	0	0	0
Defective Brakes	0	0	0	0	0	0	0
Defective Tire, Tire Failure	1	0	0	0	0	0	1
Defective Lights	0	0	0	0	0	0	0
Defective Suspension	0	0	0	0	0	0	0
Defective Steering	0	0	0	0	0	0	0
Other Vehicle Defect or Factor	2	0	0	0	0	0	2
Unknown	1	0	0	0	0	0	1
Total	57	29	2	0	0	0	88

Crashes by Ap	paren	t Phys	sical C	onditi	on An	d Drive	er
Apparent Physical Condition	Dr 1	Dr 2	Dr 3	Dr 4	Dr 5	Other	Total
Normal	50	28	1	0	0	0	79
Under the Influence	2	0	0	0	0	0	2
Had Been Drinking	0	0	0	0	0	0	0
Had Been Using Drugs	0	0	0	0	0	0	0
Asleep	4	0	0	0	0	0	4
Fatigued	0	0	0	0	0	0	0
ill	0	0	0	0	0	0	0
Handicapped	0	0	0	0	0	0	0
Other	1	1	0	0	0	0	2
Total	57	29	1	0	0	0	87

		Drive	er Age by Ur	nit Type		
Age	Driver	Bicycle	SnowMobile	Pedestrian	ATV	Total
09-Under	1	0	0	0	0	1
10-14	0	0	0	0	0	0
15-19	6	0	0	0	0	6
20-24	12	0	0	0	0	12
25-29	8	0	0	0	0	8
30-39	13	0	0	0	0	13
40-49	20	0	0	0	0	20
50-59	13	0	0	0	0	13
60-69	9	0	0	0	0	9
70-79	2	0	0	0	0	2
80-Over	2	0	0	0	0	2
Unknown	2	0	0	0	0	2
Total	88	0	0	0	0	88

Fixed Object Struck	
Fixed Object Struck	Total
1-Construction, Barricades Equipment, etc.	0
2-Traffic Signal	0
3-R.R. Crossing Device	0
4-Light Pole	1
5-Utility Pole (Tel. Electrical)	0
6-Sign Structure Post	4
7-Mail Boxes or Posts	0
8-Other Poles, posts or supports	1
9-Fire Hydrant/Parking Meter	0
10-Tree or Shrubbery	1
11-Crash Cushion	0
12-Median Safety Barrier	11
13-Bridge Piers (including protective guard rails)	0
14-Other Guardrails	0
15-Fencing (not median barrier)	0
16-Culvert Headwall	0
17-Embankment, Ditch, Curb	0
18-Building, Wall	0
19-Rock Outcrops or Ledge	0
20-Other	3
21-Gate or Cable	0
22-Pressure Ridge	0
Total	21

Traffic Control Devices	
Traffic Control Device	Total
1-Traffic Signals (Stop & Go)	0
2-Traffic Flashing	1
3-Overhead Flashers	4
4-Stop Signs - All Approaches	0
5-Stop Signs - Other	1
6-Yield Sign	1
7-Curve Warning Sign	0
8-Officer, Flagman, School Patrol	0
9-School Bus Stop Arm	0
10-School Zone Sign	0
11-R.R. Crossing Device	0
12-No Passing Zone	0
13-None	33
14-Other	17
Total	57

Road Character	
Road Character	Total
1-Level Straight	34
2-Level Curved	1
3-On Grade Straight	16
4-On Grade Curved	6
5-Top of Hill Straight	0
6-Top of Hill Curved	0
7-Bottom of Hill Straight	0
8-Bottom of Hill Curved	0
9-Other	0
Total	57

Inj	ury Data	
Severity Code	Injury Crashes	Number Of Injuries
K	0	0
Α	0	0
В	3	3
С	4	5
PD	50	0
Total	57	8

Light	
Light	Total
1-Dawn (Morning)	1
2-Daylight	40
3-Dusk (Evening)	1
4-Dark (Street Lights On)	8
5-Dark (No Street Lights)	6
6-Dark (Street Lights Off)	1
7-Other	0
Total	 57

			Crashes by	y Crash Ty <sub>l</sub>	pe and Type	e of Locatio	n			
Crash Type	Straight Road	Curved Road	Three Leg Intersection	Four Leg Intersection	Five Leg Intersection	Driveways	Bridges	Interchanges	Other	Total
Object in Road	8	2	0	0	0	0	0	0	3	13
Rear End / Sideswipe	17	2	1	0	0	0	0	0	8	28
Head-on / Sideswipe	0	0	0	0	0	0	0	0	0	0
Intersection Movement	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	0	0	0
Train	0	0	0	0	0	0	0	0	0	0
Ran Off Road	3	2	0	0	0	0	0	0	0	5
All Other Animal	0	0	0	0	0	0	0	0	0	0
Bike	0	0	0	0	0	0	0	0	0	0
Other	1	0	0	0	0	0	0	0	2	3
Jackknife	1	0	0	0	0	0	0	0	0	1
Rollover	0	0	0	0	0	0	0	0	0	0
Fire	2	0	0	0	0	0	0	0	0	2
Submersion	0	0	0	0	0	0	0	0	0	0
Rock Thrown	0	0	0	0	0	0	0	0	0	0
Bear	0	0	0	0	0	0	0	0	0	0
Deer	5	0	0	0	0	0	0	0	0	5
Moose	0	0	0	0	0	0	0	0	0	0
Total	37	6	1	0	0	0	0	0	13	57

# Maine Department Of Transportation - Traffic Engineering, Crash Records Section Crash Summary II - Characteristics

			Crachoe	w Waathar I	ight Condit	ion and Pa	ad Surface				
			Crasnes	y Weather, L	igni Condi	ion and Ro					
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	lce, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Blowing Sand or Dust											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Clear											
Dark (No Street Lights)	0	1	0	0	0	0	0	0	0	0	1
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	5	0	0	0	0	0	0	0	0	5
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	20	0	0	0	0	0	0	0	0	20
Dusk (Evening)	0	1	0	0	0	0	0	0	0	0	1
Other	0	0	0	0	0	0	0	0	0	0	0
Cloudy											
Dark (No Street Lights)	0	1	0	0	0	0	0	0	0	0	1
Dark (Street Lights Off)	0	1	0	0	0	0	0	0	0	0	1
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	1	1
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	7	0	0	0	0	0	0	0	1	8
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Fog, Smog, Smoke											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0

			Crashes b	y Weather, L	ight Condit	ion and Ro	ad Surface				
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	Ice, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total
Other											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Rain											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	4	4
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	1	1
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	2	2
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Severe Cross Winds											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	0	0	0	0	0	0	0	0	0
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0
Sleet, Hail, Freezing Rain											
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0
Dark (Street Lights On)	0	0	0	0	0	0	0	0	0	0	0
Dawn (Morning)	0	0	0	0	0	0	0	0	0	0	0
Daylight	0	0	1	0	0	0	0	0	2	0	3
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0
Other	0	0	0	0	0	0	0	0	0	0	0

	Crashes by Weather, Light Condition and Road Surface													
Weather Light	Debris	Dry	Ice, Packed Snow, Not Sanded	Ice, Packed Snow, Sanded	Muddy	Oily	Other	Snow Slush, Not Sanded	Snow, Slush, Sanded	Wet	Total			
Snow														
Dark (No Street Lights)	0	0	0	0	0	0	0	0	0	0	0			
Dark (Street Lights Off)	0	0	0	0	0	0	0	0	0	0	0			
Dark (Street Lights On)	0	0	1	0	0	0	0	0	0	0	1			
Dawn (Morning)	0	0	0	0	0	0	0	0	1	0	1			
Daylight	0	0	0	1	0	0	0	2	3	1	7			
Dusk (Evening)	0	0	0	0	0	0	0	0	0	0	0			
Other	0	0	0	0	0	0	0	0	0	0	0			
TOTAL	0	36	2	1	0	0	0	2	6	10	57			

# APPENDIX H RENEWAL & REPLACEMENT – MAINTENANCE PROGRAM

### YORK TOLL PLAZA (MM7.3) MAINTENANCE DATA COMPARISON

### Detailed Renewal and Replacement Program Estimate for the Existing Plaza

				Major Plaza R	Rehabilitation (3)			, and the second	Asphalt Pavement (	8)	Toll Sys	stem (9)	Roadway (10)	Buildings (10)	N	12)	Total	
		Tunnel Rehab. Program (2)	Profile Reconstruction	on & Final Overlay (4)	Concrete Bumper Reconstruction (5)	Replace Booths, Island & Lane Slabs, Canopy (17 Lanes) (6)	EZ-Pass Remove & Reset (7)	Mill and Fill Overlay 11/2" (100%)	Mill and Fill Overlay 11/2" (50%)	Pavement Crack Sealing	Equipment Routine/Annual Maintenance	Equipment Replacement (17 Lanes)	Routine Plaza Maintenance	Routine Maintenance	Tandem Booth Operations	Plaza Paint and Surface Sealing	Canopy Roof Sealing	
Uni	it Price (1)	\$61,171	\$520,478	\$2,326,129	\$79,939	\$7,426,300	\$541,059	\$2,754,096	\$1,377,048	\$12,301	\$216,424	\$106,090	\$74,263	\$21,218	\$5,305	\$106,090	\$53,045	Annual and R&R
C	Quantity	6	6	1	6	1	1	1	1	1	1	17	1	1	1	1	1	Expenditures (2010\$)
	Unit	Lane	Phase	LS	Phase	LS	LS	LS	LS	LS	LS	Lane	LS	LS	LS	LS	LS	1
	Interval	1	1	16	1	1	1	16	8	4	1	10	1	1	1	5	10	1
	2010	\$185,000	\$520,000	-	\$80,000	\$1,240,000	\$90,000			\$12,300	\$216,400	-	\$74,200	\$21,200	\$5,300	\$106,000		\$2,550,400
	2011	\$185,000	\$520,000		\$80,000	\$1,240,000	\$90,000	tior		, , , , , , , , , , , , , , , , , , , ,	\$216,400		\$74,200	\$21,200	\$5,300	,,	\$53,000	\$2,485,100
	2012		\$520,000		\$80,000	\$1,240,000	\$90,000	era			\$216,400		\$74,200	\$21,200	\$5,300			\$2,247,100
	2013		\$520,000		\$80,000	\$1,240,000	\$90,000	ď			\$216,400		\$74,200	\$21,200	\$5,300			\$2,247,100
	2014		\$520,000		\$80,000	\$1,240,000	\$90,000	Şe.			\$216,400		\$74,200	\$21,200	\$5,300			\$2,247,100
	2015		\$520,000	\$2,330,000	\$80,000	\$1,240,000	\$90,000	er <del>s</del>		\$10,500	\$216,400	\$1,800,000	\$74,200	\$21,200	\$5,300	\$106,000		\$6,493,600
	2016							ò			\$216,400		\$74,200	\$21,200	\$5,300			\$317,100
	2017							la			\$216,400		\$74,200	\$21,200	\$5,300			\$317,100
	2018							Fir			\$216,400		\$74,200	\$21,200	\$5,300			\$317,100
<b>⊢</b>	2019							8		\$12,300	\$216,400		\$74,200	\$21,200	\$5,300			\$329,400
ě	2020							en			\$216,400		\$74,200	\$21,200	\$5,300	\$106,000		\$423,100
>	2021							stm.			\$216,400		\$74,200	\$21,200	\$5,300		\$53,000	\$370,100
	2022							si <u>k</u>			\$216,400		\$74,200					\$317,100
	2023							¥	\$1,380,000	\$6,150	\$216,400		\$74,200	¥,-**	\$5,300			\$1,703,250
	2024							ille			\$216,400		\$74,200		\$5,300			\$317,100
	2025							Pro			\$216,400	\$1,800,000	\$74,200			\$106,000		\$2,223,100
	2026							i			\$216,400		\$74,200		\$5,300			\$317,100
	2027	\$185,000						рө		\$12,300	\$216,400		\$74,200		\$5,300			\$514,400
	2028	\$185,000						pn			\$216,400		\$74,200					\$502,100
	2029	\$185,000						Incl			\$216,400		\$74,200		\$5,300			\$502,100
	2030	\$185,000						_			\$216,400		\$74,200	\$21,200	\$5,300	\$106,000		\$608,100
	Total	\$1,110,000	\$3,120,000	\$2,330,000	\$480,000	\$7,440,000	\$540,000	\$0	\$1,380,000	\$53,550	\$4,544,400	\$3,600,000	\$1,558,200	\$445,200	\$111,300	\$530,000	\$106,000	\$27,348,650
																	Annual (14)	\$1,367,433

### Detailed Renewal and Replacement Program Estimate for a New Plaza at Existing Location

			Major Plaza F	Rehabilitation (3)			Asphalt Pavement (8)			Toll System (9) Roadway (1			(10) Buildings (10)		Total		
	Tunnel Rehab. Program (2)	Profile Reconstruction	on & Final Overlay (4)	Concrete Bumper Reconstruction (5)	Replace Booths, Island & Lane Slabs, Canopy (17 Lanes) (6)	EZ-Pass Remove & Reset (7)	Mill and Fill Overlay 11/2" (100%)	Mill and Fill Overlay 11/2" (50%)	Pavement Crack Sealing	Equipment Routine/Annual Maintenance	Equipment Replacement (17 Lanes)	Routine Plaza Maintenance	Routine Maintenance	Tandem Booth Operations	Plaza Paint and Surface Sealing	Canopy Roof Sealing	
Unit Price (1)	\$61,171	\$520,478	\$2,326,129	\$79,939	\$7,426,300	\$541,059	\$2,754,096	\$1,377,048	\$12,301	\$216,424	\$106,090	\$74,263	\$21,218	\$5,305	\$106,090	\$53,045	Annual and R&R
Quantity	6	6	1	6	1	1	1	1	1	1	17	1	1	1	1	1	Expenditures (2010
Unit	Lane	Phase	LS	Phase	LS	LS	LS	LS	LS	LS	Lane	LS	LS	LS	LS	LS	1
Interval	1	1	16	1	1	1	16	8	4	1	10	1	1	1	5	10	
2010										\$216,400		\$74,200	\$21,200				\$311,8
2011										\$216,400		\$74,200	\$21,200				\$311,8
2012										\$216,400		\$74,200	\$21,200				\$311,8
2013										\$216,400		\$74,200	\$21,200				\$311,8
2014									\$12,300	\$216,400		\$74,200	\$21,200				\$324,1
2015										\$216,400		\$74,200	\$21,200		\$106,000	0	\$417,8
2016										\$216,400		\$74,200	\$21,200				\$311,8
2017										\$216,400		\$74,200	\$21,200				\$311,8
2018								\$1,380,000	\$6,150	\$216,400		\$74,200	\$21,200				\$1,697,9
2019										\$216,400		\$74,200	\$21,200				\$311,8
2020										\$216,400	\$1,800,000	\$74,200	\$21,200		\$106,000	\$53,000	
2021										\$216,400		\$74,200	\$21,200				\$311,8
2022									\$12,300	\$216,400		\$74,200	\$21,200				\$324,1
2023										\$216,400		\$74,200	\$21,200				\$311,8
2024										\$216,400		\$74,200	\$21,200				\$311,8
2025										\$216,400		\$74,200	\$21,200		\$106,000	0	\$417,8
2026							\$2,750,000	)	\$12,300	\$216,400		\$74,200	\$21,200				\$3,074,1
2027										\$216,400		\$74,200	\$21,200				\$311,8
2028										\$216,400		\$74,200	\$21,200				\$311,8
2029										\$216,400		\$74,200	\$21,200				\$311,8
2030	\$185,000								\$12,300	\$216,400	\$1,800,000	\$74,200	\$21,200		\$106,000	\$53,000	\$2,468,1
Total	\$185,000	\$0	\$0	\$0	\$0	\$0	\$2,750,000	\$1,380,000	\$55,350	\$4,544,400	\$3,600,000	\$1,558,200	\$445,200	\$	0 \$424,000	0\$106,000	\$15,048,1
																Annual (14)	\$752,40
erential (13)	\$925,000	\$3,120,000	\$2,330,000	\$480,000	\$7,440,000	\$540,000	-\$2,750,000	\$0	-\$1,800	\$0	\$0	\$0	\$0	\$111,30	0 \$106,000	50 \$0	\$12,300,5
			1				1	1		·	·	-	1		1	Annual (14)	\$615,02

- 1. Construction prices are in 2010 dollars, as derived from MTA, MDOT, and recent industry unit pricing for materials and work of similar or like nature.
- 2. Tunnel rehab. program consists of work similarly performed on 9 lanes at the York Plaza to date, and for which 6 lanes remain to be rehabilitated. The work includes rehabilitation of concrete slabs; replacement of PVC conduits with galvanized rigid metal conduit; replacement of electrical wires, wire ways, and conduits; replacement of AVI/AVC/LC wire ways; replacement of loop detectors; pressure injection of concrete cracks and construction joints; sealing and caulking of rehabilitated concrete slabs; and signing and maintenance of traffic. Similar to other work described below, this would be phased 3
- 3. Phased construction work is based on the assumption that this work would most expeditiously occur by utilizing the whole lane, and that a maximum of 3 lanes can be taken out of service at one time, in order that the Plaza remain at an acceptable level of service. Based on established plaza volumes and previous field experience, this has been approximated at 3 lanes per phase. Therefore to cross the 17 lane plaza would require 5.6 (say 6) phases.
- 4. Profile reconstruction is based on a profile developed to specifically address and correct the incoming 200' of approach either side of the plaza where excessive sag results in low-bed hangups, concrete slab/tunnel impact, and poor drainage. Reconstruction consists of exist. pavement removal, fill gravel to subbase grade, then 12" of new pavement to profile grade.
- 5. Concrete bumper reconstruction consists of wrecking out the old bumpers, prep. and place new concrete slab, and mount 35 mph crash cushion with safety lighting. This would be done in conjunction with the profile phasing.
- 6. Replacement of the booths, island and lane slabs, and canopy, is work considered programmatic in nature. This work would need to occur every 20 years in order to maintain the tunnel top, approach slabs, booths, bumpers, and the canopy in sound condition, in good working order, and to address advances in technology, changes in the worker's environment, and future demands of the
- automotive/transportation industry. The most recent work of this nature at York occurred in 1996 with the advent of Transpass. Having this work simultaneous with the reminder of plaza work minimizes overal lane closures. It is assumed to occur on a similar 6-year phased construction cycle in order for the plaza to operate acceptably during construction.

- 7. E-Z Pass Remove and reset is that work associated with booth replacement in order to remove and reinstall up-to-date ETC equipment. Based on industry standards, this is estimated at \$30,000 per lane, and would occur at the same time as the booth and island work.
- 8. Mill and fill overlay consists of the periodic (20 yr) milling of existing pavement, recapping with 1 1/2" of new pavement, and striping for 1800 If of approach either side of the plaza. The 50% mill and fill operation assumes that every 10 years, that approximately 1/2 of the entire plaza would need this type of repair, on an as-needed basis (some lanes receive more wear than others). Note: the amount is different when the mill & fill is combined with the profile reconstruction due to the interior 200' either side of the plaza having just been paved. This mill & fill would be timed to occur along with the final phase of profile work so to result in a uniform "like new" total plaza area. Pavement crack sealing is assumed to occur on a periodic basis (every 4 years) to help maintain the pavement surface, and also occurs with
- 9. Toll System maintenance consists of two components; the routine/annual maintenance of ETC equipment (as currently contracted with Transcore), and the industry expected life cycle of plaza equipment, which has been estimated at \$106,000 per lane every 10 years.
- 10. Roadway and building maintenance are those annual costs associated with the standard maintenance of the plaza area and the buildings (snowplowing, mowing, boiler maintenance, etc.).
- 11. Tandem booth operations is the annual cost associated with the seasonal set-up and take-down of the tandem toll booths. These are currently needed to help process the seasonally high summer traffic
- 12. Plaza paint, surface sealing, and canopy roof sealing are those periodic applications of paint, concrete sealer, and asphaltic roof sealer that are assumed to be needed to keep these plaza components in sound condition, good appearance, and to protect the steel and concrete beneath.
- 13. Differential consists of the cost of the existing plaza maintenance minus the cost of a new plaza at existing location maintenance costs.
- 14. Annual is the overall cost of the 20 year period and are reported in constant 2010 dollars.

# APPENDIX I GLOSSARY

### **GLOSSARY**

- <u>30<sup>th</sup> Highest Hour traffic</u>: The volume of traffic present in a single hour that is exceeded only 29 times in a typical year.
- AASHTO: American Association of State Highway and Transportation Officials
- **Absolute Peak Hour traffic**: The volume of traffic present in a single hour that is never exceeded in a typical year.
- <u>All Electronic Tolling (AET)</u>: A type of tolling where tolls are collected either by an electronic transponder or by video tolling; there is no cash collection option.
- <u>Capacity</u>: The amount of vehicles in a given time frame (e.g. vehicles per hour) that a roadway or facility can accommodate; typically reported for a stated level of service, e.g. length of backup or average delay per vehicle.
- <u>Cash Tolling (Conventional Tolling)</u>: The method of toll collection in which a patron is required to stop at a toll booth, pay cash for the toll and then resume highway speed.
- <u>Design Guidelines</u>: A set of recommended rules or criteria that have been developed over time based on experience and that are to be applied to in similar situations. Typically design guidelines are developed by a national organization with responsibilities to protect the safety of a large group or population, e.g. traffic light operation is contained in Manual of Uniform Traffic Control Devices published by the Federal Highway Administration to be used across the Nation.
- <u>Electronic Toll Collection (ETC)</u>: The method of toll collection in which tolls are collected without cash via the use of electronic means.
- Existing Site Evaluation: The title of (this) report developed by HNTB at the request of the MTA that documents the re-evaluation of options for rehabilitating/reconstructing the York Toll Plaza at its existing site or in close proximity and which recommends option(s) that warrant being carried forward for further consideration
- **E-ZPass**: A brand of electronic toll collection system utilized on the Maine Turnpike and other Northeast states.
- **FHWA:** Federal Highway Administration
- **Footprint**: The outer boundary or approximate limit of work for the proposed toll plaza design.
- <u>High Crash Location (HCL)</u>: A link or node that has eight or more reported crashes over the past three years <u>and</u> the link or node must have a "critical rate factor" (CRF) over 1.00. (The critical rate factor is a ratio of the crash rate at a particular link or node divided by the statewide crash rate average for a similar type of facility. The term "rate" is calculated by number of crashes divided by the number of millions of annual entering vehicles).
- <u>Highway Speed Tolling</u>: A toll collection technique in which users pay a toll through some form of electronic means at highway speeds (55-65mph), e.g. E-ZPass. Similar to the dedicated E-ZPass toll lanes now in use on the Maine Turnpike with the difference being traveling at normal highway speeds versus the 10 miles per hour as posted currently. Same as Open Road Tolling.

- <u>HNTB Corporation</u>: General Engineering Consultant to the Maine Turnpike Authority.
- <u>LD534</u>: A Resolve directing the Maine Turnpike Authority to Study the Relocation of the York Toll Booth enacted by the Maine Legislature in 2007.
- <u>Location Study Report</u>: The title given to a report that, as currently planned, will be given to the U.S. Army Corps of Engineers for purposes of documenting the study of the York Toll Plaza. The report will contain information on conditions, deficiencies, options explored to rehabilitate and reconstruct, existing site options, alternative site options and recommendations for proceeding further with the York Toll Plaza Replacement.
- <u>Maine Turnpike Authority</u>: a quasi-state agency created by the Maine Legislature in 1941 to construct, manage and operate the 109 mile, toll highway from Kittery to Augusta.
- <u>Mainline</u>: The thru travel portion of the highway; as opposed to entrance and exit ramps, service plazas etc..
- <u>Merge</u>: The driving maneuver in which an entering vehicle from an on-ramp makes to move onto the mainline with other mainline traffic.
- MUTCD: Manual on Uniform Traffic Control Devices
- <u>Node and Link System</u>: A system established by the Maine Department of Transportation to catalog traffic statistics. A four-digit number is assigned to each node (intersection, major bridge, railroad crossing, and crossing of town, county or urban compact lines etc.). The segment of road that connects the nodes is referred to as a link. Data can now be compiled based on these node and/or link numbers.
- **Open Road Tolling**: A toll collection technique in which users pay a toll through some form of electronic means at highway speeds (55-65mph), e.g. E-ZPass. Similar to the dedicated E-ZPass toll lanes now in use on the Maine Turnpike with the difference being traveling at normal highway speeds versus the 10 miles per hour as posted currently. Same as Highway Speed Tolling.
- <u>Pre-paid video products</u>: Various types of accounts that can be set up to allow toll payment based on a video camera capturing a license plate number at one or more toll plazas.
- <u>Processing Rate</u>: The average rate at which tolls can be collected during a specific period of time and for a specific number of lanes, often reported as per lane per hour, e.g. 320 vehicles per lane per hour can pay their toll.
- **Profile grade**: The slope of the roadway measured along mainline.
- **Queue**: Traffic backup.
- **Ramp**: Portion of roadway where vehicles enter or exit the mainline.
- Reversible Lane: A toll lane that can be operated in either direction, e.g. Northbound and Southbound directions.
- <u>Slow-speed dedicated ETC lanes</u>: A toll lane that only accepts Electronic Toll Collection and only at a slow speed; currently 10 mph on the Maine Turnpike.
- State of the Practice: State of the Practice and Recommendations on Traffic Control Strategies at Toll Plazas; a report under a project initiative by the Federal Highway Administration to identify the 'state of the practice' for traffic control strategies at toll

- plazas. The document summarizes recommended guidelines for agencies and departments that operate or plan to design and build such facilities.
- Tandem Booth (Tandem Lane Operation): A toll collection method that expands the capacity of cash collection by adding a tolling booth inline and immediately downstream of an existing booth. Tolls can be collected by two toll attendants simultaneously for groups of 3 or 4 vehicles. Typical increase in capacity is approximately 30%.
- **Tangent**: A straight portion of highway.
- <u>Transponder and Receiver</u>: Two pieces of equipment necessary to have Electronic Toll Collection. A transponder sends a signal identifying an account number and a receiver collects the transponders signal to assess a specific toll for that location.
- <u>Tunnel</u>: For many toll plazas the best way to provide toll attendants with safe access to the toll booths is by a tunnel built beneath the toll plaza. In addition, the tunnel can serve as housing for electrical and data infrastructure necessary for toll collection.
- <u>Utility Building</u>: The building used to house communication, mechanical and electrical systems, toll staff offices and amenities and for other infrastructure necessary to operate a toll plaza.
- <u>VISSIM</u>: A driver behavior-based simulation program that is used to simulate a wide variety of traffic operations, from urban arterials to freeway interchanges to complex toll facilities.
- <u>Weave</u>: A driving maneuver in which two or more traffic streams must cross the path of the other, i.e. the right hand lane traffic moves into the left hand lane and the left hand lane traffic moves into the right hand lane. An example is an on-ramp followed closely by an off ramp; the on-ramp traffic must cross the path of a mainline vehicle needing to exit mainline.

# APPENDIX J PUBLIC INVOLVEMENT, MEETING NOTES, QUESTIONS & ANSWERS

### **Public Involvement**

The following meetings have occurred to present information and gather input:

### A. Municipal Meetings

- 1. Town staff input and information sharing throughout
  - a) Annual Town Visit meetings December 16, 2004
  - b) Annual Town Visit meetings November 28, 2005
- 2. Town Managers' meetings
  - a) 1st meeting Sept. 26, 2006
  - b) 2nd meeting including Plaza site tour November 29, 2007
  - c) 3rd meeting January 22, 2008
  - d) 4th meeting February 15, 2008
- 3. Joint Select Board meeting October 25, 2006
- 4. Joint Select Board presentation January 23,2008

### B. Permitting Agency Meetings

- 1. State/Federal Interagency meeting October 10, 2006
- 2. State/Federal Interagency meeting November 10, 2009 (planned)

### C. Legislative Meetings

- 1. Legislative hearing on LD 534 April 13, 2007
- 2. Legislative Tour & Briefing August 9, 2007
- 3. Legislative Tour & Briefing August 10, 2007
- 4. Legislative Tour & Briefing September 21, 2007
- 5. Legislative Tour & Briefing December 10, 2007
- 6. LD534 presented to Transportation Committee April 3, 2008
- 7. Existing Site Evaluation presentation to Transportation Committee-July 7, 2009
- 8. Existing Site Evaluation presentation to York County Delegation- August 10, 2009

### D. Public Meetings

- 1. Public Informational meeting February 27, 2008
- 2. Public Informational meeting April 3, 2008
- 3. Meeting of York Selectman and MTA Board April 29, 2008
- 4. Meeting of York Citizens and MTA staff May 15, 2008
- 5. Meeting of York Selectmen and MTA Board June 16, 2009
- 6. Meeting of York Selectmen and MTA Board November 5, 2009 (planned)

### Meeting Notes, Questions & Answers

Following are notes from one York Citizen's meeting not already contained in the LD 534 Appendix as well as the formal responses to Questions posed by the York Board of Selectmen and the citizen's Think Again group.

- 1. York Citizen's meeting May 15, 2008
- 2. Public Informational Meeting Questions June 9, 2008
- 3. York Selectboard Questions August 26, 2009
- 4. Think Again Questions September 4, 2009
- 5. Think Again Questions October 26, 2009

### **MEETING NOTES**



5/15/2008

HNTB Project No. 09009-XW-005-011

York Citizens Meeting

### Location:

York Beach Fire Station (Upstairs)

### Purpose:

To inform the Citizen's group and York Town Select people that the MTA has decided to re-evaluate the existing site. Also to get input from the citizen's group and York Town Select people about their feelings on what within the process is important to them.

### Attending:

Members of York Town Select Board York Town Manager York Legislator Members of York Citizens Group Conrad Welzel - Maine Turnpike Authority Dan Paradee - Maine Turnpike Authority Bruce Pelletier - Maine Turnpike Authority Scott Warchol - Maine Turnpike Authority Roland Lavallee - HNTB Paul Godfrey - HNTB Kevin Slattery - HNTB Ray Hanf - HNTB

Conrad began the meeting by emphasizing the need for enhanced communications in the process. He asked that we start the meeting by introducing ourselves.

After the introduction had occurred, Conrad explained that the Maine Turnpike Authority Board has asked HNTB, chief consulting engineer, to step back and conduct a more in depth evaluation of the existing site of the York Toll Plaza. There has been an ongoing effort for environmental work at the alternate sites and that work will be wrapped up in the near future. Then work on the alternate sites will cease until the alternatives for the existing location have been fully evaluated and decisions has been made by the Board on the next steps for this project. Conrad stated that the purpose of the meeting was to share basic information and criteria and gather the committee's comments, concerns and input. Conrad then introduced Roland Lavallee.

Roland began by explaining the guidelines that are being used for the project. A Selectman asked "How can we be convinced that HNTB is truly investigating the existing site?"

Roland replied that we are not going to leave any stone unturned and that we want to work with you. Conrad added that the Board is convinced that the existing site needs to be fully evaluated and engage the committee as if this was step one of the process. Dan P. added that as we move this forward the first question the permitting agencies are going to ask is 'Why can't this work at the existing site.' So by stepping back we will ensure that this project will be able to move forward once it reaches the permitting stage.

Roland explained where the guidelines that HNTB is using on the project came from. Emphasis was put onto the fact that until recently Federal Highway Administration had not established standards/guidelines that applied to toll plazas previously. The reason for this was that tolling was not allowed on federally funded roads. As time progressed and legislation was passed federal highways were starting to be tolled with federal funds. This presented a need for some type of federally accepted guidelines, "The State of the Practice and Recommendation on Traffic Control Strategies at Toll Plazas." Roland explained the four criteria that were being used based on the State of the Practice.

The committee questioned whether the criteria being used should not apply to an existing site. They felt that there has to be a way to make the existing site work and still be safe.

Roland explained that the existing site does meet any of the criteria that help make a facility safe.

The committee feels that the York Toll Plaza really is not that unsafe.

Paul Godfrey explained that the existing site is a High Crash Location (HCL) on both bounds of the south side of the toll plaza. It ranks in the top ten in highest crash locations in the state of Maine. He also explained about how the crash statistics are calculated such that they can be compared and they how are used.

The committee requested that they have access to the newest data that HNTB has to confirm that the location is an HCL on both bounds because they thought that it was only on one bound.

Roland explained the different options that have been looked at with respect to the existing site.

• First looked at Option 2 from LD534 (Infrastructure Upgrade Only). He talked about the sight distance issues that exist at York and a committee member compared them to the Hampton Toll Plaza. The comparison with Hampton continued citing the issues because of the two bridges that obstruct the sightline. It was pointed out that Option 2 fixes the infrastructure only, lacks the safety improvements (i.e the sight distance, the bottom of a hill, separation from an interchange and on a straight stretch.

Roland brought out the roll plans of the sketches at the existing site that meet the engineering criteria. He explained to the group that these images are very rough sketches and meant to show the extremes that are necessary to build a plaza at the existing site that meets the engineering criteria. He showed the group both options that meet the engineering criteria at the existing site, then he showed them Option 4 from LD 534.

Committee asked "Why are we relying on line of sight? Why not some form of video imagery that would inform the driver that there is a plaza ahead and make a decision of what lane to get into?"

Roland said that HNTB would investigate this video imagery. The critical point in toll plazas is the decision point, and this is what is critical to accidents.

Roland went on to explaining the operational difficulties of HST at the existing site (Option 4 from LD534). With this option the decision point is much further away than it is now, and can't see the reason why you are making that decision. If you wanted to get off at the Exit 7 interchange then you would have to make that decision without even being able to see the interchange. Generally, a typical interchange design is such that you can see an exit before you have to make that decision.

Committee wanted to know if safety is such a concern then why not remove the toll plaza completely and implement video tolling? The emphasized that other agencies are attempting to changes to this type of tolling even though their rates are not as high as 80-90% commuters.

Dan P - Video tolling is meant for facilities that are highly commuter traffic facilities. Facilities like Toronto or Atlanta have a high commuter base, but they also have local jurisdiction on their side to enforce the non-payers. The MTA would have to send bill to all the out-of-staters that do not have an electronic tag for a \$1.75 toll. Enforcement for those people is not possible based on the current law structure. Cashless tolling is a great technology, but this is purely a business decision for the betterment of the MTA and the state of Maine.

What research has been done on side tolling? NJ has a ticket system, why not that?

Paul G. - Every significant toll facility in America has plazas at the beginning and at the end.

Removing the barrier plaza at York can not be done; there would be a gross inequity in the system. A ticket system is very inefficient and will cause more queuing than today because of slower processing rates.

The committee felt that the MTA and HNTB did a poor job of informing the public during the LD process. They felt that the MTA and HNTB did not do what was required to meet the requirements of LD534. They were never given the whole story of homes being impacted.

Conrad W. - The MTA's understanding of LD534 requirements was to hold meetings like the Select board meetings. MTA did not feel that having a full committee meeting was necessary to fulfill the requirements of LD534

The committee requested that the MTA supply them with the notes from today's meeting and keep Rob Yandow informed throughout the process.

The MTA and HNTB will supply the town with the meeting notes as requested and an open dialogue will be kept between the MTA and Rob Yandow to keep the committee informed.

The goal is to have by June 19<sup>th</sup>, a report that discusses the existing site and presents that information the MTA board. A wide range of alternatives will be evaluated in this report. MTA and HNTB will inform the committee of MTA meetings with respect to this project.

The meeting was closed by Conrad W. thanking everyone for coming and wanted them to feel that the MTA and HNTB would keep them informed throughout the process.

This is our understanding of items discussed and decisions reached. Please contact us if there are changes or additions.

Submitted by,

**HNTB CORPORATION** 

Ray Hanf, E.I Staff Engineer

cc: Roland Lavallee, Paul Godfrey

Encl (1)



# Responses to Questions MTA public meeting on the replacement of the York Toll Plaza York Middle School April 3, 2008

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- 1. Introduction
- 2. Purpose of MTA & Accountability
- 3. Purpose of Toll Collection and York Plaza
- 4. York Plaza Conditions and Concerns (Deficiencies)
- 5. Feasibility Study & Proposed Facility
- 6. What Would it Take to Build at the Existing Location?
- 7. Site Identification & Screening Process
- 8. Environmental Considerations
- 9. Right-of-Way Considerations

### 1) Introduction

On April 3, 2008, the Maine Turnpike Authority staff held a well attended public meeting at the York Middle School in York Maine to update residents and receive comments and questions regarding an ongoing study about the replacement of the York Toll Plaza. Recognizing that such a large forum does not always provide an opportunity to answer all questions adequately, MTA staff recorded questions with the intent of providing written answers. This document contains those answers.

It is important to note that the Turnpike Authority, at the urging of the York Board of Selectman and in response to concerns raised by local citizens, has significantly adjusted the process and schedule of this study since the April 3, meeting. Most notably, the Turnpike Authority has agreed to commission a more in-depth study of the feasibility of reconstructing the toll plaza at the existing location. These adjustments in process and schedule had to be accurately reflected in the answers contained in this document and thus prolonged its completion.

This is not intended to be the conclusive response to all local questions and concerns, but is rather just another step in the process to enhance the dialogue on this important and challenging issue.

### 2) Purpose of MTA & Accountability

1. Why does the Turnpike Authority still exist and collect tolls?

Response: The Maine Turnpike Authority was established by the Maine Legislature in 1941 to function as an independent agency of government with the power to issue revenue bonds and collect tolls for the purpose of building, maintaining and operating an express highway. As an independent agency, the Turnpike was created to carry its own debt and credit rating, completely separate from the state's debt and credit rating.

At the time, it was generally understood that once the debt for the construction of the Turnpike was paid off, the tolls would be removed and the cost of maintaining the Turnpike would be paid for, like other state highways, through the gas tax and various other taxes. However, when the issue came before the Legislature in the early 1980's, legislators were confronted with several financial realities.

- In order to maintain and operate the Turnpike, the Legislature would have had to significantly raise the gas tax or redirect funding from other transportation projects around the state.
- In 1982, The Turnpike was nearly 35 years old and experiencing significant traffic growth. The Legislature recognized that substantial investments to rehabilitate the original infrastructure would be required in the foreseeable future.
- The Legislature foresaw the need for major capital improvements on the Turnpike including the construction of new interchanges and the eventual widening of the southern section of the Turnpike. They understood that these projects would require substantial investments that might not be possible without continued toll revenue.
- The Legislature understood that eliminating tolls and relying instead on the gas tax to maintain the Turnpike, would significantly increase the cost burden on Maine residents, while decreasing the burden on out-of-state users. Out of state drivers contributed only 20% of the gas revenues collected in the state, but they contributed up to 50% of the tolls collected.

For these and various other reasons the Maine Legislature voted in 1982 to continue the Maine Turnpike Authority and the collection of tolls. The tolls are used to fund operations and maintenance as well as to pay debt service on the existing bonds.

#### 2. To whom is the Maine Turnpike accountable?

Response: The Turnpike Authority was created by an act of the Maine Legislature. Its annual operating budget and any adjustments to the borrowing cap must be approved by the Maine Legislature.

Six members of the Maine Turnpike Authority Board of Directors are appointed by the Governor and confirmed by the Maine Senate. The seventh member is exofficio and is the Commissioner of Transportation or his/her designee. The Governor's appointees must be selected to provide representation from the counties along the Turnpike corridor, including York, Cumberland, Androscoggin and Kennebec.

The Turnpike Authority is also accountable to its bondholders. Bondholders are represented by bond counsel to assure that the Maine Turnpike is properly maintained and managed. The Maine Turnpike is one of only six toll agencies in the country that has earned AA credit ratings from all three of rating agencies: Standard & Poors, Fitch and Moody's. The Maine Turnpike is also required to comply with applicable Maine Department of Environmental Protection and United States Army Corps of Engineers environmental permits.

### 3) Purpose of Toll Collection and York Plaza

- 1. Why doesn't the MTA spend more money on encouraging E-ZPass vs. cash? Response: The Maine Turnpike Authority conducts E-ZPass promotional campaigns, employing television advertising, newspaper advertising and direct mail. The most recent effort, which took place in November of 2007 consisted of an extensive 42,000 piece mailing to all residents of 13 towns in southern York County that were not identified as E-ZPass customers. The direct mail effort was supported by a three week large space display advertising campaign in newspapers serving the southern York County area. The total cost of the promotional program was \$41,534.00. The MTA will continue to pursue creative, targeted and cost-effective marketing strategies
- 2. Why are tolls collected from school buses?

  Response The MTA is required by its bond resolution to collect tolls from all vehicles in an equitable manner to pay for the maintenance and operation of the roadway.
- 3. Why does the MTA want to build a new toll plaza?

  Response The new toll plaza project is being contemplated because of the identification of deficiencies and safety concerns with the existing plaza as documented in the LD534 Response Report. The current plaza has outlived its life expectancy through a series of retrofits, not the least of which was expanding the plaza from 11 lanes to 17 lanes. Current data supports the construction of a new facility as the most prudent expenditure of funds.
- 4. Why doesn't the MTA remove the York Toll?

Response: The ideal way to distribute tolls fairly and equitably to the patrons traveling on toll highways, such as the Maine Turnpike, is with strategically placed toll plazas. Well placed toll plazas work to maximize equity and balance toll rates in all types of toll systems. The critical element is that the toll plazas bookend the toll road itself. All major toll roads of significant distance in this region of the United States have a mainline toll plaza located at both ends. This includes the Maine Turnpike, Massachusetts Turnpike, New Jersey Turnpike, Garden State Parkway, and Pennsylvania Turnpike.

Removal of the York Toll plaza without other significant toll system changes will exacerbate toll rates and toll equity. For example, out-of- state patrons entering from the south will be able to travel to Gray without paying a toll. In order to make up this lost revenue, toll rates at the remaining mainline and interchange toll plazas will have to go up significantly, or other toll system infrastructure will need to be added (see response below). Significant toll rate increases at interchange and northern mainline toll plazas will primarily affect Maine residents and will likely result in diversion to local roads as patrons choose not to utilize the Maine Turnpike for short to moderate distance trips. In conclusion, the York Toll Plaza plays a big part in allowing the Maine Turnpike Authority to effectively and equitably distribute tolls to all patrons, including the large amount of patrons that come from out-of-state.

5. Why doesn't the MTA remove York Toll and collect the toll revenue at all other toll locations?

Response: Without a southern mainline plaza, the only way to collect cash tolls from vehicles entering the Turnpike from the south would be to reconstruct exiting toll booths at every plaza from Wells to Gray. This would roll back the significant operational gains made ten years ago when the Turnpike Authority converted to a faster, more efficient and cost-effective system of toll collection.

In 1997, the Maine Turnpike converted from a toll ticket system to a new system of fixed fares and electronic toll collection. The changes were driven by a pressing need to handle ever-increasing traffic volumes more efficiently and to reduce the rising operational cost of collecting tolls.

Under the fixed fare system, all cash paying customers of the same vehicle class pay the same amount when entering the Turnpike and exit the Turnpike at most interchanges without stopping to pay a toll. By collecting the same fixed fare cash amount from every customer upon entry, the system eliminated time consuming fare calculations and dramatically sped up toll collection. More importantly, the system eliminated the need for customers to stop and pay a toll when exiting at Turnpike interchanges. Because exiting toll booths were no longer necessary, many were converted to additional entering lanes, increasing the thru-put capacity at each plaza and preventing the need for costly and environmentally impactful toll plaza expansions. In its first year of operation, the new system eliminated more than 25 million vehicle stops, which in turn reduced congestion, gas consumption, air pollution and turnpike operating costs. The reintroduction of exiting tolls to collect revenue lost by the elimination of the York toll plaza would result in millions of unnecessary vehicle stops and would increase congestion, air pollution and gas consumption.

6. Why doesn't the MTA remove the York Toll, keep the toll free exits, and simply replace the lost revenue by increasing entry tolls at every other location? Response: If the southern toll plaza is eliminated and exit tolls are not reintroduced, we estimate that entry tolls at all locations would have to be increased by \$0.90 to make up for the lost revenue. This would result in extreme

toll inequity for Turnpike users. For example, under such a system motorist entering the Turnpike in York could travel more than 50 miles to Gray without paying a toll. A motorist traveling 31miles from Wells to Gray would pay \$1.50 (\$0.90 + 0.60). A motorists traveling just 1 mile from Exit 47 to Exit 48 in Portland would also pay a toll of \$1.50. The toll rates for the New Gloucester and West Gardiner mainline toll plazas would also need to increase to \$1.75. This proposal would create extreme toll rate inequities and would significantly shift toll burden currently paid by out-of-state users onto Maine resident users.

- 7. Why can't we remove the York Toll and make up the lost revenues by increasing tolls incrementally from south to north? For example, charge 60 cents at Wells, 75 cents at Kennebunk, \$1.00 at Biddeford and so on.

  Response: This proposal would create even greater toll rate inequities by allowing motorists who enter from Exit 7 or further south to travel for free up to Exit 63, while charging excessively high tolls for motorists making short trips between exits in the Biddeford Saco area and the greater Portland area. This would also shift more of the toll burden from out-of-state users to Maine resident users.
- 8. Can One-Way Tolling be applied at the York Toll Plaza? Response – One-way tolling is a method of toll collection that involves charging twice the fare in one direction, while allowing toll free travel in the other direction. The Maine Turnpike Authority conducted a feasibility study of one-way tolling in 2005. The feasibility study took place at the same time and benefited from the experience of a two-year, one-way tolling demonstration project at the Hampton Toll Plaza on the New Hampshire Turnpike. Based on the findings of the feasibility study and the experience of Hampton Toll Plaza demonstration project, the Maine Turnpike Authority determined that oneway-tolling was not a viable tolling strategy for Maine. The Authority's decision was largely due to concerns about the number of vehicles that would divert onto local roadways to avoid the double-tolled direction. The study estimated that an average of 11.7% of the vehicles would divert around the toll plaza to avoid the doubled toll. Note that one-way tolling was not resumed at the Hampton Toll Plaza following the demonstration project for the same reason. A closer look at one-way tolling suggests that it is only successful on bridges, tunnels and in rare instances on highways, where there is little opportunity to divert around the facility to avoid the toll. The only successful examples of oneway tolling in our region of the country are on bridges and tunnels in urban areas, such as the Tobin Bridge in Boston, Tapanzee Bridge in New York and the Benjamin Franklin Bridge in Philadelphia. It is successful on these facilities because it is virtually impossible to divert around them and reach your destination in a reasonable amount of time. This is not the case on the Maine Turnpike and other more rural toll highways, where the opportunity for diversion exists. A doubled toll in one direction at the York Toll Plaza would likely result in

an unacceptable level of diversion onto Rt. 1 and other alternative routes.

9. Why doesn't the Maine Turnpike adopt cashless tolling?
Response: Cashless tolling may become a universally viable technology someday in the future, but not the identifiable future, particularly on a highway like the Maine Turnpike, which serves such a diverse mix of users.

The most common application of cashless tolling is a system in which a very high percentage of a highway's users have an electronic toll collection device (E-ZPass) in their vehicle and pay their tolls accordingly. Tolls are collected from the small percentage of motorists who do not have electronic toll collection by capturing a video image of their vehicle's license plate and sending the registered owner a bill.

Successful examples of cashless tolling involve highways in urban areas that serve primarily as commuter routes and have a very high rate of electronic toll collection usage, generally exceeding 80%. In addition, the vast majority of their users typically reside within the same jurisdiction or use the same electronic toll system operator, making it possible to conduct a billing and enforcement program for motorists without electronic toll collection.

The Maine Turnpike shares none of the characteristics that are essential for a successful cashless tolling program. The Maine Turnpike is primarily a rural highway. It is not a commuter-oriented highway. Most Maine Turnpike drivers are occasional users and a high percentage of them are from out-of-state. Nearly 50% of the users of the York Toll Plaza are from out-of-state.

While E-ZPass usage on the Maine Turnpike is nearing 50% and continues to grow, there is no expectation, given the highway's diverse user base, that the rate will reach the 80% -90% range in the near future. That means that the Authority would be required to collect a significant portion of its revenue by capturing video images of license plates and sending a bill to the vehicle's owner. Because the Maine Turnpike serves so many occasional users, the cost of processing and sending a bill could exceed the toll amount to be collected. There is no universal, reliable system in place that would allow the Authority to access the names and addresses of out-of-state drivers for billing purposes, and certainly no system to enforce penalties for unpaid video tolls.

Will the Turnpike's E-ZPass technology soon become obsolete?

Response: Like any technology, electronic toll collection is always evolving, but there is no indication that the current system will become obsolete in the foreseeable future. The Maine Turnpike Authority is an active, voting member of the E-ZPass Interagency Group (IAG), which is comprised of 24 agencies, operating in 13 states that provide compatible E-ZPass technology to their customers. Together, the IAG agencies have issued more than 17 million active E-ZPass tags. Given the significant commitment by the Maine Turnpike and all other IAG member agencies to create and maintain a system that is compatible

from state to state, it is highly unlikely that any sudden technology changes would be adopted by the IAG that would render the systems of member agencies obsolete.

### 4) York Plaza Conditions and Concerns (Deficiencies)

1. What are the traffic delays at York Toll Plaza? What impact has E-ZPass had on the delays?

Response: E-ZPass has had a positive influence on delays and backups at the York Toll Plaza. One of the more notable factors in this has been the shift in cash paying customers to the E-ZPass system. For the existing arrangement and number of lanes, on average, dedicated E-ZPass lanes can process approximately three times as many vehicles as a cash lane. Following is some of the more recent delay and backup data.

- In 2005 northbound backups averaged 1157' with 173 seconds of delay for cash customers. By comparison E-ZPass customers averaged 120 seconds of delay.
- In 2005 southbound backups averaged 4335' with 442 seconds of delay for cash customers. By comparison E-ZPass customers averaged 375 seconds of delay.

Experience indicates that, as cash-payers shift into the E-ZPass program, toll plaza backups and delays diminish. However, given the mix of users that include cash-paying patrons and E-ZPass patrons, we will continue to encounter situations in which cash backups block access to the dedicated E-ZPass lanes exacerbating backups and delays significantly. This diminishes the potential benefit of the growth in E-ZPass usage. The solution to this circumstance is the safe separation of the cash paying patrons from the E-Z Pass patrons.

- 2. If the York Toll Plaza has safety problems, how can the MTA still operate it? Response: All highways and toll plazas have safety challenges. It is the responsibility of the operator to minimize those safety challenges. Over the years the MTA has invested a significant amount of money to upgrade and repair the existing plaza to minimize crashes and traffic flow problems that often result in crashes. But these upgrades and repairs are not able to address the plaza's more fundamental safety problems of being located near an interchange, on a curve and at the bottom of a hill. These fundamental problems will only cause the plaza to become more unsafe as traffic volumes increase. The toll plaza study is being conducted to ensure the future, long-term safe operation of the plaza.
- 3. Why is the speed limit for the E-ZPass lane 35 mph at the Hampton Toll Plaza in New Hampshire, and 10mph at York?

Response: The approach to both York and the Hampton Plazas is signed at 35mph. The speed limit immediately before and after both plazas is 10mph for E-ZPass customers.

4. Why are the E-ZPass lanes on the right side?

Response: When the MTA introduced electronic toll collection (ETC) in 1997, the dedicated ETC lanes were located on the left of the plaza for approaching traffic. This configuration seemed to make sense because it allowed ETC users to travel straight through the plaza. The MTA, however, received complaints from residents of nearby communities saying that the ETC lanes were often blocked by tourists who seem to congregate near the middle of the plaza. The middle lane also made it difficult to access the interchange. The MTA held focus groups with local residents, which concluded that the ETC lanes should be placed on the far right side, allowing users to go around the backups in the middle of the plaza and access the York interchange easier. The MTA responded by moving ETC lanes to the far right. In 2005, the MTA added back ETC lanes on the left side of the plaza, so now there are dedicated ETC lanes on both the left and right side of the plaza. It should also be noted that all toll lanes will accept E-ZPass.

### 5) Feasibility Study & Proposed Facility

- 1. How will the plaza be plowed and kept safe during a snowstorm?

  Response: The MTA maintenance crews will plow this plaza much the same way the mainline is plowed and maintained. With the presence of median barriers and barriers separating cash from E-ZPass patrons, the plowing will consist of a number of one-way loops with typical snow removal procedures in certain areas.
- 2. How will the toll plaza be designed so that it will be visually pleasing?

  Response: The conceptual design for a new plaza is in the very preliminary stages with only a few initial thoughts; the toll plaza should be in keeping with southern Maine and be a subtle but welcoming 'gateway' to Maine. The new plaza will replace the existing substandard, rusted, antiquated, and bumpy plaza that more than 17 million people experience each year as they enter and depart Maine.
- 3. Why is the proposed toll plaza being designed to accommodate large volumes of traffic when bottlenecks occur downstream at the Hampton Toll Plaza in NH?

  Response: The MTA has a responsibility to its customers and to the State of Maine to operate as safely and efficiently as possible. While it is important for agencies in neighboring states to communicate and cooperate, MTA standards of safety and operation should not be determined by the standards of other highways or facilities.
- 4. Why is the plaza currently designed with a total of 21 lanes? If Highway Speed Tolling efficiently and quickly processes vehicles, why are there more lanes than the existing 17 lane plaza?

Response: The MTA is still in the early stage of design development. Initial designs called for 21 lanes consisting of seven northbound and eight southbound cash lanes with three highway speed tolling lanes in each direction. This is a reasonable preliminary estimate of the number of lanes required based on current

traffic projections, E-ZPass usage, toll collection processing rates and acceptable vehicle backups. As part of the MTA's ongoing avoidance and minimization (of impacts) process, traffic modeling parameters are being refined and updated to reduce the number of lanes while providing a safe plaza and reasonable level of service.

- 5. What factors into the width and length of the proposed toll plaza?

  Response: The width of the plaza footprint is a function of the number of lanes and necessary support buildings. See the question above for discussion on the number of lanes. The length of plaza footprint is based on a design that allows for: 1.) E-ZPass and cash paying vehicles to safely diverge and merge, 2.) cash paying vehicles to slow down and choose a cash lane, 3.) an appropriate distance for vehicles to queue, and 4.) for the cash paying vehicles to accelerate and merge into one lane before merging with the E-ZPass vehicles.
- 6. How can traffic safely merge at 65 mph after paying tolls?

  Response: Cash customers will exit and enter the mainline using an off-ramp and on-ramp that meet all of the standard guidelines of a typical interstate interchange at 65 mph posted speed.
- 7. How does the crash rate on the Maine Turnpike compare to National rate? If the Turnpike is much lower, why is there a need to lower the crash rate? Response: The standard of comparing crash rate statistics in Maine is not against National values but instead against statewide values. Crash rate data was requested of the MaineDOT for the three year periods of 2003-2005 and 2004-2006. This data shows that the roadway immediately south of the York Toll plaza for both the Northbound approach and the Southbound departure are high crash locations; in fact the Northbound approach has the #11th highest crash rate out of 1,054 high crash locations within the State of Maine.
- 8. Can the accident data for the High Crash Locations be provided?

  Response: Yes. Data for High Crash Locations as well as all crash data for the Turnpike is available from the MaineDOT for any interested party. The MTA has also provided this information to the Town of York. In summary, both the northbound and southbound lanes on the south side of the York Toll Plaza are rated to be High Crash Locations by the MaineDOT. The northbound lanes on the southside of the plaza are ranked as the 11<sup>th</sup> highest crash location of 1,054 high crash locations in the state.
- 9. What consideration has there been for access to the plaza for fire and police? Response: Access for emergency vehicles has been discussed in general terms with town officials. This type of access is always a part of the design process for all plazas and service buildings. From these early discussions, we have the required level of information necessary for conceptual planning and will work with local fire, police and emergency management to acquire more detailed information as the project moves into preliminary and final design

10. If funding is so critical for the Turnpike, is constructing a new toll plaza more imperative than repairing bridges and other infrastructure?

Response: The roadways, bridges, interchanges, toll plazas, service areas and maintenance areas are subjected to increasing stress due to age, growing levels of traffic and the demands of the harsh northern New England climate. To ensure the sound condition and effective operation of the Turnpike, the Authority's 20 year plan funds and implements proactive Operation and Maintenance, Reserve Maintenance and Capital Improvement programs. The vigilance of the Authority through these programs has resulted in a well-maintained and efficiently-operated Turnpike. As the Authority looks to future initiatives, such as the reconstruction of the mainline toll plaza in York, it will continue to assure that turnpike facilities meet current safety standards as well as projected demands. Given that the York Toll Plaza handles more than 16 million vehicles per year and generates 40% of the revenue necessary to maintain the MTA's overall infrastructure, its safe and efficient operation is no less important than any bridge or section of roadway.

### 6) What Would it Take to Build at the Existing Location?

1. Can the York plaza be reconstructed at the existing site?

Response: At the urging of the York Selectman, the Turnpike Authority has directed its consulting engineer to conduct a more in-depth study about the possibility of constructing a new plaza at the existing location. Prior to this the MTA commissioned feasibility study that considered three different alternatives at the existing site in addition to the no-build alternative. The study concluded that each of the alternatives failed to achieve the basic safety and efficiency objectives originally intended by the toll plaza improvement project, and failed to meet the basic design guidelines established by the Federal Highway Administration for safe toll plaza design and operation. The study also indicated that the cost of building at the existing site would be similar to the cost of building at a new site that would achieve the project objectives and meet federal guidelines for toll plaza safety.

The following are operational issues identified as unresolved at the existing location alternative that affect both capacity and the safety of patrons and staff:

- A. Safety concerns remain due to proximity of Chases Pond Road interchange. Confusing traffic patterns will result with access to the on and off ramps occurring within the cash lanes of toll plaza area.
- B. The plaza will remain at the low point of a hill which is not recommended. This creates a safety concern due to the potential of heavy vehicles losing their brakes and striking the plaza or stopped traffic. In addition the hill leads to heavy engine braking noise southbound and heavy acceleration noise northbound as commercial vehicles approach and depart the plaza.

- C. Sight distance will not improve, in fact from both north and south approaches it will get worse due to cash lanes being moved further from the center of the mainline. Sight distance is compromised by the close location of Chases Pond Road Bridge and horizontal curve of the mainline approach. Improper sight distance, leads to inefficient decisions and unsafe last second lane changes.
- D. Wetland and other environmental impacts will be significant and obtaining permits will be more difficult. The mitigation of these impacts, even if allowed, would add \$3-10 million to the 'similar' project costs resulting in a project cost exceeding a new location.
- 2. What is the value of the wetlands around the existing plaza? When comparing sites, is the quality of the wetland considered?

Response: Wetland type, area, quality and function are considered when screening sites. Wetlands adjacent to the existing toll plaza are substantive and associated with the Little River. While some of those nearby wetlands have experienced impacts attributable to nearby facilities (such as the toll plaza), the effects are limited to the immediate proximity. The wetland is extensive, diverse, and one of the larger contiguous wetlands in the study area. Similarly, wetlands adjacent to other development or roadways may also have experienced degradation or changes to the functions, which is also considered.

- 3. How much has the ground at the toll plaza settled?

  Response: From available information, pavement in the immediate plaza area has settled as much as 4.5 feet.
- 4. With proper engineering, can the settlement of the existing site be remedied? Response: Yes, the existing site could be engineered to minimize the effects of differential settlement, though at a substantial cost. Soil settlement is only one of the operational and safety concerns at the plaza.

### 7) Site Identification and Screening Process

1. Why does the MTA consider the York Plaza project in the early stages of the project development process when the LD534 Report was delivered as Final to the legislature's Transportation Committee?

Response: There has been much confusion about the relationship between a study report which was completed to meet the specific requirements of a law passed by the Maine Legislature (LD 534) and the Turnpike Authority's broader study regarding the reconstruction and possible relocation of the southern toll plaza, which is still ongoing.

In LD 534, the Legislature required the Turnpike Authority to document the need for the replacement of the southern toll plaza as well as the reasons why the existing location may not be suitable for this replacement project. The parameters of this study and report were clearly defined by the Legislature and did not

include any discussion of alternative sites. The MTA completed the report and presented it to the Legislature's Joint Standing Committee on Transportation, as required by the law. The MTA has since received correspondence from the House and Senate Chairmen of the Transportation Committee confirming that the MTA has completed and complied with the requirements of LD 534.

The MTA's study regarding the replacement and possible relocation of the southern toll plaza is a separate and much more extensive undertaking including items reported in the LD 534 Response Report. The purpose of the study is to inform the Turnpike Authority Board of the deficiencies of the existing plaza and to recommend strategies to address those deficiencies and to make operational improvements that will allow the facility to function safely and efficiently in the future. It will present the Board with a range of options from rehabilitating the plaza, to modifying the plaza in conjunction with adjacent mainline reconstruction (to meet current design criteria), to building a new plaza at an alternate site. Benefits, impacts and costs will be included in the report for comparison purposes. This study was and is still in the early stages. The MTA Board: 1) has not received the study report, 2) has not made any decisions about the feasibility of replacing the plaza in the current location, 3) has not yet considered any alternative locations, and 4) has not filed for any environmental permits.

• Once the Turnpike Board makes a decision, the regulatory agencies such as the Maine Department of Environmental Protection and the U.S. Army Corps of Engineers will review all the data and will make their own determination if permits for a project are feasible.

#### 2. Was the public involved in LD534?

Response: LD534 required that the MTA should "hold informational sessions with interested parties." The MTA staff sought guidance on this requirement from the Chairs of the Legislature's Transportation Committee. They confirmed that a public meeting with selectmen from York, Ogunquit and Wells televised on local access cable would satisfy the intent of the law. (The MTA also held a number of other meetings as contained in the following response) The MTA arranged and participated in that meeting on January 23, 2008. The MTA reported back to the Legislature's Transportation Committee at a public meeting on April 3, 2008. Again, it is important to note that LD534 was specifically focused on the technical information regarding the deficiencies of the York Toll Plaza. It did not include any discussion of alternate sites, environmental impacts, community impacts or other issues that have since generated public interest.

#### 3. What public meetings have been held to date?

Response: It is important to understand that while the subject of replacing the York toll plaza has been discussed with local officials and at public meetings for several years, specific information about potential alternate sites and their potential community and environmental impacts was not available until recently. The MTA has provided information as it has become available during the course

of the study. The following meetings have occurred to present information and gather input:

### A. Municipal Meetings

- 1. Town staff input and information sharing throughout
  - a) Annual Town Visit meetings December 16, 2004
  - b) Annual Town Visit meetings November 28, 2005
- 2. Town Managers' meetings
  - a) 1st meeting Sept. 26, 2006
  - b) 2nd meeting including Plaza site tour November 29, 2007
  - c) 3rd meeting January 22, 2008
  - d) 4th meeting February 15, 2008
- 3. Joint Select Board meeting October 25, 2006
- 4. Joint Select Board presentation January 23,2008

### B. Permitting Agency Meetings

1. State/Federal Interagency meeting – October 10, 2006

### C. Legislative Meetings

- 1. Legislative hearing on LD 534 April 13, 2007
- 2. Legislative Tour & Briefing August 9, 2007
- 3. Legislative Tour & Briefing August 10, 2007
- 4. Legislative Tour & Briefing September 21, 2007
- 5. Legislative Tour & Briefing December 10, 2007
- 6. LD534 presented to Transportation Committee April 3, 2008

#### D. Public Meetings

- 1. Public Informational meeting February 27, 2008
- 2. Public Informational meeting April 3, 2008
- 3. Meeting of York Selectman and MTA Board April 29, 2008
- 4. Meeting of York Citizens and MTA staff May 15, 2008

### 4. Why weren't the LD534 Options compared to the Site Identification and Screening Alternatives?

Response: The LD534 Response Report details the investigation and findings related to possibilities of addressing specific deficiencies and safety issues at the existing plaza. A range of the upgrade and modification options were developed for the existing toll plaza that address some of these deficiencies. (It became apparent that looking at a generic relocation alternative may also be necessary.) The Site Identification and Screening Report details the investigation and location of possible sites along the Maine Turnpike corridor that hold potential for meeting basic design guidelines for the construction of a mainline toll plaza as well as addressing the identified deficiencies and safety issues. The options dealing with the existing site can not fairly be compared to the alternative locations for the simple fact that the existing site options do not meet the basic engineering design guidelines for mainline toll plazas currently in use today. Even though the existing site options are shown with associated costs, these numbers do not tell the whole story, e.g. simply replacing the toll booths, canopy and tunnel does not address traveler safety, congestion, or staff safety.

5. Why aren't the results of the LD534 and Site Identification and Screening Reports combined?

Response: The LD report was prepared at the request of the Legislature to address specific questions of the Legislature. The Site Identification and Screening report is being prepared for submission to the Army Corps of Engineers for the purpose of obtaining a LEDPA (Least Environmentally Damaging Practicable Alternative). The report documents the entire site location process, which is consistent with good transportation planning practices as well as federal and state environmental laws. Elements of the LD report, such as documenting project purpose and need and evaluating the existing facility location, are also elements required by federal and state environmental laws. In summary, the Maine Department of Environmental Protection and the Army Corps of Engineers will review both the feasibility of the existing location as well as alternate locations.

- 6. The Site Identification and Screening Report began with 16 sites and narrowed the candidates to four. What criteria were considered to eliminate the 12 sites?

  Response: The 12 sites were not carried forward due to their high levels of impacts including one or more of the following reasons: residential impacts or proximity to higher density development, wetland or natural resource impacts, impacts to tidal wetlands, and/or refined engineering screening.
- 7. How can a design be shown if a site is not yet selected?

  Response: Conceptual site designs were developed to compare multiple locations and to assess relative impacts between alternatives. This is a standard planning/engineering method. Additional site refinement, design and consideration of public input will need to be applied to the four alternative sites to develop even more site-specific information for use when screening the sites.
- 8. When comparing the four alternative sites, how is the criteria weighted in the comparison matrix? What consideration is given to homes?

  Response: The environmental permitting agencies do not provide a specified weight or factor for comparing dissimilar resources (homes, wetlands, etc.).

  Resources and potential impacts are quantified and compared or ranked within each resource and compared on whole. Generally, residences and wetlands are the most prevalent consideration in screening sites.
- 9. How are people represented in the comparison matrix of the four alternative sites? Response: People are represented in the homes/residences categories including densities of homes, proximity of homes, land-use type and the inclusion of proposed developments.
- 10. What is the cost comparison of reconstructing the existing plaza vs. a new site? Response: It is important to note here that a comparison of cost alone does not tell a complete story. First and foremost is that an alternative that does not meet basic goals, purpose and/or design guidelines can not fairly be compared to an

alternative that does meet all of these criteria. As well, at the current stage of development there are a number of items that are not accounted for either completely or partially, e.g. wetland impacts and the mitigation ratio they must be replaced at, soil engineering and the extent to which advanced construction methods might need to be applied. With that said, reconstruction of the existing plaza, while not addressing all safety or operational issues, and not meeting the basic engineering design criteria could cost \$37 million dollars plus an additional \$10 million dollars worth of wetland mitigation costs (estimated 26 acres impacted) plus upwards to \$15 million dollars for advance soil construction. Still, the estimate for the existing site alternatives does not include potential costs of reconfiguring the Chases Pond Road interchange or its complete relocation to meet some of the basic design guidelines; which could also add millions to the cost, pushing the total cost to over \$70 million dollars. A new plaza alternative in a new location could cost \$36-38 million with an additional \$0.5 to \$4 million in wetland mitigation costs (estimated 1-11 acres impacted). A new plaza would be located such that other unknown costs are minimized and/or avoided, e.g. soils, interchanges, roadways, etc. Based on location selection criteria a new location would meet all the basic design criteria as well as address deficiencies and issues currently plaguing the existing plaza. Therefore a new plaza in a new location may cost up to \$40 million dollars. To reiterate, costs of reconstructing at the existing site vs. building a new plaza at an alternative site are not the only factors for comparing options. Reconstructing the existing plaza leaves many deficiencies unresolved including safety concerns that are a leading factor in the Plaza being identified as a High Crash Location.

- 11. When selecting a site, are cemeteries considered? There is at least one near MM11.3. *Response: Yes, cemeteries are considered a significant constraint.*
- 12. When selecting a site, are vernal pools considered? There are many surrounding all of the alternative sites.

Response: Yes, vernal pools are considered in the evaluation. An initial site inspection was conducted to identify vernal pools and significant wildlife habitat within potential project footprints and within a 500 foot buffer area from the footprint.

13. How are wetland impacts estimated?

Response: Wetland areas were identified for all candidate sites in the same manner using aerial photographs, Natural Resources Conservation Service Soil Survey mapping of hydric soils, National Wetland Inventory mapping of wetlands, and USGS topographic maps. The wetland information for alternative sites is equivalent and only used to make comparisons between initial alternatives (Phase 1) for screening. Subsequent information will be added to refine wetland boundaries to compare the Phase 2 alternatives. Once the preferred site is selected, formal wetland delineations will be conducted to determine exact wetland boundaries, locations surveyed, and permit applications will be prepared

using refined site design and field-delineated wetlands. Other information such as functional assessments and ecological resources will be included.

14. Are wildlife sanctuaries reviewed and considered?

Response: Yes. If land in the Wildlife Sanctuary was identified as a special wildlife habitat or critical habitat area, then that area would be considered in the screening analyses. If the Wildlife Sanctuary is not designated as special or critical habitat, no special consideration is made

15. Will any roads be relocated? Who would pay for this?

Response: At this stage of planning, the MTA does not anticipate the relocation of any local road. As the project enters into design, there may be a need to address some existing roadside ditches and grading. The MTA would incur the costs for such work to any public road if the work is necessitated by MTA construction.

16. Will security for the York Water District Treatment Plant be compromised if the selected site puts the plaza in close proximity?

Response: The treatment plant and Chases Pond are not currently fenced from nearby properties, but the Turnpike right-of-way is fenced. A fence will be installed along the right-of-way between the toll plaza and all abutters. Sites at Mile Markers 8.7 and 9.9 are the closest to the treatment plant, and based upon the conceptual design, it is unlikely that any additional tree clearing between the Turnpike and the treatment plant will be needed.

17. If the water line is required to be relocated, who will pay for it?

Response: This is a legal question that would depend in part on the nature of the York Water District's property rights in the property through which the line runs. The MTA would work with the York Water District to determine these rights and responsibilities accordingly.

18. How much on-site investigation has there been?

Response: To date, staff, engineers, planners, surveyors and scientists have conducted various preliminary field investigations to collect and/or verify publicly available data to be able to develop the conceptual plans. As the project progresses there will be a need for more detailed information gathering in all of these areas. Most recently in April and May 2008, environmental scientists have been onsite to verify wetlands and locate vernal pools.

19. Is the MTA's mapping accurate?

Response: Mapping resources used to date for site identification and screening is of the accepted scale, quality and resolution to meet expectations of all review and permitting agencies. As the project progresses, refined mapping and information will be gathered and used.

20. How will all of the public input be reviewed and used before selecting the preferred site to rebuild the York Toll Plaza?

Response: The Turnpike Authority is reviewing the information and confirming that all data is considered and there are no substantive data gaps for making a site selection. Any new information will be included in the site screening and permitting processes.

21. Has the public said anything that would affect the MTA's decision of rebuilding the York Toll Plaza at an alternative site?

Response: The MTA received a lot of information from the April 3, 2008 meeting. Examples of information that the MTA will pursue further includes environmental impacts, land use, public infrastructure, possibility of a cemetery and the additional meetings with a smaller core group of York residents and officials to spend more time learning various items about the project and the area.

22. Is it possible that all four sites could be rejected?

Response: Any and all of the sites are subject to elimination during the course of the study.

### 8) Environmental Considerations

1. How is air quality going to be addressed; for example ozone non-attainment area; exhaust blowing to the beaches?

Response: The Federal and State Permit process will dictate the procedures for analyzing air quality. Since this area is a non-attainment area for ozone, Maine is required to prepare State Implementation Plans (SIP) that show how the state will improve the air quality to attain the National Ambient Air Quality Standards. Both new and improvement highway projects must be contained in the area's Transportation Improvement Program (TIP). The modeling procedures for ozone and NO2 require long term meteorological data and detailed area wide emission rates for all existing and potential sources. This modeling is performed by the Maine Department of Transportation (MaineDOT) in conjunction with Metropolitan Planning Organizations (MPOs) for the region to show that regional emissions plus projects in the TIP are in conformance with the SIP and the Clean Air Act (CAA) amendments. The Portland Area Comprehensive Transportation Committee (PACTS) and the Kittery Area Comprehensive Transportation Study (KACTS) are the two MPOs responsible for this analysis. Once the MaineDOT and MPOs have completed their analysis, it is forwarded to the FHWA for final ruling on the TIP's conformance with the SIP and the CAA and its amendments. Conformance with the SIP means that the area will be on schedule with complying with the CAA and its amendments throughout the state.

2. How is lighting going to be addressed?

Response: Lighting will be developed for the selected site during the preliminary and final design stages. Lighting technology has improved over the years with the benefits being better ability to control the 'night sky' effect as well as better control of surface illumination and its reflectivity. The design will incorporate

fixtures that direct light downward and are consistent with safety practices for highway lighting.

3. How is noise going to be addressed?

Response: The noise levels along the project will be addressed according to the Maine Turnpike Authority's Highway Traffic Noise Policy. This policy parallels the Maine Department of Transportation's Noise Policy, with both policies following the criteria set forth in 23 CFR 772 which is the FHWA's highway traffic noise policy. Future noise levels will be modeled according to FHWA procedures, impacts and potential mitigation measures will based on the Highway Traffic Noise Policy.

The noise heard at a highway speed toll plaza is similar to what is heard along the mainline today and is less than what is heard at the existing plaza today. A good portion of this is attributed to the design guidelines for locating a toll plaza and the implementation of highway speed tolling. Noise will be addressed during the preliminary and final design stages.

4. How will the groundwater supply be protected?

Response: The toll plaza facility will be designed and constructed to meet current building and safety codes. Storm water management systems will meet current Maine Department of Environmental Protection standards to protect groundwater and surface waters.

5. How will adjacent streams and other waterways (that eventually lead into the ocean) be protected from stormwater pollution?

Response: For a project such as the proposed toll plaza, the Turnpike is required by law to construct stormwater management systems that meet the State of Maine requirements. Compared with older design and construction methods, new construction methods are vastly improved.

6. How are the Priority Coastal Rivers (Cape Neddick and Josias) being evaluated, treated, prevented, avoided etc?

Response: These rivers are known resources and are identified in the site selection and screening process. See responses to storm water and groundwater above. The Cape Neddick and Josias Rivers are not listed as Non-point Source Priority Watersheds, Coastal Waters or Rivers and Streams by the Department of Environmental Protection.

7. How will pollution of water supply be prevented?

Response: The York Public Water Supply is derived from surface water taken from Chases Pond. The Turnpike and toll plaza alternatives are not in the watershed of Chases Pond. The water inlet to the public system is uphill of the Turnpike and the distance from the nearest proposed work area for a toll plaza to the inlet is 1,050 feet for Site 8.7 and 900 feet for Site 9.9. Drainage from a toll plaza or the roadway cannot physically enter Chases Pond. The main water line crosses beneath the Turnpike similar to many other public utilities beneath roads

and highways. Measures will be taken to protect the pipe during construction. Crossing or relocating a water main is a routine utility protection/relocation occurrence and should not pose any pollution threat to the water supply.

### 9) Right-of-Way Considerations

1. How will access to the toll plaza be decided?

Response: Site access from an identified local road for MTA employees and other associated parties is noted in the comparison matrix of the four alternate sites in the Site Identification and Screening Report and will be further analyzed for the preferred site.

2. What is the MTA doing to consider the "human factor" when proposing a project at the scale of a new mainline plaza?

Response: The MTA is required by the regulatory permitting agencies to consider both human resource and natural resource impacts in the development of this project.

- 3. How are homes values in a poor housing market going to be fairly established? Response: It is one of the goals of the MTA not to displace anyone. However, in these situations, home values, are established using generally accepted appraisal practices such as the use of comparable sales in the same or similar markets. Because all the homes in a region are under the same market conditions, the "market value" is a relative value that rises and falls affecting all homes equally.
- 4. How much money has been set aside for purchase of land?

Response: Money has not been specifically set aside for the purchase of land. However, the MTA is committed to setting aside the amount of money necessary to assure that landowners receive fair and appropriate compensation for any land acquired.

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### Maine Gurnpike Authority

2360 CONGRESS STREET PORTLAND, MAINE 04102

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EXECUTIVE DIRECTOR
NEIL R. LIBBY
DEPUTY EXECUTIVE DIRECTOR
CHIEF FINANCIAL OFFICER
PETER S. MERFELD, P.E.
CHIEF OPERATIONS OFFICER

August 26, 2009

Michael Estes, Chairman Town of York Board of Selectmen York Municipal Office 186 York Street York, Maine 04903

Dear Chairman Estes:

I want to express my appreciation to you and your fellow Selectman for providing the Maine Turnpike Authority (MTA) with a list of thoughtful questions pertaining to the Existing Site Evaluation Report and other related issues. MTA staff and the staff of our General Engineering Consultant have worked together to provide the attached responses. Some of the responses are a bit lengthy, but we felt it was important to be as comprehensive as possible.

If you have additional questions, please forward them to us at your convenience and we will do our best to answer them as promptly and completely as possible. In addition, if you believe another meeting of the MTA Board and the York Board of Selectmen is necessary to discuss details of the ESE Report, we would be happy to arrange such a meeting.

At this juncture, given that key questions posed by the York Board of Selectmen were focused on the investigation of alternative sites, I believe it would be most productive for the Authority to accept the recommendations in the ESE Report, which include the advancement of three options at the existing location and the resumption of the investigation of alternative sites, using the new and smaller toll plaza footprint.





Page 2 of 2 Michael Estes August 26, 2009

As you know, the GEC recommended that the alternative investigation be resumed for two reasons: (1) the GEC believes that the new, smaller footprint may allow the plaza to be built at an alternative location that meets federal highway safety guidelines while avoiding the displacement of homes and minimizing the impacts to the environment and private property, and (2) the environmental permitting agencies will require that an investigation of alternatives be completed.

I believe it is in all of our interest to get the alternatives investigation underway, so that, in due time, all of the options—those at the existing site and those at alternative sites – can be put squarely on the same table for comparison. In my judgment, this is the only way to engage the public in a comprehensive and meaningful discussion of the options and to make a reasoned decision in the end. It is my intention to make a motion of this nature at the Authority's next monthly meeting on September 9, 2009. If you do not agree that this is the most appropriate course of action, I hope that you will contact me as soon as possible.

Again, I wish to thank you and your colleagues for reviewing the ESE Report and for providing us with the opportunity to respond to your questions. We look forward to working with you toward a positive solution to this matter.

Sincerely,

Gerard P. Conley Sr.

Chairman

Maine Turnpike Authority

cc: York Board of Selectmen
Robert Yandow, Town Manager
Maine Turnpike Authority

### Maine Turnpike Authority

Maine Turnpike Authority responses to questions posed by the York Board of Selectmen regarding the Existing Site Evaluation Report and other related issues.

August 26, 2009

1. In the last year, has the percentage of electronic toll use increased and, if so, by how much?

ETC usage on the Maine Turnpike System increased in 2008 over 2007 by approximately 4.7 percent. This increase is not a reliable predictor of future annual growth, because it was distorted by two significant and one-time events: (1) the New Hampshire Turnpike implemented a toll increase which caused many additional New Hampshire residents to sign up for E-ZPass to receive toll discounts and (2) the Maine Turnpike experienced temporary growth in E-ZPass transactions as a result of the I-295 rehabilitation project. ETC Transactions accounted for 52.2 percent of all transactions in 2007 and were approximately 56.9 percent in 2008.

2. If you were to chart the last 5 years what is the average increase in electronic toll use?

As illustrated in the chart below, there have been several significant events over the last five years that have resulted in dramatic, but anomalous increases in E-ZPass transactions. These are one-time events that prevent the calculation of annual average E-ZPass transactions over the last five years from being an accurate or reliable predictor of future growth. For example, in 2005 the MTA converted to the E-ZPass system and became compatible with more than 40 E-ZPass facilities, operating in 8 states. The acceptance of out-of-state E-ZPass tags resulted in a large, one-time, spike in E-ZPass transactions. Also, between July and August of 2005, the New Hampshire Turnpike incrementally launched an E-ZPass system, which contributed to a surge in Maine E-ZPass transactions in the latter part of 2005 and throughout 2006. As mentioned above, in 2008, the increase was again inflated by a toll increase on the New Hampshire Turnpike which caused many additional New Hampshire residents to sign up for E-ZPass in order to obtain toll discounts. Also in 2008, the Maine Turnpike experienced temporary E-ZPass transaction gains due to the traffic diversion to the turnpike as a result of the I-295 rehabilitation project.





### Maine Turnpike Authority - Transactions by Payment Class

		Percentage of	Percentage of Transactions	
	Increase	ETC	Cash	
2008	4.70%	56.90	43.10	
2007	2.61%	52.20	47.80	
2006	7.42%	49.59	50.41	
2005	17.63%	42.17	57.83	
2004	-0.25%	24.54	75.46	
2003		24.79	75.21	

#### Chart Notes:

- 1. February 1, 2005, MTA implements E-ZPass, making Maine Turnpike's system comparable with all other E-ZPass states, which caused a surge in E-ZPass transactions
- 2. In mid-2005, the New Hampshire Turnpike implemented E-ZPass, which increased Maine E-ZPass transactions in late 2005 and throughout 2006.
- 3. In 2008, New Hampshire Turnpike implemented a toll increase which caused many additional New Hampshire residents to sign up for E-ZPass, thus inflating Maine transactions. Also in 2008, the Maine Turnpike experienced temporary E-ZPass transaction growth due to the southbound closure of I-295 for construction.
- 3. Based on statistical data what do you see as the estimated time frame for electronic tolling to be the norm?

It is not clear if this question is inquiring about the future of "electronic toll collection (ETC)," meaning an E-ZPass-type system or similar automated vehicle identification technology, or "all-electronic toll collection (AET)" meaning a system in which cash toll collection is eliminated entirely and all tolls are collected through a duel system involving (1) an E-ZPass type system (2) video tolling system that would capture license plate images of all non E-ZPass users, match their license plates to a mailing address data base and mail them a bill for the toll. Therefore, we will provide information regarding the likely future of both methods on the Maine Turnpike. Also, the term "norm" is somewhat subjective, but we will attempt to provide you with information that should satisfy the substance of your question.

### Electronic Toll Collection (ETC)

The use of "electronic tolling" (E-ZPass-type system) is already the industry norm as one of the standard methods of toll collection. Most toll agencies offer some form of "electronic tolling".

The Maine Turnpike Authority estimates that ETC usage will continue to grow, but at a slower rate than has been the case since 2004. As noted in an earlier response, E-ZPass

transactions account for nearly 57 percent of all transactions. As noted in the Existing Site Evaluation, the growth in E-ZPass usage is predicted to be approximately 3 percent per year over the next few years, but it is expected to slow to about 1 percent per year thereafter. This is primarily due to the fact that over the past 12 years the market of frequent Turnpike users has been saturated by the E-ZPass product. This is strongly indicated by the fact that the majority of new E-ZPass accounts being opened today are by people who travel the Maine Turnpike less than once per week. For the most part, frequent users are already in the program. As a result, the E-ZPass tags issued today are generating a significantly smaller number of ETC trips than those issued several years ago. The MTA anticipates that ETC usage will reach approximately 65 percent of transactions by the end of 2014. Annual percentage growth beyond that date will be slight due the maturity of the market.

### All-Electronic Toll Collection (AET)

All-Electronic Tolling (AET) is a very recent approach that just three agencies have implemented on five toll roads and four more agencies are considering for four more roads. This is a small number considering that there are 85 toll highways across the United States. Further, no AET systems are currently operating or are being planned for the New England region, even by agencies that are upgrading their ETC capabilities. For example, the New Hampshire Turnpike is scheduled to implement an Open Road Tolling system at the Hampton Toll Plaza on I-95, much like the one proposed by the Maine Turnpike Authority. They will not be implementing an AET system at that location.

In addition to the small number of installations, there are a number of important factors (specific to each agency and toll road) that must be taken into account when evaluating the feasibility of AET for a particular facility. They include, but are not limited to the, percentage of electronic toll usage, concentration of commuter traffic, in-state vs. out of state traffic, ability to recover and enforce toll payment, financing and legislative restrictions. Because these factors vary widely between facilities, there is currently no clear indication that the industry will move to AET, as a "norm", at any predictable point in the future.

With respect to AET on the Maine Turnpike, the MTA was initially intrigued by the concept of eliminating cash tolls and collecting all tolls electronically. As a result the MTA asked its General Engineering Consultant (GEC) to include an All-Electronic Tolling Feasibility Review (AET Review) as part of the Existing Site Evaluation Report (ESE Report). As stated on page 13 of the ESE Report: "no existing cash-based agency has completed a total conversion to AET and therefore there is little to no available comparable information to assist other agencies with forecasting the applicability of AET for their own roadway."

The potential applicability of AET to the Maine Turnpike is discussed in detail in the AET Report. In summary however, the report indicated that AET may be feasible on a limited number of highways that serve (1) a high percentage of E-ZPass users, (2) a high percentage of commuters, and (3) a high percentage of users that reside with a common jurisdiction, making it possible to obtain accurate information to bill customers for video tolls and to enforce payment if necessary.

For example, the 407 ETR in Canada is a commuter-dominated artery serving the City of Toronto with ETC usage rates exceeding 80 percent. In the U.S., the Central Texas Turnpike operates AET on SH 183 A and SH 130, both of which are also commuter dominated highways that enjoy ETC usage rates of about 85 percent.

The AET report concluded that AET is not feasible for a facility like the Maine Turnpike that shares none of the above mentioned essential characteristics. The Maine Turnpike is not a commuter-dominated highway. It serves a widely diverse customer base, which includes a large percentage of infrequent users and visitors from out-of- state.

Today, about 43 percent of the vehicles traveling through the York Toll Plaza pay tolls in cash. Under an AET system those tolls would have to be collected by obtaining a video image of each license plate, matching that license plate to a Maine or out-of-state mailing address, and mailing the owner of the vehicle a bill for the toll. The process itself would be expensive and highly unreliable and largely unenforceable, particularly given the fact that no effective reciprocity and enforcement system between states exists. The report estimates that the adoption of AET could put at risk as much as \$17 million per year in uncollectable revenue.

Failure to effectively collect video tolls from Maine's diverse customer base would result in higher tolls for those frequent, in-state users who depend on the Maine Turnpike. The GEC concluded that there is no reason to expect that reciprocity and enforcement agreements or the Maine Turnpike's diverse customer base will change to such an extent that all-electronic toll collection will become a viable option during the next 20 year period.

### Open Road Tolling (ORT)

While AET may not be a feasible or financially responsible option for the Maine Turnpike in the foreseeable future, the MTA is planning to introduce Open Road Tolling (ORT) at the York Toll Plaza and at other suitable mainline toll plazas. ORT, also known as Highway Speed Tolling, will enable E-ZPass users to pay their tolls by simply passing beneath a sensor at normal highway speeds. The system would continue to accommodate cash paying customers, who would briefly depart the mainline of the highway to pay at a traditional toll plaza. ORT is discussed on pages 14 and 15 of the ESE Report.

### 4. Do you have a preferred site for the toll plaza and, if so, where is it?

The MTA does not have and will not have a preferred site for a new toll plaza until it has completed the multi-phased, evaluation process, referred to as "The Highway Methodology". This process is prescribed by the United States Army Corps of Engineers (USACE) and considers engineering criteria, environmental impacts, public input, cost and regulatory agency review.

A brief explanation of where we stand today and what must be accomplished before the MTA can settle on a preferred alternative may improve understanding of this issue.

The established process referenced above began in 2005 with the definition of the Purpose and Need statement. By the spring of 2008, it had progressed to the site screening and selection stage. The process was suspended, however, in April of 2008 due to concerns raised by the public, which prompted the York Board of Selectmen to request that the MTA redirect the GEC to conduct a more detailed study of the existing toll plaza location to determine "what it would take" to construct a safe and efficient toll plaza there. The GEC had previously eliminated the existing site from consideration early in the process because its location near an interchange, on a curve, at the bottom of hill and other physical deficiencies violated the basic engineering guidelines set out by the Federal Highway Administration for the construction of safe and efficient toll plazas.

On June 16, 2009 the GEC presented its conclusions and recommendations of the Existing Site Evaluation to the MTA Board and York Board of Selectmen. The GEC recommended the advancement of a "no build" option and two additional options at the existing location for further consideration. The GEC also recommended that the MTA resume its investigation of alternative locations beyond the existing location, emphasizing that the environmental permitting agencies would require such an investigation, particularly in light of the potential environmental impacts and the considerable costs of the options recommended for advancement at the existing location. Furthermore, the Federal Clean Water Act Section 404 (b)(1) Guidelines requires all practicable alternatives be considered, which in this instance must include alternative site locations.

The GEC also reported that the advanced engineering conducted as part of the existing location study, particularly the reduction in size of the plaza from 23 to 15 lanes, may significantly reduce the environmental and community impacts at alternative sites. The GEC indicated that the smaller plaza footprint may now allow a new plaza to be built at an alternative location without displacing any homes and while meeting the engineering and safety guidelines, minimizing environmental and private property impacts and reducing project costs.

The process going forward will include the following:

- A) Following the USACE Highway Methodology (HWM), the GEC will complete the Site Identification and Screening Study for alternative locations, using the new, smaller plaza size. (Much of this work was accomplished during the initial Site Identification and Screening Study that began in 2007 and was suspended in spring of 2008 in order to conduct a more detailed evaluation of the existing site, as requested by the York Selectmen.)
- B) The GEC will prepare a comparison of the best of the existing site locations and the best of the alternative site locations, based on satisfying the project purpose and need, avoidance and minimization of environmental and community impacts and practicable costs.
- C) The York Select Board will be invited to participate in a MTA meeting, during which the GEC will present the comparison of the best sites at both the existing site location and the alternative site locations, as well as a recommended short list of sites to be advanced for further consideration.
- D) The MTA staff will hold a public meeting in York to present and receive comment on the GEC's comparison of the best sites at both the existing site location and the alternative site locations and the GEC's recommended short list of sites to be advanced for further consideration.
- E) The MTA will finalize and submit a Phase 1 Study Report to the USACE, which will include the recommended short list of sites, as well as the process used to comply with the Highway Methodology to select those sites.
- F) The USACE and other permitting agencies will review the Phase 1 Study Report and must confirm that the Highway Methodology was properly followed and that the short list of sites is appropriate.
- G) The GEC begins Phase 2 of the USACE Highway Methodology by refining designs and conducting additional field investigation for the short list of sites.
- H) The GEC will prepare a comparison of the short list of sites, as well as a recommended preferred alternative based on additional engineering, avoidance and minimization of environmental and community impacts and practicable costs.
- I) The York Select Board will be invited to participate in a MTA meeting, during which the GEC will present the comparison of the short list of sites, as well as a recommended preferred alternative.

- J) The MTA Board will hold a public hearing in York to present and receive comment on the GEC's comparison of the short list of sites, as well as the recommended preferred alternative.
- K) The MTA will finalize and submit Phase 2 Report containing the preferred alternative.

It is important to note that once a preferred site is submitted by the MTA, the U.S. Army Corp of Engineers must certify that the preferred site is also the Least Environmentally Damaging Practicable Alternative (LEDPA). The MTA may not submit applications for environmental permits until this certification is received from the USACE

5. If the York toll booth was all electronic tolling how large would it have to be?

For initial conversation, an (AET) toll plaza may consist of an overhead frame capable of holding AET sensors and cameras, and a small utility building to house related infrastructure necessary to run the AET system. Given the nature of AET, additional lanes beyond mainline lanes are not necessary. However, there would need to be room for potential mainline lane widening, clear zone width to the overhead frame foundation, the foundation itself and the utility building. For conceptual purposes, this could fit within the MTA's existing Right-of-Way

6. According to the FHWA's recommended guidelines for plaza location and design, "proposed plaza construction and modifications should be designed with anticipation of increasing ETC utilization, and eventual removal of conventional plazas ...." (P. 15, State of the Practice and Recommendations on Traffic Control Strategies at Toll Plazas)

This is why the MTA is planning a plaza with Open Road Tolling (ORT) and the minimal number of cash lanes to ensure the safe and efficient flow of traffic. Additionally, as ETC penetration grows, justifying additional ORT lanes, cash lanes will be removed to make way for the additional ORT lanes. In this manner, the MTA is using the same space twice – first as a cash lane and then as an ORT lane when appropriate. The new plaza will be designed to accommodate a relatively simple and inexpensive removal of manual cash lanes and the installation of ORT lanes when necessary.

The FHWA's State of the Practice and Recommendations on Traffic Control Strategies at Toll Plazas goes on to say the following: "Economical conventional plaza design and construction is desirable where there is no existing regional use of ETC, cash collection metering affectively improves facility operations, and relatively low commuter traffic volumes are forecasted."

The operative phrase above is "relatively low commuter traffic volumes are forecasted". Currently about 11% of the traffic passing through the York Toll Plaza is commuter traffic, with 7% enrolled in the MTA's commuter discount program. About 51 percent of the vehicles passing through York Toll Plaza are from out-of-state. The low percentage of commuter customers combined with the high percentage of out-of-state travelers add to the already considerable future uncertainty about effectiveness of collecting tolls and enforcing payment through the use of AET. This uncertainty translates into significant financial risk.

7. How does the MTA reconcile its reliance on FHWA guidelines with its intention to spend tens of millions of dollars and uproot York citizens to build a new toll plaza incorporating conventional tolling?

This question raises a number of separate, but related issues which we will attempt to address individually below. As an introduction to our responses, however, it may be useful to cite language contained in the original legislation that created the Maine Turnpike Authority in 1941:

"The economic and social well-being of the citizens of the State requires that the transportation system be developed in a comprehensive manner and depends upon the safety, efficiency and modern functional state of the turnpike."

To fulfill this statutory charge, the MTA is required to undertake capital improvements to ensure the safety, efficiency and modern functional state of the highway, its bridges, toll plazas and other critical components of the infrastructure. In doing so the MTA must carefully consider and attempt to properly balance the interest of Maine citizens, our state's economy, the environment, our customers, neighboring communities, nearby landowners and a myriad of other concerns in an effort to serve the public good. This is often a difficult and challenging responsibility, but the MTA has a long and documented history of meeting this responsibility successfully.

### Federal Highway Administration guidelines

The purpose of Federal Highway Administration guidelines is to promote safety, consistency and the use of best practices with respect to the construction, rehabilitation and repair of the nation's highways. Just as cities and towns throughout Maine utilize nationally recognized building codes to ensure the long term security and safety of their citizens, the Maine Turnpike Authority utilizes Federal Highway Administration Safety guidelines to ensure the safety of the more than 100 million people who travel the highway every year.

### Costs

The York Toll plaza is a vital piece of Maine's transportation infrastructure. It currently serves more than 16 million vehicles per year and generates more than \$37 million per year, which is used to maintain the state's most important highway.

The York Toll Plaza was constructed in 1969 with an expected structural lifespan of 25 years. It is now approaching its 41<sup>st</sup> year of operation. The deteriorating condition of the plaza has become a significant and increasing concern with respect to the safety of both motorists and employees. As stated in the Existing Site Evaluation Report, Section 3, Project Purpose and Need, "Based on the York Toll Plaza's crash rate history and operational performance, it is clear that the present plaza cannot deliver, today or in the future, as safe, efficient and modern operation as required of the turnpike."

A significant investment will be required to replace the plaza, whether it is replaced at the existing location or elsewhere. It is the MTA's obligation to Maine citizens, toll payers and bondholders to ensure that the end result of that investment is a safe, efficient and modern toll plaza that not only addresses the deficiencies of the existing plaza, but is capable of performing safely well into the future.

### Home Displacement

It has never been the intention of the MTA to "uproot York citizens." On the contrary, it has always been the intention of the MTA to build a safe, efficient and modern toll plaza while minimizing and if possible eliminating the need to displace homes or impact private property. As we have explained from the beginning of this study, the US Army Corps of Engineers' site selection process begins by identifying all possible sites that meet basic engineering criteria and by considering the largest potential impacts (worst case scenarios) at each of those locations. From that point, the process is primarily devoted to the avoidance or minimization of those impacts, with the goal of meeting the requirement of the environmental permitting agencies to advance the Least Environmentally Damaging Practicable Alternative.

This commitment to impact avoidance and minimization is evident in the progress that has been made to date with respect to the York Toll Plaza Study. The recommendations included in the GEC's Existing Site Evaluation Report not only called for the advancement of three options at the existing toll plaza site, none of which require the displacement of homes, but also calls for the resumption of the alternative site investigation based on the GEC's confidence that the revised, minimized toll plaza could be constructed at an alternative location while achieving FHWA guidelines and without displacing any homes.

### Cash Toll Collection

While the MTA looks forward to the introduction of Open Road Tolling at the York Toll Plaza, which will allow E-ZPass users to pay their tolls by simply passing beneath a sensor at normal highway speed, we also recognize that the need to offer the option of cash toll payments will continue well into the foreseeable future. The MTA's position on this issue is supported in considerable detail by the GEC in the AET Report. This issue was also addressed at some length in response to Question #3. In short, the GEC concluded that the Maine Turnpike's highly diverse customer base, which is largely made up of infrequent users and out-of-state travelers who may originate from states that do not have E-ZPass, demands that the Maine Turnpike continue to offer cash toll collection in order to effectively and fairly collect revenue from all Turnpike users.

8. Why is the MTA applying dimensional standards for the location of new toll plazas to an existing plaza?

The existing toll plaza needs to be replaced. This will require a significant expenditure of funds whether at the existing location or at a new location. It is the MTA's goal to develop a replacement plan that is most prudent, which includes meet ing as many of the appropriate standards for good operation and safety as possible, avoiding and minimizing impacts to community and environmental resources and doing so with the least expenditure of funds.

In developing a comprehensive replacement plan, a number of problems or deficiencies can be identified (and subsequently addressed) without reference to any specific standards, e.g. rusting support columns, leaking roof, corroding electrical wiring, etc. There are also a number of symptoms that require more in-depth investigation to understand the underlying problem or deficiency. These deficiencies often require a standard against which to be compared to determine acceptability. It is these standards, e.g. sight distance, distance from an interchange, proximity to a curve or hill that forms the basis of a comprehensive evaluation and recommended resolution. Whether a toll plaza exists or is proposed, its merit and consequences are still measured against these standards. The existing York Toll Plaza, in order to function according to "best practices" and national guidelines, must be evaluated against these same practices and guidelines.

9. If, as the MTA argues, the York toll plaza is in such a conspicuously deficient location, why did HNTB build it in a mire on a curve at the bottom of a hill to begin with?

The location of the existing York plaza was not selected by HNTB or the MTA, nor was its location based on engineering criteria or best practices. Its location was primarily determined by political negotiations between state and federal transportation officials surrounding the construction of the Piscataqua River Bridge and the new section of

highway connecting the bridge with the Maine Turnpike. Both HNTB and the MTA opposed the decision at the time. Knowledge of this history and its long term consequences, with which we are now dealing, serve as a reminder as to why engineering and environmental best practices should factor heavily into long term transportation investment decisions. Fortunately, the strengthening of the environmental permitting process over the last 40 years, in particular the USACE Highway Methodology, combined with the recent development of FHWA guidelines for toll plazas, requires a more deliberative and accountable decision making process for today's significant capital projects.

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### Maine Gurnpike Authority

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GERARD P. CONLEY, SR., PORTLAND, CHAIRMAN LUCIEN B. GOSSELIN, LEWISTON, VICE CHAIRMAN DOUGLAS A. VOLK, PORTLAND, MEMBER RICHARD E. VALENTINO, SACO, MEMBER THOMAS B. FEDERLE, ESQ., MANCHESTER, MEMBER DIANE M. DOYLE, SACO, MEMBER GREGORY G. NADEAU, LEWISTON, MEMBER EX-OFFICIO

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EXECUTIVE DIRECTOR
NEIL R. LIBBY
DEPUTY EXECUTIVE DIRECTOR
CHIEF FINANCIAL OFFICER
PETER S. MERFELD, P.E.
CHIEF OPERATIONS OFFICER

September 3, 2009

Ms. Joan Jarvis P.O. Box 519 York Harbor, ME 03911

Dear Ms. Jarvis: Joan

Thank you for submitting questions on behalf of Think Again regarding the Existing Site Evaluation and other related issues. MTA staff and the staff of our General Engineering Consultant have worked together to provide the attached responses.

Given that key questions posed by both the York Board of Selectmen and Think Again were focused on the investigation of alternative sites, MTA Chairman Conley believes that it would be most productive for the Authority to accept the recommendations in the ESE Report, which include the advancement of three options at the existing location and the resumption of the investigation of alternative sites using the new and smaller toll plaza footprint.

As you know, the GEC recommended that the alternative investigation be resumed for two reasons: (1) the GEC believes that the new and smaller footprint may allow the plaza to be built at an alternative location that meets Federal Highway Administration safety guidelines while avoiding the displacement of homes and minimizing the impacts to the environment and private property, and (2) the environmental permitting agencies will require that an investigation of alternatives be completed.

Mr. Conley believes that it is in all of our interests to get the alternatives investigation underway, so that, in due time, all of the options—those at the existing site and those at alternative sites – can be put squarely on the same table for comparison. This seems to be the only way to engage the public in a comprehensive and meaningful discussion of the options and to make a reasoned decision in the end. I expect that Mr. Conley will entertain a motion to this effect at the Authority's next monthly meeting on September 9, 2009.



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THE GOLD STAR

MEMORIAL HIGHWAY

Page 2 of 2 Ms. Joan Jarvis September 3, 2009

Again, I wish to thank you and your colleagues for reviewing the ESE Report and for providing us with the opportunity to respond to your questions. We look forward to working with you toward a positive solution to this matter.

Sincerely,

Conrad W. Welzel

Governmental Relations and Planning Manager

cc: Paul E. Violette
Maine Turnpike Authority

### Maine Gurnpike Authority

## Maine Turnpike Authority responses to questions posed by the Think Again Group

September 3, 2009

1. What are the "Other options"?
Which options are viable and why?

If the Maine Turnpike Authority (MTA) approves the recommendation of its General Consulting Engineer (GEC) to resume the evaluation of alternative options, it is anticipated that that evaluation will begin by applying the revised, smaller plaza size to each of the 16 locations that were identified as meeting the basic engineering criteria during the early stages of the study. The viability of each of those locations cannot be determined until the GEC has completed the evaluation. While the smaller plaza footprint will likely reduce the various impacts at each location, the overall guidelines and criteria that formed the initial evaluation will remain consistent.

The evaluation process is prescribed by the U.S Army Corp of Engineers (USACE) Highway Methodology and is designed to consider engineering, environmental and community impacts, public input, costs and regulatory agency review. This process was initiated in earnest in 2005, but was suspended in April 2008 due to concerns raised by the public, which prompted the York Board of Selectmen to request that the MTA redirect the GEC to conduct a more detailed study of the existing toll plaza location (Existing Site Evaluation). The MTA directed the GEC to complete such an evaluation.

On June 16, 2009, the GEC presented the conclusions and recommendations of the Existing Site Evaluation to the MTA Board and York Board of Selectmen. The GEC recommended the advancement of a "no build" option and two additional options at the existing site for further consideration. The GEC also recommended that the MTA resume its investigation of alternative locations beyond the existing site, emphasizing that the environmental permitting agencies would require such an investigation, particularly in light of the potential



THE GOLD STAR MEMORIAL HIGHWA environmental impacts and the considerable costs of the options recommended for advancement at the existing site. Furthermore, the Federal Clean Water Act Section 404 (b)(1) Guidelines requires all practicable alternatives be considered, which in this instance must include alternative locations.

The GEC also reported that the advanced engineering conducted as part of the Existing Site Evaluation (ESE), particularly the reduction in size of the plaza from 23 to 15 lanes, may significantly reduce the environmental and community impacts at alternative locations. The GEC indicated that the smaller plaza footprint may now allow a new plaza to be built at an alternative location without displacing any homes and while meeting the engineering and safety guidelines, minimizing environmental and private property impacts and reducing project costs.

### 2. Now that the scope of the project has changed what is the new criteria?

The scope of the project has not changed. The scope of the project continues to be the replacement of the existing plaza with a safe, efficient and modern plaza that is able to accommodate Open Road Tolling. The criteria for this project consists of meeting the design guidelines published by the FHWA regarding toll plazas and avoiding or minimizing environmental impacts as defined by the various permitting agencies. These criteria as applied to alternative locations will be evaluated using the USACE's Highway Methodology, which considers engineering criteria, environmental and community impacts, public input, costs and regulatory agency review in order to identify the Least Environmentally Practical Alternative (LEDPA). The MTA must comply with this established method in order to obtain the necessary environmental permits and does not have the authority or power to change it.

The only significant factor that has changed has been a reduction in the footprint of the proposed toll plaza. This footprint reduction would have normally occurred later in the course of design development following the USACE screening and site selection process, which begins by considering the largest potential impacts (worst case scenarios) at each potential site, but then becomes focused on the avoidance and finally minimization of those impacts. However, in this case, the new and smaller footprint was arrived at after the MTA had suspended the site selection process in order to comply with the request of York Selectmen to conduct a more in-depth study of the existing site. As part of this Existing Site Evaluation (ESE), the GEC conducted a detailed traffic analysis along with advanced engineering that indicated that the plaza size could, in fact, be reduced. This smaller plaza size must now be applied to other sites in order to provide a comprehensive comparison and to comply with the USACE Highway Methodology.

a. Have all the impacts on the York Water District been evaluated and the costs incurred been included in the costs projections?

As noted above, the alternate site evaluation process was suspended in April of 2008 in order to comply with the request of York Selectmen to conduct a more indepth study of the existing location. In keeping with that commitment, no additional evaluation of alternative locations and their potential impacts on the York Water District or any other concern has been advanced over the last 15 months. The GEC had initiated discussions with the York Water District prior to the suspension of the alternative site evaluation process and will renew those discussions if and when the process is resumed. Any impacts on the York Water District and any costs resulting from those impacts will be thoroughly evaluated, using the new and smaller toll plaza footprint.

3. What has become of the MTA's commitment to innovative practices? With full electronic tolling already in operation in places in the US and Canada why is the MTA unwilling to adopt the future?

The MTA has been and will continue to be a leading innovator in the transportation industry. The Maine Turnpike was the first express highway constructed in New England. It was the first highway in the nation to be constructed without taxpayer dollars. The MTA was the first highway in New England to introduce electronic toll collection. Its management of the recent Turnpike Widening project is recognized as a national model for safety, efficiency and public communication. Today, the Maine Turnpike's fleet of maintenance vehicles is fueled by bio-diesel and its headquarters is designed as a LEED Certified green building. Within a year, the MTA will initiate Maine's-first truck stop-electrification pilot project that provides truckers with electricity for heat and other services, eliminating the need to idle their engines all night long.

In recognition of its sound and forward thinking business practices, the MTA's credit rating is ranked among the top six toll agencies in the nation, which enables the Authority to finance projects at the lowest rates of interest.

With respect to this project, the MTA is committed to the construction of a state-of-the-art facility, featuring Open Road Tolling, which will improve service to our customers while providing significant operational, cost containment and environmental benefits.

All-Electronic Tolling (AET) is a very recent approach that just three agencies have implemented on five toll roads and four more agencies are considering for

four more roads. This is a small number considering that there are 85 toll highways across the United States. Further, no AET systems are currently operating or are being planned for the New England region, even by agencies that are upgrading their ETC capabilities. For example, the New Hampshire Turnpike is scheduled to implement an Open Road Tolling system at the Hampton Toll Plaza on I-95, much like the one proposed by the Maine Turnpike Authority. They will not be implementing an AET system at that location.

In addition to the small number of installations, there are a number of important factors (specific to each agency and toll road) that must be taken into account when evaluating the feasibility of AET for a particular facility. They include, but are not limited to the, percentage of electronic toll usage, concentration of commuter traffic, in-state vs. out of state traffic, ability to recover and enforce toll payment, financing and legislative restrictions. Because these factors vary widely between facilities, there is currently no clear indication that the industry will move to AET at any predictable point in the future.

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For example, the 407 ETR in Canada is a commuter-dominated artery serving the City of Toronto with ETC usage rates exceeding 80 percent. In the U.S., the Central Texas Turnpike operates AET on SH 183 A and SH 130, both of which are also commuter dominated highways that enjoy ETC usage rates of about 85 percent.

The AET report concluded that AET is not feasible for a facility like the Maine Turnpike that shares none of the above mentioned essential characteristics. The Maine Turnpike is not a commuter-dominated highway. It serves a widely

diverse customer base, which includes a large percentage of infrequent users and visitors from out-of- state.

Today, about 43 percent of the vehicles traveling through the York Toll Plaza pay tolls in cash. Under an AET system those tolls would have to be collected by obtaining a video image of each license plate, matching that license plate to a Maine or out-of-state mailing address, and mailing the owner of the vehicle a bill for the toll. The process itself would be expensive and highly unreliable and largely unenforceable, particularly given the fact that no effective reciprocity and enforcement system between states exists. The report estimates that the adoption of AET could put at risk as much as \$17 million per year in uncollectable revenue.

Failure to effectively collect video tolls from the Maine's Turnpike's diverse customer base would result in higher tolls for those regular, in-state users who depend on the highway for frequent, if not daily travel. The GEC concluded that there is no reason to expect that reciprocity and enforcement agreements or the Maine Turnpike's diverse customer base will change to such an extent that all-electronic toll collection will become a viable option during the next 20 year period.

4. Can it truthfully be said that these "demonstrated deficiencies" are still valid and sustainable under future examination during the permitting process?

Yes. The York Toll Plaza was constructed in 1969 with an expected structural lifespan of 25 years. It is now approaching its  $41^{\rm st}$  year of operation -16 years beyond its expected structural lifespan.

While the deficiencies of the plaza are exacerbated by heavy traffic volumes, traffic growth is not the primary driver behind the need to replace the facility. The primary reasons for replacing the plaza are its age, its deteriorated condition, its rate of deterioration, its constant and costly need for repair, its inability to accommodate new toll technology and most importantly, the fact that it is becoming increasingly unsafe for motorists and employees. The deteriorating conditions of the plaza are not traffic growth dependent. They will continue to worsen regardless of how many vehicles the plaza serves. A deteriorated and unsafe plaza is a deteriorated and unsafe plaza regardless of how many vehicles it serves annually.

As explained in the ESE Report, Section 3, Project Purpose and Need, the existing toll plaza's deficiencies can be separated into two areas, physical and operational. The Report states: "First the physical needs are due to the poor and

failing condition of the physical infrastructure itself, including booths, canopy, access tunnel, the space limitations of the existing tollbooths, the absence of adequate toll staff protection, and the poor soil conditions. Second, the operational needs are demonstrated by the design deficiencies of the existing York Toll Plaza — a plaza and approach area that restricts operational efficiencies and meets none of the recently published, FHWA design guidelines for toll plazas. Proximity to an interchange, poor or non-existent sight distance and poor alignment have led to a high number of crashes resulting in the plaza being classified as the 11th highest crash location in the State out of over 900 such locations.

Section 3 of the ESE report concludes by stating: "Based on the York Toll Plaza's crash rate history and operational performance, it is clear that the present plaza cannot deliver, today or in the future, a safe, efficient and modern operation as required of the turnpike."

## 5. Why is old traffic and safety data being used?

The traffic and crash data used in reports throughout the course of this study represent the most current data available at the time of preparation. Further, the GEC has continually updated the data throughout the course of the study and will continue to do so.

It is also important to understand that most engineering studies employ data that is reported based on trends and not simply a "point-in time" statistic. Data based on trends provides more information and greater confidence to make long term decisions. The traffic and crash data included in the ESE are two such pieces of data that have been analyzed as trends.

The traffic analysis in sections 5 & 6 of the "Existing Site Evaluation" was based in part on a detailed review of two important traffic trends.

- The first trend was *growth in design-hour* traffic. The GEC reviewed hourly traffic at the toll plaza from 2000 through 2007 (the date of the initial analysis) in order to evaluate a reasonable rate of growth for the future.
- The second trend was *growth in the share of E-ZPass traffic*. The GEC reviewed the extent to which peak-hour E-ZPass usage has grown at York, from the inception of electronic toll collection in 1997 through to 2007 (the date of the initial analysis). Projections about how these trends would carry forward to the future were then made.

The analysis was initially conducted based on peak-hour volumes observed in the summer of 2007. The results of this analysis were initially distributed in the spring of 2008. As the project moves into final design, these trends will be once again updated based on recent trends.

With respect to crash statistics, this data is also continually updated and reviewed and is used based on longer term trends versus "point-in-time" data values. For instance crash data, while evaluated annually, is a compilation of three years worth of data. The Existing Site Evaluation references two three year compilations, 2003-2005 and 2004-2006. Recent reviews include the 2005-2007 data set and as recent as July of this year we have obtained the latest available crash data, 2006-2008, that continues to show unacceptable crash trends at the plaza.

## a. Don't HST lanes generate more serious accidents?

As part of responding to this question a few points clarifying the operation of highway speed lanes may be helpful. Highway Speed Tolling, currently referred to as Open Road Tolling (ORT), when designed according to FHWA's State of the Practice operates similar to any properly designed interchange and adjacent mainline. That is, E-ZPass users will remain on the mainline and simply pass beneath a sensor that reads the toll tag; similar to passing beneath an overpass or sign bridge. Cash customers will be required to exit the mainline on a ramp designed like any other interchange ramp. From the ramp the user will enter a cash toll plaza, pay their toll, then accelerate and reenter the mainline via an entrance ramp, again designed like any other interchange ramp From exit ramp to entrance ramp the two toll streams, traveling at different speeds, are separated by a concrete or similar barrier for safety. Highway speed lanes, as they simply pass beneath a toll sensor gantry, will not generate crashes. These locations are like other sections of roadway much like passing beneath a bridge. Given that the size of the gantry would be much smaller and less constricting than a bridge and more like an overhead sign bridge, it is possible the gantry will go undetected by the motorists.

Crash rates and trends for the complete ORT plaza area involve analysis of more than just the highway speed lane. An ORT plaza includes a length of mainline and separated cash roadway. extending from where the cash lane diverges from mainline, through the ORT gantry or cash toll plaza and to the point where the cash lanes merge back with the mainline. The GEC's experience with a number of ORT facilities around the country has been that each facility experienced similar crash trends. There was roughly a 50% reduction in both the total number of accidents and the total number of injuries within the plaza areas following the installation of ORT. It was noted that many of the post-ORT installation accidents shifted to the diverge and merge points, essentially the ramp ends. As a point of reference, the accident rates observed at the ramp ends are similar to accident rates associated with typical highway interchanges.

The GEC predicts crash rates of an ORT plaza at the existing location would be higher than at a new alternate location. This is because a new location would be selected such that the installation met basic engineering guidelines. Installation of ORT at the existing site would not meet all of these basic guidelines which were developed to improve safety. Furthermore, the severity of crashes would also likely be higher at the existing York location versus at a new location based again on the extent to which the installation met basic engineering guidelines.

When considering an ORT installation at an alternate location there is another expected benefit; the crash rate for the interchange adjacent to the existing toll plaza is expected to go down. With the plaza relocated the traveling public could maneuver thru the remaining interchange area with a single focus – the interchange.

6. What has become of the MTA's commitment of getting along with the local communities? York, Wells and Ogunquit have all told the MTA they are against moving the plaza and taking private land and homes. Taking of "only" land can have a disastrous effect on a homeowner and leave the turnpike too close to homes.

The MTA has a long history of maintaining excellent relationships with communities along the Turnpike corridor and is committed to continuing this practice. With respect to this project, the MTA has demonstrated its willingness to work with the communities by agreeing to suspend the overall site evaluation process for more than a year in order to comply with the York Selectmen's request to conduct a more detailed evaluation of the existing site.

The original legislation that created the Maine Turnpike Authority in 1941 states the following: "The economic and social well being of the citizens of the State requires that the transportation system be developed in a comprehensive manner that depends upon the safety, efficiency and modern functional state of the turnpike.

To fulfill this statutory charge, the MTA is required to undertake capital improvements to ensure the safety, efficiency and modern functional state of the highway, its bridges, toll plazas and other critical components of the infrastructure. In doing so the MTA must carefully consider the often divergent interests of Maine citizens, the state's economy, the environment, turnpike customers, neighboring communities, nearby landowners and a myriad of other concerns in an effort to arrive at a solution that best serves the public good. While the MTA appreciates and takes very seriously the communications it has received from individuals and communities, it must consider these expressions

in the context of its overall responsibility to provide safe, efficient and modern transportation for the citizens of the State of Maine and other Turnpike users. The MTA continues to believe that through hard work and continued communications, we will succeed in developing a solution that addresses the substantive concerns expressed by the communities while also addressing the needs of the State as a whole.

## 7. When will we be told the MTA's directive to the HNTB?

The initial directive from the MTA to its GEC, HNTB, was to evaluate the condition of the existing plaza, to make recommendations to address identified deficiencies and to incorporate Open Road Tolling (ORT).

The MTA directed the GEC to evaluate the corridor between Kittery and Wells in order to identify possible locations for a replacement toll plaza that meet basic engineering guidelines and are able to accommodate ORT.

In May of 2008, at the request of the York Selectmen, the MTA directed the GEC to suspend the evaluation of sites between Kittery and Wells and to initiate a separate, more in-depth evaluation of the existing site.

On June 16th 2009 the GEC presented the conclusions and recommendations of the Existing Site Evaluation to the MTA and the York Selectmen. The GEC recommended the advancement of a "no build" option and two additional options at the existing location for further investigation. The GEC also recommended that the MTA resume its investigation of alternative locations beyond the existing location, emphasizing that the environmental permitting agencies would require such an investigation, particularly in light of the potential environmental impacts and the considerable costs of the options recommended for advancement at the existing location. Furthermore, the Federal Clean Water Act Section 404 (b) (1) Guidelines requires all practicable alternatives be considered, which in this instance must include alternative site locations.

It is expected that the MTA will consider the GEC's recommendations at its next monthly meeting on September 9, 2009. If the MTA accepts the GEC's recommendation it is likely that the MTA will direct the GEC to resume its investigation of alternative locations.

8. Has a business case analysis or a return on investment analysis been conducted for this project?

With respect to the York Toll Plaza, the efforts completed to date by the MTA and its GEC are by their very nature components of a business case analysis. Answering the question, "What is the most effective treatment of the York Toll Plaza considering safety of the traveling public, environmental and community impacts, long term viability of the treatment and costs to the Maine Turnpike users?", involves a business model or return on investment analysis unlike most others. It involves a process by which each of these variables must be evaluated, understood and ultimately quantified such that various alternative treatments can be compared. Further, the majority of this analysis falls within the review of the USACE Highway Methodology.

The MTA has been reviewing a course of actions to take at the York Toll Plaza for some time. As a part of this, the MTA has undertaken studies with the intent of finding the most cost effective solution for the condition. The condition of the toll plaza and the site upon which it sits led to the determination that the required maintenance would outpace the benefits derived from the maintenance/repair activities. The MTA, understanding this basic premise, decided to undertake a review of what should be done to address the circumstance of this failing but vital piece of infrastructure.

This review was undertaken in a series of studies, reports and analyses. First, the MTA reviewed how toll collection should be undertaken. This was accomplished by comparing conventional toll collection and Highway Speed Collection now known as Open Road Tolling (ORT). This report indicated that significant benefits would be derived from ORT. Among these were:

- A reduced number of toll lanes
- Reduced operational costs
- Enhanced safety for the motorist and employees
- Enhanced air quality
- Enhanced customer convenience

Based upon a thorough review of the benefits of ORT the MTA decided to include ORT in any action regarding the York Toll Plaza. Second, the MTA evaluated the potential of One-Way tolling. This report cited the pros and cons of implementing one-way tolling. Based upon the short comings of one-way tolling, the MTA decided to move forward with a plaza that collected tolls in both directions and utilized ORT. Third, the MTA investigated All Electronic Tolling (AET). The report regarding AET presented both the pros and cons of AET. It cited the considerable revenue risk associated with the ability to collect toll revenue in this manner from out of state and out of country travelers. Based

upon the risk issues of AET, the MTA decided that for the foreseeable future it would maintain cash collection and add ORT.

The conclusion that the plaza needed to be replaced was not easily reached. The comparison of cost to repair/rehabilitate vs. replace led to the replacement option. The variance in these costs when taken in the context of long term maintenance, operations and the ability to meet the changing needs of toll collection systems leads to a full replacement option. The assessment of operations concerning less quantifiable values such as the safety of the public and the staff is more difficult. These can be critical business points that are difficult to assess as pure value. A new plaza with ORT designed in accordance with FHWA guidelines will result in a reduction of crashes. These have a value. This value assessment will continue to be reviewed as the process continues to provide the MTA with the best information with which to make a final decision.

Furthermore, the MTA's contract with its bondholders (The Bond Resolution) requires that the Authority maintain and/or replace Turnpike infrastructure based on inspection and recommendation by its GEC in order to protect the collection of toll revenues. These protective covenants are critical to the MTA's cost effective use of revenue bonds to finance capital projects.

9. Why isn't it appropriate to rehabilitate the existing location? This did not seem to be addressed in the current report.

The physical and operational deficiencies of the existing location are well documented in the ESE Report, Section 3, Project Purpose and Need and are summarized above in the answer to question #4. However, the primary focus of the ESE Report was not to articulate why it is not appropriate to rehabilitate the existing location, but rather to consider "what it would take" to overcome the deficiencies of the existing location.

To accomplish this, the GEC developed and analyzed nine different options at the existing location. These options (1, 2, 3, 4a, 4b, 6, 7, 8 and 9), are described in significant detail in the ESE Report, Section 7, Rehabilitate/Reconstruct Feasibility Analysis. In all cases, these descriptions include information regarding how each option either addresses or does not address the various deficiencies as well as the associated impacts and costs.

The final determination about whether it is appropriate to rehabilitate the existing location, cannot be made until a comprehensive comparison can be made between the options advanced at the existing site and the options advanced at alternative sites. This determination will be made in accordance

with the USACE Highway Methodology, which is designed to produce the Least Environmentally Practicable Alternative.

10. Given the settlement in the swamp around the toll plaza area, the roadway has to be repaired regardless of the plaza. Why take more land, cause further environmental damage and incur the double costs of building at a new location and repairing the existing location?

There are two distinct factors that need to be addressed in answering this question, the amount of impacts and the respective construction requirements.

First, it is too early to conclude that the construction of a safe, efficient and modern toll plaza at an alternative location will take more land, cause further environmental damage and incur double the cost than the construction of such a facility at the existing location. In fact, based on previously completed research of alternative sites, it is indeed probable that the options recommended for advancement at the existing site will result in greater environmental and right-of-way impacts than those that emerge from the alternative site investigation. The final determination cannot be made until the alternative site investigation is completed using the smaller toll plaza footprint and the best alternative sites are compared with the best sites at the existing location.

With respect to "double" costs and repairing of the roadway at the existing location; it is important to understand that mainline repair with a toll plaza and mainline repair without a toll plaza are two very different constructions with two very different cost implications. Repair of the mainline incorporating an existing site toll plaza reconstruction would be considerably more expensive, in fact more than double the cost of repairing the mainline if the toll plaza was relocated. One factor that leads to this difference is the construction technique to address settlement of approaches to the existing toll plaza.

For the scenario of relocating the toll plaza and repairing the mainline: following the removal of the existing plaza the hole left from the tunnel would be filled, the approaches would be graded to eliminate the 'bump' that exists today, a uniform depth or thickness of gravel would be constructed and a uniform thickness of pavement would be installed. This construction would allow for the reconstructed mainline to shift and settle uniformly across the poor soil area, similar to any other highway construction.

The scenario of reconstructing the plaza in place, without considering traffic control, and construction staging, will require a significantly different approach to address the settlement. The settlement surrounding the plaza as it occurs

today and has for 40 years would need to be controlled to prevent the abrupt 'bump' at the toll plaza. All of the same steps from repairing mainline due to relocation of the plaza will be performed in addition to advanced soil construction methods being applied. The toll plaza itself would likely be supported by a pier foundation as it is today so the approaches would need to be constructed in a way to minimize settlement, especially near and at the edge of the toll plaza. One way to do this would be through the use of light weight fill. Essentially, the existing gravel and some of the native soils supporting the pavement would be removed and replaced with a light weight fill to reduce the overall weight of the pavement structure and the weight the underlying weak soils must support; less weight - less settlement. While not completely eliminating settlement, this method would drastically reduce it, thus minimizing a number of safety issues with bumpers and vehicle clearance. Although this sounds simple enough, the fact that light weight fill can be 10-20 times more expensive makes the earthwork construction portion of the existing site options considerably more expensive.

Therefore, with the above early indications of fewer environmental impacts and more practicable cost options being available at alternative sites, it is in the MTA's best interest to direct its GEC to resume the investigation so that the environmental and right of way impacts along with the full costs of construction can be learned and a fair comparison can be made between existing site and alternative site options.

a. What is the incremental effect on the environment if the tollbooth location stays the same vs. relocates?

This would be dependent on the site of relocation. Table 8, Comparison Matrix contained in the ESE report, page 58 provides the estimated amount of impact given for the options at or near the existing site. If the plaza were relocated to an alternate site the impacts would be dependent on the conditions at that site. Additionally, if the plaza were to be relocated to an alternate site, the existing site would be removed and some of the wetlands impacted by the existing plaza and approaches could be restored.



# Responses to Second Series of Questions Posed by Think Again October 26, 2009

## 1) Where did the information that the New Hampshire Turnpike Authority is not planning an AET Plaza come from?

The New Hampshire Department of Transportation has publically announced their plans to implement an Open Road Tolling (ORT) system, similar to what the Maine Turnpike Authority has proposed, at the Hampton Toll Plaza. The contract for the Hampton Toll Plaza Open Road Tolling project (#15678) was advertised in June of 2009. Information about the project is readily available on the NHDOT web site at <a href="https://www.nh.gov/dot">www.nh.gov/dot</a>.

We do not know all of the reasons why the NHDOT decided not to pursue AET, but given that their traffic mix is similar to the Maine Turnpike's, their decision seems prudent. As noted in the Existing Site Evaluation (ESE) and in response to Think Again's first round of questions, of the 85 toll facilities in the nation only 5 are AET systems. They are typically roadways dominated by daily commuter traffic and a high concentration of E-ZPass users, usually in excess of 80%. In addition, the overwhelming share of their users live within a specific jurisdiction, making it possible to effectively identify vehicle owners and addresses from license plates, send bills and enforce payment.

The Maine and New Hampshire Turnpikes share none of these characteristics. Both highways serve a very diverse traffic mix, which originates from various states along the eastern seaboard of which less than 60% are E-ZPass users. This means that under an AET system both highways would be required to capture nearly half of their revenue through reciprocal, interstate and province vehicle owner identification and legal enforcement systems that do not exist today and are unlikely be developed to any standard of reliability in the foreseeable future. Failure to effectively collect video tolls from the Maine Turnpike's diverse, multi-state, customer base would necessitate higher tolls for those regular, instate users.

2) You state that the 2007 safety data was available. Why was the 2007 data not used in the report?

The General Engineering Consultant resumed collecting data for the Existing Site Evaluation in June of 2008. At that time, the most recent crash data (summary statistics and detailed crash analysis) available from the Maine Department of Transportation was for the periods 2003-2005 and 2004-2006. The 2005-2007 summary statistics became available later in 2008, but the detailed crash analysis was not. The GEC felt that it was inappropriate to insert the 2005-2007 summary statistics without first reviewing the corresponding detailed crash analysis. It is important to note that the inclusion of the 2005-2007 summary statistics would not have altered the ESE findings and/or recommendations. As noted in our September 3<sup>rd</sup> response to Think Again's first series of questions, the subsequent review of both 2005-2007 and 2006-2008 crash data continues to show unacceptable crash trends at the York Toll Plaza. In fact, the York Toll Plaza has been included on the MaineDOT's list of High Crash Locations every year for the ten year period between 1999–2008.

3) You state that there have been serious injury accidents within ORT plazas since the inception of ORT. How many fatalities and serious injury accidents have occurred at ORT locations since the inception of ORT in the United States?

The statement above was not made by the MTA or its GEC. Nor does it appear in the Existing Site Evaluation. This is not to say that serious crashes have not occurred within an ORT plaza at various locations in the United States. There are a number of ORT facilities in the United States, processing millions of vehicles, so it is entirely possible that some serious crashes have occurred. We have not conducted a study of the number of crashes and fatalities at ORT facilities and are not aware that any such study exists. The value of such a statistic would be of marginal value without additional information about the design and condition of each individual ORT plaza. There are a variety of ORT plaza designs and conditions operating in the country, some engineered to a higher safety standard than others. It is reasonable to expect that those engineered to a higher safety standard would experience fewer crashes. This, of course, explains why the GEC initially dismissed the existing York Toll Plaza as a potential location of a replacement plaza. It failed to meet basic engineering and safety guidelines due to its location near an interchange, on a curve and at the bottom of a hill. As noted in our response to Think Again's first series of questions, the GEC predicts that both the number and severity of crashes will be higher if an ORT is implemented at the existing location of the York Toll, as opposed to an alternate location that would be selected based on its compliance with engineering and safety guidelines.

4) When it is clear that AET will replace ORT, how can the economic and social well being of the citizens of Maine be ignored with expensive short term solutions that take people's land and cause further environmental damage?

The ESE report has an appendix which contains a report prepared by the GEC entitled, All-Electronic Tolling Feasibility Review on the Maine Turnpike. In the report, the GEC concluded that there is no certainty that AET will replace ORT on the Maine Turnpike or on any significant number of toll facilities in the foreseeable future. The report states:

"While there may be theoretical benefits of converting a cash & ETC [electronic toll collection] facility to AET, the significant uncertainty behind the business costs associated with AET coupled with the unique and quantified characteristics of the Maine Turnpike make the consideration of AET for the York Toll Plaza replacement not a feasible option at this point in time or in the 20 year planning horizon. The lack of industry data for similar roadways, the uncertainty relative to how customers will respond to the changes in payment methods and the uncertainty relative to revenue recovery potential for violations poses too broad a range of potential outcomes. These include significant risks to net revenue required to operate the roadway. Greater certainty around the potential impacts to toll operating costs and revenue impacts would be necessary to reduce the range of risks to an acceptable level of the further consideration of AET. Therefore, given the lack of comparable industry information to date and the revenue risk associated with uncertainties with patron behavior, HNTB does not recommend AET for the York Toll Plaza for this time, nor do we anticipate, given the significant risk described herein, that AET would be prudent for York Toll within the next 20 years."

The Maine Turnpike's enabling Legislation states: "The economic and social well being of the citizens of the State requires that the transportation system be developed in a comprehensive manner that depends upon the safety, efficiency and modern functional state of the turnpike." To fulfill this legislative charge, the MTA was authorized to collect tolls, issue revenue bonds and to maintain its own debt. The prudent exercise of these responsibilities over the last 62 years has earned the MTA credit ratings that are among the highest in the nation for toll facilities. These outstanding credit ratings allow the MTA to finance capital projects at lower interest rates and thus maintain the highway at a high standard while keeping toll rates as low as possible. This successful formula is dependent on the MTA's ability to effectively collect toll revenue, totaling more than \$80 million per year, from all of its customers. We do not believe that citizens of Maine would be well served by the adoption of a toll collection system that risked the MTA's longstanding and advantageous financial position by threatening its ability to effectively collect toll revenues. Nor would they be well-served by the adoption of a system about which little is known and what is known suggests that it is not a suitable system to efficiently collect revenue from the Maine Turnpike's diverse mix of traffic. Finally, we do not believe that it would be financially responsible to adopt a system that is dependent on reciprocal interstate and province vehicle owner identification and enforcement systems that do not exist today and are uncertain to exist to any standard of reliability in the foreseeable future.

While AET may not be a feasible or financially responsible option for the Maine Turnpike in the foreseeable future, the MTA is planning to introduce Open Road Tolling (ORT) at the York Toll Plaza and at other suitable mainline toll plazas. ORT, also known as Highway Speed Tolling, will enable E-ZPass users to pay their tolls by simply passing beneath a sensor at normal highway speeds. The system would continue to accommodate cash paying customers, who would briefly depart the mainline of the highway to pay at a traditional toll plaza. An ORT system

on the Maine Turnpike would provide the efficiency, convenience and environmental benefits of highway speed tolling for E-ZPass users, without compromising the ability to effectively collect revenue from non-E-ZPass users.

5) Where is the detailed study of the cost comparison between AET and ORT at the York Toll Plaza?

The Maine Turnpike Southern Toll Plaza Initial All-Electronic Tolling Feasibility Review (AET Feasibility Review) was included as Appendix E of the Existing Site Evaluation (ESE). This 22-page document examines capital, maintenance and operations costs, as well as revenue impacts and risks associated with AET. It concludes that implementation of an AET system on the Maine Turnpike could significantly compromise the Authority's ability to collect revenue and thus presented irresponsible and unacceptable financial risks. This conclusion made continued cost comparisons between AET and ORT unnecessary.

6) How did you generate the \$17 million per year in uncollectible revenue at the York Plaza with AET?

The \$17 million per year potential revenue loss figure is also explained in the AET Feasibility Review, pages 11-22. As noted in the report, the \$17 million figure represented the pessimistic expectation. The optimistic expectation was a loss of \$1.5 million per year. The report states: "The reality of the circumstance is that it is very unlikely that the optimistic or the pessimistic scenario will occur. It is more likely that the revenue leakage will be somewhere in the middle. This value however is significant and poses a grave threat to the Maine Turnpike."

7) What are the collection rates where AET has already been employed in other places?

We do not have information regarding the collection rates of other AET facilities. Such information would be of marginal value because of the small number of AET systems (5 of 85) have been installed on highways with the characteristics necessary for AET to succeed. As explained earlier, the Maine Turnpike shares none of these characteristics. Under an AET system, the MTA would be required to capture a substantial share of its revenue through an interstate and province vehicle owner identification and enforcement system that does not exist today and is uncertain to be developed to any standard of reliability in the foreseeable future. For these reasons, it would not be instructive to apply the experiences of the few existing AET facilities to the Maine Turnpike.

8) What are the collection rates at the York Toll Plaza? (We heard about the Dover, NH man who went thru the York Plaza 90 times without paying.)

As noted in the AET Feasibility Review, page 17, the Maine Turnpike's toll collection rate is currently estimated to be 98.3% (1.7% revenue leakage). The Maine Turnpike Authority's enforcement system has proven to be effective. The toll violator referenced in your question was arrested by Maine State Troopers, pled guilty to charges, was required to pay restitution plus fines and served time in jail.

## APPENDIX K YORK TOLL PLAZA REPLACEMENT TECHNICAL REPORT IN RESPONSE TO MAINE LD534

## MAINE TURNPIKE AUTHORITY



## York Toll Plaza Replacement Technical Report In Response to Maine LD534

Prepared for: Maine Turnpike Authority

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Date: February 15, 2008



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#### **SECTION 1 - EXECUTIVE SUMMARY**

In 2007, the Maine Legislature passed LD 534, A Resolve, Directing the Maine Turnpike Authority to Study the Relocation of the York Toll Booth. The Maine Turnpike Authority (MTA) has prepared this technical report in response to LD 534. The report presents the existing conditions and deficiencies of the York Toll Plaza, the industry standards for design and construction of toll plazas, the public comments on its rehabilitation and relocation, and a final recommendation for addressing the plaza's deficiencies.

Situated seven miles from the New Hampshire border, the 17 lane York Toll Plaza is considered by many interstate travelers to be the "gateway" to Maine. The toll plaza began as a temporary 11 lane structure constructed on the Maine Turnpike in York, Maine in 1969 as part of the realignment of Interstate 95 and the construction of the Piscataqua River Bridge. Numerous maintenance and rehabilitation projects have been constructed to improve the capacity of the plaza, to cope with its aging components, and to provide safety for both the traveling public and toll staff. However, the York Toll Plaza's life expectancy has passed and it is no longer able to provide adequate safety or meet future traffic demands.

There are a number of operational issues related to the plaza's location that affect both capacity and the safety of patrons and staff. 1) The plaza is located 500'-700' from the Exit 7 Interchange causing unsafe merging and weaving of traffic within the plaza limits. This also leads to an inefficient use of toll lanes. 2) The plaza is on a horizontal curve. Southbound traffic tends to drift to the outside of the curve, reducing utilization of all tollbooths, i.e. left side lanes become over-utilized and right side lanes underutilized. The curve also blocks sight to all southbound lanes/booths until approximately 1500' away. This leads to inefficient decisions and unsafe last second lane changes. 3) The plaza is at the low point of a hill. This creates a safety concern due to the potential of heavy vehicles losing their brakes and striking the plaza or stopped traffic. In addition the hill leads to heavy engine braking noise southbound and heavy acceleration noise northbound as commercial vehicles approach and depart the plaza. 4) Last, the plaza is approximately 2200' from the Chases Pond Road bridge. This limits sight distance for northbound traffic to the merging on-ramp traffic, backed-up traffic and toll booths. A driver's line of sight is also blocked by roadway signage. All four of these characteristics, nearby interchange, roadway curve, bottom of a hill and nearby bridge, contribute to increased crash potential and decreased operational performance.

In addition to these location related deficiencies, there are numerous infrastructure deficiencies that also must be addressed. 1) The original tollbooth structure was designed in the 1960s and is deficient by today's standards including insufficient space for collector activities and tolling equipment as well as very narrow lanes. 2) Current standards for toll booths incorporate a double concrete bumper to provide safety for the toll collector and driver by redirecting any missteered vehicle back into a lane to versus striking a toll booth. York's single bumper design does not adequately protect staff or turnpike patrons and more importantly the existing bumpers have almost completely disappeared due to sinking into the poor soil. 3) The toll collectors' access tunnel beneath the booths is in poor condition and in need of rehabilitation. The tunnel is too narrow due to addition of tolling electronics and modern utilities. The concrete tunnel experiences significant water infiltration due to its age and the many utility penetrations. 4) The structural supports for the existing canopy are at capacity and cannot feasibly handle additional signing including the more modern signs which are larger and heavier than existing. 5) The original plaza was built in an area with poor subsurface soil conditions, mainly consisting of

compressible clay. Due to these soil conditions, the plaza tunnel, booths and canopy were constructed on H-piles to prevent settlement of the entire structure. However, the roadway approaches to the plaza were not pile-supported. As a result, the approaches (and bumpers) have and continue to settle as the clay soil consolidates. The noticeable slope approaching and leaving the plaza is a result of the roadway settling away from the pile-supported plaza. The age of the plaza, the outmoded conditions of the existing tollbooths, canopy, tunnel, and the poor soil conditions all contribute to the overall poor condition and performance of the plaza. These deficiencies contribute to classifying the existing infrastructure as functionally obsolete.

In addition to location and infrastructure deficiencies, the York Toll Plaza will not be able to service future traffic demands. Today, the plaza processes over 16 million vehicles per year up from five million vehicles in 1970. With total traffic expected to grow approximately 2.0% per year over the next 20 years, capacity improvements are needed to efficiently and safely process this ever increasing traffic. The MTA has researched various tolling technologies with the goal to identify a more efficient means of tolling. Based on the percentages of cash and E-ZPass customers, the projected traffic increases, and the amount of infrequent users from out-of-state, the tolling technology that best serves the MTA is Highway Speed Tolling. This allows E-ZPass users to pay their toll electronically while driving thru the plaza at normal highway speeds of 55-65 mph. Cash customers will exit from mainline to pay their toll at traditional cash booths, then accelerate and merge back into the mainline with E-ZPass customers. Following the research, MTA made a decision to implement Highway Speed Tolling at the Southern Toll Plaza as well as at other mainline plazas.

From the evaluation, and in conjunction with the plaza's accident history, the York Toll Plaza is operationally inefficient, structurally deficient, is located such that these conditions compromise overall staff and patron safety. The York Toll Plaza is in need of major rehabilitation or replacement to improve operations and meet current design guidelines. To determine the most effective course of action that addresses immediate and future needs, a comprehensive evaluation of the following five options was completed.

#### Option 1: No Build (Leave Plaza in Existing Condition and Tolling Arrangement)

Option 1 does not satisfy any of York Toll Plaza's safety or operational needs, present or future. This option leaves the Plaza requiring extensive ongoing maintenance. **This Option is dismissed from further consideration.** 

#### Option 2: Infrastructure Upgrade with No Additional Capacity

Option 2 addresses only the structural deficiencies of the existing infrastructure. This option does not provide the needed additional capacity, does not address the location deficiencies, does not meet current industry design standards and will not address many safety or operational issues for Turnpike patrons and staff. The cost to provide this option would be lost without benefit as it would not remedy any of the truly needed improvements. **This Option is dismissed from further consideration.** 

Option 3: Upgrade Existing Site with Conventional Tolling and Increased Capacity

Option 3 increases capacity and upgrades the infrastructure but does not address the safety and operational concerns associated with the current plaza location. The cost of this option would be more than two-thirds the cost of the relocated option but would provide only marginal benefit. In addition, there is no opportunity for implementing modern Highway Speed Lanes with this option. **This Option is dismissed from further consideration.** 

#### Option 4: Upgrade Existing Site with Highway Speed Tolling and Increased Capacity

Option 4 marginally improves traffic capacity and ETC processing time but fails to address the safety concerns associated with the current plaza location. Full efficiency of Highway Speed Tolling will not be realized due to the location on a curve and near a hill. A costly interchange reconfiguration and reconstruction will be necessary resulting in confusing and complicated traffic patterns. The cost of this option is similar to that of the full build option but provides far less benefit. To effect additional safety benefits in traffic movements would require an interchange reconstruction that is far greater than considered here, likely more than doubling the cost of this option. **This Option is dismissed from further consideration.** 

#### Option 5: Relocate Plaza to Alternate Location with Highway Speed Tolling

Option 5 will result in a toll plaza that 1) operates safely for both Turnpike patrons and staff, 2) provides adequate capacity for current and future traffic demands, 3) meets today's industry standards for plaza location and infrastructure needs, and 4) implements modern technology to efficiently process Turnpike traffic with Highway Speed Tolling lanes. This Option is the most cost effective way to meet York Toll Plaza's safety and operational needs and will allow the York Toll Plaza to be a prominent "gateway" to the State of Maine. **This Option is the only reasonable option and is the choice the MTA will pursue.** 

#### Recommendation:

The results of the alternatives analysis support the MTA selecting and pursuing Option 5; constructing a new toll plaza, with Highway Speed Tolling, in a new location. Constructing a toll plaza in a new location will result in 1) safer operations for both Turnpike patrons and staff, 2) adequate capacity for current and future traffic demands, 3) a plaza that meets industry design standards for layout and operations, and 4) the ability to implement modern and more efficient Highway Speed Lanes. **None of the other four options are able to provide all of these features.** 

Option 5 is the most cost effective way to meet York Toll Plaza's needs and it will allow the York Toll Plaza to be a prominent "gateway" to the State of Maine. Constructing a new plaza, with Highway Speed Tolling, at a new location is the most prudent direction for addressing existing safety and operational issues and future needs of a Southern Toll Plaza and gives the Maine Turnpike Authority a sound investment in a facility that will provide the public with a safe, efficient, and modern toll plaza today and into the future.

The Maine Turnpike Authority will continue with the York Toll Plaza Replacement project by pursuing the identification of a new location for the plaza that meets national engineering standards and that will accommodate Highway Speed Tolling.

#### **SECTION 2 - INTRODUCTION**

In 2007, the Maine Legislature passed LD 534, A Resolve, Directing the Maine Turnpike Authority to Study the Relocation of the York Toll Booth. Section 1 of this Resolve states that "the Maine Turnpike Authority may not relocate the York Toll Booth until the Authority has had the opportunity to study the need for and the expense of replacing a functional toll booth. The Authority shall gather information on various approaches to address the issue of relocating the toll booth. In gathering the information, the Authority shall hold informational sessions for discussions with interested parties." Section 2 states that "the Maine Turnpike Authority shall submit a report to the Joint Standing Committee on Transportation no later than December 15, 2007. This report must include recommendations on whether to relocate the York Toll Booth."

The purpose of this technical report is to respond to the requirements of LD 534. This report will document the feasibility of the following Options with regard to the present and/or a new toll plaza in Southern Maine:

Option 1: No Build (Leave Plaza in Existing Condition and Tolling Arrangement)

Option 2: Infrastructure Upgrade with No Additional Capacity

Option 3: Upgrade Existing Site with Conventional Tolling and Increased Capacity

Option 4: Upgrade Existing Site with Highway Speed Tolling and Increased Capacity

Option 5: Relocate Plaza to Alternate Location with Highway Speed Tolling

Situated seven miles from the New Hampshire border, the 17 lane York Toll Plaza is considered by many interstate travelers to be the "gateway" to Maine. The plaza processes over 15 million vehicles per year which equates to \$34 million in revenue (39% of total Maine Turnpike revenue). Truck traffic accounts for nearly 15% of the plaza's use. Today, approximately 50% of total vehicles, and 80% of truck traffic, utilize E-ZPass, the Maine Turnpike's form of Electronic Toll Collection (ETC). It is anticipated that total ETC usage will grow to between 75% - 80% by year 2020. The plaza processes a nearly equal blend of traffic from in-state and out-of-state travelers. Many of the in-state travelers are southern Maine commuters. Recreational traffic increases dramatically during the summer months (June through September), with traffic peaking northbound on Friday evenings and southbound on Sunday afternoons. Two-way traffic through the plaza peaks during the mid-day hours on Saturdays.

The existing toll plaza began as an 11 lane temporary structure constructed on the Maine Turnpike in York, Maine in 1969. During this time period, the US Department of Transportation was trying to phase out toll facilities. However, in more recent years, Federal Legislation tone has changed because of the ever present challenges in funding the nation's transportation system. In the early 1980's the Maine Legislature decided to continue the use of tolls to fund the operation and maintenance of the Turnpike as well as to fund widening, modernization, and the Interchange Program. Numerous maintenance and rehabilitation projects have been constructed to improve the capacity of the plaza and to maintain its aging components.

A few of the major modernizations are described here. In the late 1970's, a two lane plaza expansion was constructed to respond to increased traffic demand. In 1997, the plaza was modified to incorporate electronic toll collection to keep pace with changing toll technology. In 1999 two dedicated ETC lanes were added to form the current configuration of 17 lanes in response to increased traffic and increased use of ETC. In 2001, the canopy over the original lanes was extended to cover all but the exterior dedicated ETC lanes. In 2005, the plaza was

included in the system-wide conversion from TransPass to E-ZPass. As traffic demand continued to grow, vehicle type, size and speeds changed, and tolling technology evolved; it became clear that the majority of these modifications were temporary fixes to improve capacity and extend the plaza's useful life as long as possible.

Based on the evaluation in this report and in conjunction with the plaza's accident history and operational performance, it is clear that the York Toll Plaza is not completely aligned with current practices and design guidelines and is in need of major rehabilitation or replacement to improve operations and meet these guidelines. Current deficiencies impact safety of both Turnpike staff and patrons and increase the overall operation and maintenance costs. Capacity improvements are also needed to efficiently and safely process the ever increasing traffic volumes at a reasonable level of service. While the addition of tolling lanes and ETC have improved the plaza's capacity, additional toll lanes or highway speed toll lanes are needed to meet the future traffic volumes. Similarly, while recent infrastructure upgrades have improved the overall operation for both patrons and employees, these upgrades have only been considered short-term improvements and have met only a portion of the immediate needs. The MTA decided in 2001 that the future needs of the entire plaza should be addressed. A more comprehensive evaluation was necessary to determine immediate and future needs, including what type of modifications would be required to bring the plaza up to current design standards and best practices, and to determine why a new plaza should be built.

This report compares and contrasts various levels of rehabilitation and reconstruction that address some or all of these deficiencies. As part of improving the plaza operations, the report also documents benefits and shortcomings of various tolling strategies including conventional toll booths, electronic toll collection and highway speed tolling. To begin this discussion, the following is a summary of current design guidelines followed by conditions of the existing plaza.

#### SECTION 3 - TOLL PLAZA DESIGN GUIDELINES FOR MAINLINE LOCATIONS

It is worthy to note, that the existing York Toll Plaza was constructed many years prior to the development of any formal national design guidelines pertaining to toll plazas. Responding to similar situations of the many tolling agencies across the country, the Federal Highway Administration (FHWA) completed a lengthy research project in 2006 aimed at consolidating the most current best practices for the design and construction of toll plazas. A report titled "State of the Practice and Recommendations on Traffic Control Strategies at Toll Plazas, (2006)" was published. The purpose and focus of this report was to develop guidelines for designing and implementing traffic control strategies and devices at toll plazas that, for example, inform drivers which lanes to use for specific methods of payment, reduce speed variance, discourage lane changing and properly install equipment and devices.

In addition the FHWA report for current toll plaza design guidance, the Maine Turnpike utilized the following two references for guidance on how a toll plaza should interface with a Turnpike mainline and adjacent roadways: 1) "Geometric Design of Highways and Streets," (2004) American Association of State Highway and Transportation Officials (AASHTO); and, 2) "Freeway and Interchange Geometric Design Handbook," (2005) Institute of Transportation Engineers (ITE).

The following recommendations, regarding the location of a toll plaza, are based on the FHWA Guidelines unless otherwise noted:

- Locate toll plaza on a horizontal straight section with no curves. Placing a toll plaza on a curve 1.) reduces driver sight distance, 2.) causes additional distractions to drivers thereby increasing potential for accidents, 3.) reduces plaza operational efficiency as some booth lanes will be over utilized and some underutilized, and 4.) may create engineering challenges relating to roadway cross slopes and super elevation needs. Locating a toll plaza on a straight section of roadway should result in improved sight distance, driver awareness, and facility safety when compared to a location on a horizontal curve.
- Locate the toll plaza on a roadway high point. Placing a toll plaza at the crest of a hill will provide sight distance advantages and plaza operational benefits as the approach upgrade will aide in slowing vehicles down while the departure downgrade will aide in accelerating vehicles. FHWA Studies have been done to determine acceptable levels of grade approaching and departing a toll plaza. Grades 3.0% and steeper have an adverse affect on the performance of commercial vehicles and grades less than 0.5% create drainage problems and possible icy conditions in the winter. Therefore, grades approaching and departing the toll plaza should be within the range of 0.5% to 2.0%.
- Provide adequate decision sight distance (DSD) in advance of the toll plaza. This distance is comprised of two individual distances. DSD, as defined by AASHTO, is the distance needed for a driver 1.) to detect an unexpected or otherwise difficult to perceive information source or condition in the roadway environment that may be visually cluttered, 2.) recognize the condition or its potential threat, 3.) select an appropriate speed and path, and 4.) initiate and complete the maneuver safely and efficiently. For highway speed tolling (HST), one DSD requirement is to provide 1,500 ft sight distance before the split point between highway speed and conventional plaza lanes. This distance assumes vehicles are traveling at 70 mph and advance signing is provided in accordance with FHWA Guidelines. The second DSD

requirement for HST and the DSD requirement for conventional toll booths is to provide adequate sight distance from the split point to the toll plaza or approximately 2,000 feet. The driver should be able to see the toll plaza at the point of split between highway speed lanes and conventional plaza lanes.

- Provide 3,500 ft separation between toll plaza and overhead structures. This distance is based on previous DSD criteria defined. Ideally, the driver should have unobstructed views of the split point and plaza, thereby improving facility safety. This requirement will also reduce or eliminate potential impacts to existing overhead structures.
- Provide one mile (5,280 ft) minimum separation between toll plaza and interchanges. A toll plaza placed near an interchange may create traffic weaving issues, signing difficulty, a wide range of vehicle speeds and general driver confusion.

#### SECTION 4 - CONDITION OF EXISTING YORK TOLL PLAZA

The York Toll Plaza was constructed on the Maine Turnpike at the current location in 1969. As mentioned in Section 2 Introduction, a number of modifications, rehabilitations and alterations have been implemented since then to increase capacity, improve operations and keep pace with the ever changing traffic stream. However, the plaza is now functionally obsolete. The age of the plaza, the outmoded conditions of the existing tollbooths, canopy, tunnel, and poor soil conditions all contribute to the overall poor condition and performance of the plaza. The proximity to the Exit 7 Interchange and improper geometry compromise staff and motorist safety, and further render the existing facility inadequate. Details of these deficiencies are summarized below. The insufficient capacity York suffers is detailed in Section 6.

#### A. Horizontal Geometry

The FHWA Guidelines state that a toll plaza should be located on a straight section of roadway and not on a horizontal curve. The York Toll Plaza was built on a horizontal curve. As detailed under the Sight Distance heading, the combination of the existing horizontal and vertical curves reduces the available sight distance to the plaza. Limiting sight distance in this way affects the lane choice decision a driver must make and forces the driver to make that decision in a much shorter period of time. This becomes critical in high volume periods when lane distribution plays a larger role in overall plaza capacity. The horizontal curve also reduces the ability of this location to support Highway Speed Tolling. This will be discussed in more detail later in the report. The curved roadway also has an operational impact on the plaza, specifically in the southbound direction. Vehicles approaching southbound make a sweeping right turn approaching the plaza. This movement creates a tendency for southbound vehicles to travel through toll lanes on the outside of the curve (interior of the plaza) and reduces utilization of the tollbooths on the inside of the curve. Traffic that is not uniformly distributed in the plaza reduces operational efficiency, with some lanes over-utilized and some underutilized. While a certain amount of non-uniform usage is common at plazas, the existing roadway curve exacerbates the skewed distribution.

#### B. Vertical Geometry

The FHWA Guidelines recommend toll plazas be located on a crest vertical curve. Locating the plaza on a high point will increase sight distance and provide operational benefits, as the approach up-grade will aide in slowing vehicles and the departure down-grade will aide in accelerating vehicles.

The existing York Toll Plaza is located at the low point of a hill that begins just north of the plaza. This vertical geometry presents undesirable conditions for traffic departing northbound and approaching southbound. The northbound impact is primarily operational in nature, since the roadway north of the plaza includes a significant grade of 4.72% that impacts acceleration for departing vehicles, especially trucks. There is currently a truck climbing lane in this area to mitigate this condition. The southbound approach represents a concern from a safety perspective since it is on the downgrade of 4.72%. This creates a condition where vehicles (especially trucks) must brake sooner to compensate for the downgrade in addition to the significant speed reduction required in the plaza area. While the Maine Turnpike has a rule prohibiting excessive noises, this condition also contributes to some truck drivers using noisy engine brakes to assist

with the deceleration. An additional safety concern associated with this down grade is the potential for vehicles which have lost their brakes to strike the plaza.

#### C. Sight Distance

The FHWA Guidelines imply that toll plazas should be sited such that motorists will be able to see the plaza while driving at posted speeds with adequate stopping and decision sight distance. Bridges and vertical curves can negatively impact the sight distance. There are two crest vertical curves and a horizontal curve that limit decision sight distance to the plaza for Southbound traffic, and the Chase's Pond Road bridge limits these distances for Northbound traffic. As mentioned earlier, limiting sight distance affects the decisions drivers make as well as forces them to make those decisions in a much quicker time. During high volume periods, less informed decisions can lead to poor operation and an increased risk of crashes.



Figure 1 Northbound Sight Distance
Bridge and Horizontal Curve Negatively Impact Sight Distance



Figure 2 Southbound Sight Distance
Horizontal Curve and Down Gradient Are Not Desirable Due To Safety and Operational Concerns

#### D. Proximity to Overhead Structures

The proximity of the plaza to the Chase's Pond Road bridge limits the available sight distance as seen in Figure 1 Northbound Sight Distance. Desirably, there should be a 3,500 ft separation between the plaza and overhead structures. This distance is based on previously described components of Toll Plaza Decision Sight Distance in Section 3. Ideally, the driver should have unobstructed views of the split point and plaza thereby improving facility safety. The Chase's Pond Road Bridge, being 2,200 feet south of the existing plaza, and being on a horizontal curve, limit the available sight distance for northbound traffic.

#### E. Proximity to Interchange

The proximity of the Chase's Pond Road Interchange (Exit 7) located immediately south of the toll plaza presents undesirable safety and operational conditions for the plaza from both a traffic weaving and a sight distance perspective. The Federal Highway Administration's (FHWA) recently published "State of the Practice and Recommendations on Traffic Control Strategies at Toll Plazas," recommends a one (1) mile separation between toll plazas and interchanges. The interchange southbound off ramp is less than 1,000 feet from the plaza and the northbound on ramp is less than 500 feet from the plaza. The proximity of these interchange ramps to the plaza creates traffic weaving issues, signing difficulty and driver confusion. The MaineDOT has classified the York Toll Plaza in the northbound direction as a High Crash Location (2003-2005 crash data). This designation is likely a result of the significant weaving that occurs due to the location of the on ramp.



Figure 3 Exit 7 Interchange Ramps South of York Toll Plaza

#### F. Toll Booths and Concrete Bumpers

The original tollbooth structures were designed in the 1960s and are considered deficient by today's standards from a space, layout, protection and systems perspective. The original design did not anticipate the need for additional equipment required by modern technology such as computers and ETC systems. The current booths have limited space for collector activities and

become extremely crowded during peak periods when all lanes are open, requiring one booth to have two attendants serving both directions. Current toll islands are designed for these smaller booths and will not accommodate the larger modern booths as installed at other locations on the Maine Turnpike. Existing heating systems are outdated, take-up more space than modern components and only provide a minimum amount of comfort. Modern booths are assembled with the latest heating and ventilating systems to provide better comfort.

Current standards for toll booths incorporate a double concrete bumper to provide safety for the toll collector and to redirect an errant vehicle into its lane. The bumper is nearly non-existent in Figure 4 compared to a newer bumper in Figure 5. This is due to poor soil conditions in the area which is allowing these bumpers to settle. Soil settlement is discussed in more detail in a following section.



Figure 4 York Toll Booth, Single Bumper and Settled Island



Figure 5 New Gloucester Toll Booth, Double Bumper and Raised Median

#### G. Tunnel

A narrow tunnel is located under the York Toll Plaza to serve as the main passageway for employees to safely access the toll booths and as a utility corridor to and from the individual booths. The tunnel is in poor condition and in need of rehabilitation. The tunnel is located in an area of high groundwater and experiences significant water infiltration. The tunnel ceiling has numerous cracks and utility penetrations which also allow for the infiltration of surface water into the tunnel. From a safety perspective, having water in the tunnel is undesirable due to the electrical and communication utilities present, as well as for the Turnpike employees during access to and from the booths. Note the leak stains behind and around the electrical cabinets and data conduits in addition to the significant corrosion to some of these utilities. The majority of these utilities were added to accommodate electronic tolling. These additions have reduced the passage width as well as increased the leaks and safety concerns. Numerous repairs have been completed in the tunnel to mitigate the water infiltration but it remains an ongoing maintenance concern. The extensive costs associated with a comprehensive tunnel repair rival the costs for a new tunnel.



Figure 6 York Tunnel
(Note Leak Stains and Narrow Passageway)



Figure 7 New Gloucester Tunnel

#### H. Canopy

A canopy is located over the toll lanes as seen in Figure 8. The structural supports for the existing canopy are at capacity due to the signage that has been placed on the structure over time. The placement of electronic variable messages signs on the canopy allows staff to change messages such as "Any Vehicle", "E-ZPass", and "Lane Closed". However, the installation of these larger and heavier signs is not feasible due the condition of the existing canopy.



Figure 8 Canopy and Signs at York Plaza

#### I. Soil Conditions

The original plaza was built in an area with poor subsurface soil conditions, mainly consisting of compressible clay. With this site condition recognized in the design, the plaza tunnel, booths and canopy were constructed on foundation piers to prevent settlement of the entire structure due to consolidation of the clay soils. However, the roadway approaches to the plaza were not pier-supported. As a result, the approaches have and continue to settle as the clay soil consolidates. In an effort to mitigate the ongoing settlement of the roadway approaches, the addition of pavement has been routinely necessary. Even with the pavement shimming work, the plaza has a noticeable slope approaching and leaving the plaza, with the roadways settling away from the pier-supported plaza. This can be seen in Figure 9. This approach settlement has created a range of adverse conditions, from low bed tractor trailer striking the concrete slab (See Figure 10 Damaged Concrete Slab at Plaza) to excessive settlement of the approach slabs and protective concrete bumpers that were previously discussed. Vehicles that strike the concrete slab with their trailer bottoms increase potential for vehicle accidents, and settlement of the approach slab and concrete bumpers reduces the ability of the bumpers to absorb vehicle collisions increasing risk to toll plaza staff and patrons. Both conditions result in safety concerns.



Figure 9 Settlement of Approach Slab



Figure 10 Damaged Concrete Slab at Plaza

#### J. Summary of Existing Conditions

To summarize, the existing plaza - including both infrastructure and location - is functionally obsolete. The facility is nearly 40 years old and not conducive to safe operation with today's traffic volumes and speeds. With respect to the FHWA's current Design Guidelines and Best Practices, the plaza's layout and location are non-conforming to many standards. Decision sight distance, proximity to an interchange and bridge and capacity, are all current deficiencies that impact the safety of Turnpike staff and patrons and increase overall operation and maintenance costs. In addition, the proximity to Exit 7, Chase's Pond Road, is exacerbating the plaza's High Accident Location status. The poor soil condition also contributes to the overall inadequate condition of the plaza, safety and operations, and seriously jeopardizes the feasibility of site reuse for a toll plaza. Reuse of the site is discussed in Section 7 Alternatives Analysis.

## **SECTION 5 - TOLL COLLECTION STRATEGIES**

Two general types of toll collection systems are in general use today. One is the "ticket system" where motorists receive a ticket upon entering the system and then surrender the ticket and a cash toll upon exiting the system. The other is the "barrier system" where a set cash toll is charged based on a vehicle's number of axles. The Maine Turnpike currently operates a barrier toll system with electronic toll collection in all toll lanes.

With electronic toll transponders, patrons are not required to stop and pay cash. Electronic tolls can be collected in a traditional stop-and-go cash toll lane as well as through a dedicated ETC lane. ETC in both stop and pay lanes and dedicated ETC lanes requires patrons to slow to a maximum speed of 10 mph while passing through the plaza to ensure the safety of staff as well as their own. With the advent of Highway Speed Tolling (HST), ETC patrons are allowed to travel at higher speeds (55-65 mph). For safe operations, these HST facilities physically separate the ETC and cash paying patrons. ETC patrons remain on the mainline of the highway and cash paying patrons exit to the right to a conventional toll plaza. HST and conventional tolling facilities are further discussed in the Toll Plaza Layout segment of Section 6.

A few toll agencies are now operating toll roadways where no cash tolls are collected. In these instances, all of the tolls are collected electronically either by the use of electronic transponders or video tolling where license plate data is recorded. This type of operation is typically feasible on roadways with extremely high commuter traffic. A cashless toll plaza is not currently feasible for the Maine Turnpike at York due to the current level of ETC usage of 50% and the high number of infrequent drivers.

The Maine Turnpike Authority also studied the concept of collecting tolls at York in only one direction in 2005. One-way tolling essentially involves charging twice the one-way fare in one direction, while making the other direction toll-free. The concept of one-way tolling in this area came to the forefront in August 2003, when New Hampshire's Governor authorized the New Hampshire DOT to conduct a one-way tolling experiment at the Hampton Toll Plaza. One-way tolling trials were conducted in the late summer/fall of 2003 and again during the summer of 2004. However, New Hampshire has not identified permanent plans to convert Hampton Toll Plaza to one-way tolling.

The Maine Turnpike Authority voted to cease further consideration of a one-way toll at the York Plaza based on the following findings:

- Loss in Revenue. Implementation of one-way tolling is anticipated to result in a net revenue loss of approximately \$2.0 million dollars per year.
- Local Diversion/Traffic Impacts. The average rate of diversion by implementing one-way tolling is anticipated to be 7.0% or roughly 1,600 vehicles for an average day in 2007 shifting to local roads. (Present diversion rate is 1% 2%)
- *Toll Opportunity*. Doubling the toll at York in one direction may limit the ability to effectively increase toll rates in the future.

## SECTION 6 - TOLL PLAZA CAPACITY, SIZING AND LAYOUT

## A. Toll Plaza Capacity

A toll plaza should have adequate capacity to safely and effectively process the anticipated traffic without excessive queues and delays. However, unlike roadways and intersections which have national standards addressing capacity, no such standards exist for toll plazas. Each toll agency typically has its own goal as to adequate capacity. The Maine Turnpike Authority's goal is to have a toll plaza meet two objectives throughout its design horizon of 20 years. The first objective is to keep average delays during the peak hour to approximately one minute or less. The second objective is to keep average queues during the peak hour to 300' or less.

The operations of the existing plaza from 2007 to the design year of 2030 have been evaluated by comparing projected busiest traffic volumes with the capacity of the lane configuration. Northbound and southbound were analyzed separately.

## 1. Northbound Analysis

The Northbound plaza does not reach its capacity throughout the design horizon of the plaza. However, experience has shown that queuing can be significant when a plaza exceeds 90% of its capacity. Therefore, the NB plaza as currently configured has the potential to experience significant design-hour queuing in the next 20+ years.

In order to remain below capacity, it is critical to periodically alter the configuration of the plaza. Between 2007 and 2024, it is anticipated the E-ZPass volumes will double while cashpaying volumes decline by 25%. Therefore, over time, cash lanes need to be converted to E-ZPass lanes in order to adequately serve the rapidly growing volume of E-ZPass patrons. As can be seen in Table 1, the northbound plaza exceeds 90% capacity now and in the design year regardless of how the existing nine (9) lanes are configured.

**Table 1 Forecasted Northbound Capacity of Existing Plaza** 

Year	Design-Ho	ur Volume	Lane	Configur	ation	% Capacity
i eai	Cash	E-ZPass	Cash	Tandem	E-Z	% Capacity
2007	1,979	2,187	5	2	2	92.6%
2008	1,947	2,302	5	2	2	92.6%
2009	1,915	2,419	5	2	2	92.5%
2010	1,883	2,538	5	2	2	92.6%
2011	1,851	2,658	5	2	2	92.5%
2012	1,819	2,780	5	2	2	92.6%
2013	1,787	2,904	5	2	2	92.6%
2014	1,756	3,029	5	2	2	92.7%
2015	1,725	3,156	5	2	2	92.8%
2016	1,693	3,285	5	2	2	92.9%
2017	1,663	3,415	5	2	2	93.0%
2018	1,632	3,547	5	2	2	93.2%
2019	1,603	3,680	5	2	2	93.4%
2020	1,575	3,814	5	2	2	93.7%
2021	1,547	3,950	5	2	2	93.9%
2022	1,519	4,087	5	2	2	94.2%
2023	1,493	4,226	5	2	2	94.5%
2024	1,468	4,365	4	2	3	95.0%
2025	1,444	4,506	4	2	3	95.4%
2026	1,418	4,651	4	2	3	95.8%
2027	1,390	4,800	4	2	3	96.2%
2028	1,362	4,952	3	2	4	96.6%
2029	1,337	5,103	3	2	4	97.1%
2030	1,314	5,255	3	2	4	97.5%

#### 2. Southbound Analysis

Unlike the northbound plaza, the southbound plaza is over-capacity throughout the 23-year analysis period regardless of how the existing lanes are configured, as seen in Table 2. The SB plaza has the potential to experience significant design-hour queuing in each of the next 20+ years.

**Table 2 Forecasted Southbound Capacity of Existing Plaza** 

Year	Design-Ho	ur Volume	Lane	Configur	ation	% Canacity
i ear	Cash	E-ZPass	Cash	Tandem	E-Z	% Capacity
2007	2,330	1,906	5	2	2	103.2%
2008	2,300	2,021	5	2	2	102.4%
2009	2,269	2,138	4	2	3	101.7%
2010	2,239	2,256	4	2	3	102.2%
2011	2,209	2,376	4	2	3	104.1%
2012	2,179	2,498	4	2	3	105.4%
2013	2,148	2,622	4	2	3	105.1%
2014	2,119	2,747	4	2	3	104.4%
2015	2,089	2,874	4	2	3	103.7%
2016	2,059	3,003	4	2	3	108.7%
2017	2,030	3,134	3	2	4	108.0%
2018	2,001	3,266	3	2	4	107.3%
2019	1,972	3,400	3	2	4	106.6%
2020	1,945	3,535	3	2	4	106.0%
2021	1,917	3,672	3	2	4	105.4%
2022	1,891	3,810	3	2	4	104.8%
2023	1,866	3,949	3	2	4	109.3%
2024	1,842	4,090	2	2	5	108.7%
2025	1,817	4,233	2	2	5	108.1%
2026	1,792	4,379	2	2	5	107.5%
2027	1,765	4,530	2	2	5	106.9%
2028	1,736	4,684	2	2	5	106.4%
2029	1,705	4,844	2	2	5	105.8%
2030	1,673	5,007	2	2	5	105.2%

# 3. Temporary Measures to Increase Capacity

Given the capacity constraints of the existing York Toll Plaza and the ever changing directional demand, the three middle lanes have been made reversible; i.e., the lanes can be operated for either northbound or southbound traffic depending on need (Note: these lanes are always on the left for approaching traffic; see the three lane signs to the left of the E-ZPass sign in Figure 8.) This introduces safety concerns and creates a situation that is contrary to the industry standard of locating dedicated ETC lanes on the far left side of available toll lanes; e.g., one or more (reversible) cash lane may be to the left of a dedicated

ETC lane. Slow speed ETC patrons now must travel between stopped traffic on both sides of them

To meet some of this increasing demand, the Authority has implemented operation of tandem booths. This is a temporary measure until additional capacity can be added by constructing additional temporary booths in line with permanent booths for cash collection. The use of tandem booths requires a flagger to direct drivers into the lane and two toll collectors per lane. This is confusing for the Turnpike patron due to their unfamiliarity with the practice and only results in an additional capacity of 30%, or approximately 100 vehicles per hour. In addition, their use presents accountability concerns relative to toll collector audits. Therefore, due to safety concerns of the flagger operating in the toll lanes, patron confusion, and accountability concerns, the extensive use of tandem booths to address long-term capacity needs is not desirable.

Constructing additional booths for cash paying patrons would require significant widening of the approach and departure zones, relocation of the utility building and significant wetland impacts. In addition, the required widening would have a major impact to the existing interchange located to the south. However, even with the additional lanes, these improvements would solely provide increased capacity to the plaza and would not address any of the operational and safety deficiencies associated with the existing plaza. These deficiencies are further discussed in the following sections.

The York Toll Plaza requires additional capacity. In its current configuration, the northbound side of the plaza will operate at near-capacity levels during peak periods for the next 23 years, with significant queues and delay, while the southbound side is already inadequate for the design-hour demand and experiences lengthy queues and delay. Moreover, in order for the existing plaza to cope with future traffic conditions, the MTA will need to (a) continually modify the lane configuration by adding more ETC dedicated lanes; (b) continue to operate tandem tollbooths (two booths in parallel in a toll lane) during peak periods; and, (c) add additional booths for cash toll collection. Both (a) and (b) are undesirable from a safety and operational perspective and (c) is undesirable due to the costs; all three fail to improve the safety and operational issues associated with sight distance, alignment, plaza settlement and interchange weaving.

## B. Toll Plaza Sizing

The process of developing an appropriately-sized toll plaza for the Maine Turnpike is described below:

<u>Step 1</u> – Develop Design-Hour Volumes (DHV's). The Maine Turnpike Authority is using the absolute highest hour due to the importance of this gateway toll plaza.

<u>Step 2</u> – Develop traffic projections. In order to evaluate toll plaza operations throughout the design horizon of the toll plaza, it is necessary to estimate the extent to which design-hour traffic will grow over time. At the York Toll Plaza, historical data suggests that design-hour traffic will grow approximately 2.0% per year over the next 20 years.

<u>Step 3</u> – Identify payment types. In order to properly analyze a toll plaza, it is critical to understand the peak-hour split between cash-paying patrons and E-ZPass patrons. Generally

speaking, the efficiency of a given toll plaza increases as the percentage of E-ZPass patrons increases. In 2007, approximately 50% of the peak-hour patrons at the York Toll Plaza had an E-ZPass. It is also necessary to project how the share of E-ZPass patrons will change over time. Experience has shown that the share of E-ZPass patrons grows by at least 1% - 2% per year. At the York Toll Plaza, peak-hour usage of electronic toll collection has grown from about 10% in 1997 to roughly 50% in 2007.

The end result of Steps 2 and 3 is an estimate of the number of peak-hour patrons (both cash and E-ZPass) passing through the toll plaza during each year of the toll plaza's design horizon.

<u>Step 4</u> – Perform initial plaza sizing and configuration. Based on the volumes and payment types developed in Steps 3 and 4, it is possible to develop an initial estimate of the appropriate toll plaza size. At the York Toll Plaza, the following operating standards were used to determine plaza size:

- Patrons with an E-ZPass proceed through a conventional toll lane at a rate of 1,100 vehicles per hour (vph).
- Patrons with an E-ZPass proceed through a highway-speed toll lane at a rate of 1,800 vph.
- Patrons paying cash pass through a conventional toll lane at a rate of 289 vph.
- The end result of this step is to identify the total number of lanes (both cash and dedicated E-ZPass) required to handle the peak-hour volumes

<u>Step 5</u> – Test via simulation. After estimating the appropriate size of the toll plaza, the performance of the proposed size is simulated in VISSIM computer model. The simulation serves two important purposes:

- Provides a visual illustration of the performance of the plaza, providing qualitative feedback concerning the performance of the plaza; and,
- Provides information on queues and delays at the plaza, providing quantitative feedback as well.

Table 3 summarizes the required lane configuration for plaza sizing for each of the five (5) options that are considered in Section 7 Alternatives Analysis. A complete traffic forecast and model was developed for each option including optimizing the way each lane operates. Traffic forecasting and model creation were completed according to the above-described procedure. The exceptions are the No Build and Infrastructure Upgrade scenarios (Options 1 and 2) which both continue to operate with the same number of lanes as they do today. Each option was evaluated and optimized for existing, intermediate and design year conditions, including volumes, ETC usage and heavy vehicle parameters. The operational results of modeling are contained in Table 4 Traffic Queue and Delay Summary below. Expected queues and vehicle delays for the existing plaza configuration as well as for the various options being considered are listed for comparison.

**Table 3 Toll Plaza Sizing** 

		Option 1	Option 2	Option 3	Option 4	Option 5
			Alternate Site			
		Existing Layout New Layou				,
		No Build	Infrastructure Upgrade Only	Upgrade with Conventional Tolling	Upgrade with Highway Speed Tolling	Relocate Plaza with Highway Speed Tolling
	rthbound					
	tal Available orthbound Lanes	10	10	12	11	9
Cash	One Direction	7	7	7	7	7
Ca	<b>Reversible Lanes</b>	3 <sup>1</sup>	31	3 <sup>1</sup>	0	0
ETC	Highway Speed Lanes	0	0	0	2 (3 future)	2 (3 future)
Ramp	Dedicated Ramp Booths	0	0	2	2	0
	uthbound					
	tal Available uthbound Lanes	10	10	13	12	10
Cash	One Direction	7	7	8	8	8
Ca	<b>Reversible Lanes</b>	31	3 <sup>1</sup>	3 <sup>1</sup>	0	0
ETC	Highway Speed Lanes	0	0	0	2 (3 future)	2 (3 future)
Ramp	Dedicated Ramp Booths	0	0	2	2	0
To	tal Lanes	$17^{2}$	17 <sup>2</sup>	$22^{3}$	23 <sup>3</sup>	$19^{3}$
To	tal Width	295 ft	295 ft	454 ft	549 ft	435 ft

<sup>&</sup>lt;sup>1</sup>Reversible lanes are capable of being operated as either northbound or southbound.

<sup>2</sup>Existing number of lanes - does not meet plaza size needs for present or future.

<sup>3</sup>Number of lanes required to meet plaza sizing projections.

**Table 4 Traffic Queue and Delay Summary** 

		Existin No B Infrast	1 & 2: ng Site uild/ ructure rade	Existin Upgrad Conve	on 3: ng Site de with ntional ling	Existir Upgrae	de with y Speed	Optic Alter Locatic Highwa Toll	nate on with y Speed
	Year	2010	2020	2010	2020	2010	2020	2010	2020
<u>.</u> _	NB Queue (ft)								
NB Peak Hour (Friday PM)	average	257	211	46	42	124	95	130	96
ık F ay I	max	347	314	276	243	177	140	188	134
Pea rida	NB Delays (sec)								
E E	cash	61.0	54.6	35.7	34.6	31.1	22.7	33.8	24.9
	E-Zpass	14.4	21.0	13.4	20.5	5.2	6.6	5.2	4.1
our (	SB Queue (ft.)								
SB Peak Hour (Sun PM)	average	1347	720	111	93	196	130	198	132
	max	1674	1657	155	299	273	175	267	171
	SB Delays (sec)								
	cash	292.0	200.2	73.1	72.4	62.4	38.0	62.2	39.1
<b>J</b>	E-Zpass	153.7	77.1	25.7	21.4	5.6	7.2	4.5	4.6
NB & SB Peak Hour (Sat AM)	NB Queue (ft)								
	average	209	1133	91	169	143	118	142	113
	max	343	1670	376	550	206	166	192	170
	NB Delays (sec)								
	cash	57.0	129.8	50.9	100.2	40.1	28.6	40.2	29.0
	E-Zpass	20.7	63.4	18.4	39.5	4.3	4.6	4.0	3.6
	SB Queue (ft.)								
	average	400	1067	118	163	148	115	176	140
	max	782	1673	354	564	198	158	252	190
	SB Delays (sec)								
	cash	81.7	140.9	61.2	131.6	44.9	36.9	51.8	37.1
	E-Zpass	53.9	80.5	20.2	25.1	3.2	4.7	3.4	3.3

# C. Toll Plaza Layout

To begin the task of understanding the requirements, impacts and cost of these various plaza options, the following discussion outlines the physical layout or footprint of the plazas. References used to develop the design of a toll plaza are:

- "State of the Practice and Recommendations on Traffic Control Strategies at Toll Plazas," (2006) Federal Highway Administration (FHWA).
- "Geometric Design Highways and Streets," (2004) American Association of State Highway and Transportation Officials (AASHTO).
- "Freeway and Interchange Geometric Design Handbook," (2005) Institute of Transportation Engineers (ITE).

Two general plaza layouts are feasible for collecting cash and electronic tolls at a barrier toll plaza. One is a conventional toll plaza with toll booths and slow speed dedicated ETC lanes and the other is a conventional toll plaza with toll booths and highway speed tolling lanes. The conventional plaza layout requires all mainline traffic approaching the toll plaza, to slow down to pay the toll either with cash or with E-ZPass at a booth, and then accelerate to regain mainline speed. A highway speed plaza requires the Turnpike patron to choose between highway speed tolling (HST) or exiting the mainline for conventional cash toll collection. The traveling patron choosing HST may continue thru the mainline section of the plaza at the typical highway speed paying the toll using E-ZPass. The Turnpike patron utilizing cash tolls would exit-off the mainline section, come to a stop, pay a toll the traditional way, then accelerate to re-enter the mainline section. The following General Plaza Layout depicts the components of each of these layouts.

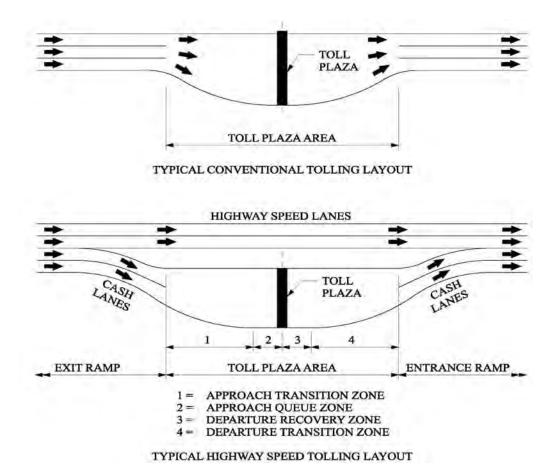


Figure 11 General Plaza Layout – Conventional and Highway Speed

In both of these layouts, the toll plaza area is designed following the guidelines from FHWA's "State of the Practice and Recommendations on Traffic Control Strategies at Toll Plazas." The toll plaza area consists of four zones: Approach Transition Zone, Approach Queue Zone, Departure Recovery Zone, and Departure Transition Zone. The exit/entrance ramps of the highway speed plaza layout are designed according to AASHTO standards. Table 5 Toll Plaza Layout Summary lists the component lengths for each of the identified options and associated tolling layout.

Table 5 Toll Plaza Layout Summary

				Length (feet)	(feet)			
Components	Option 1 & 2 Existing Site No Build / Infrastructure Upgrade	Option 1 & 2 Existing Site No Build / nfrastructure Upgrade	Opti Existii Conventid Lay	Option 3 Existing Site Conventional Plaza Layout	Option 4 Existing Site Highway Speed Plaza Layout	on 4 ng Site y Speed	Option 5 Alternate Site Highway Spee Plaza Layout	Option 5 Alternate Site Highway Speed Plaza Layout
	NB: 8 Cash Lanes	SB 9 Cash Lanes	NB: 8 Cash Lanes	SB: 10 Cash Lanes	NB: 7 Cash Lanes	SB: 8 Cash Lanes	NB: 7 Cash Lanes	SB: 8 Cash Lanes
Exit Ramp	NA	NA	NA	NA	2500	2500	2500	2500
Approach Transition Zone	200	200	1425	1625	1200	1400	1200	1400
Approach Queue Zone	200	700	300	300	300	300	300	300
Departure Recovery Zone	200	200	200	200	200	200	200	200
Departure Transition Zone	700	700	1425	1625	1200	1400	1200	1400
Entrance Ramp	NA	NA	NA	NA	2500	2500	2500	2500
TOTAL	1800	1800	3350	3750	0062	8300	0062	8300

#### **SECTION 7 - ALTERNATIVES ANALYSIS**

The five options for a York Toll Plaza replacement have been developed based on infrastructure need, tolling strategies, and traffic demand. Mindful of developing a complete range of alternatives, the following options vary from a do-nothing or No-Build alternative to a newly constructed plaza with the latest in tolling technology. Considerations for each option included:

- safety;
- capacity;
- operation and physical conditions of the plaza;
- adherence to the previously established FHWA guidelines;
- cost; and,
- natural resource impacts.

Below is a discussion of each option's construction elements, design and operations deficiencies, and benefits and summary. Following this discussion are figures of the layouts and two tables that highlight the option costs and compare the various elements.

Option 1: No-Build

Option 2: Infrastructure Upgrade with No Additional Capacity

Option 3: Upgrade Existing Site with Conventional Tolling and Increased Capacity

Option 4: Upgrade Existing Site with Highway Speed Tolling and Increased Capacity

Option 5: Relocate Plaza to Alternate Location with Highway Speed Tolling

# Option 1: No-Build

For comparison purposes a No-Build option is introduced and discussed. This option would not invest in any upgrade or replacement of the facility. As it exists, this plaza is not in conformance with the current FHWA Design Guidelines and Best Practices. According to recent accident records, this plaza is considered a High Crash Location. Noteworthy deficiencies include the plaza not located at a high point or on a horizontal straight section of mainline. The Chase's Pond Road Interchange (Exit 7) is within 1,000 ft exacerbating accident potential especially for the Northbound on ramp merge area. The Southbound off ramp is also very close to the Plaza and requires unsafe weaving maneuvers to access the ramp. Sight distance criteria is not met for either direction of travel. Due to subsurface conditions, the bumpers that protect staff in the toll booths are sinking and creating additional safety concern.

The physical infrastructure, booths, tunnel, and canopy are all in urgent need of major renovation. This alternative will not address any of these issues, most notably are the sinking roadway and deteriorating undersized tunnel.

From an operational perspective, there are currently significant vehicle queue (backup) problems during the busiest periods. During these peak periods, the dedicated ETC lanes have limited access due to inadequate visibility and the lengthy queues that extend back into the mainline three-lane section. Once able to maneuver into one of the two dedicated ETC lanes for each direction, patrons are limited to a 10 mph speed limit which slows processing time. Another concern with the ETC lanes is that this moving traffic is typically sandwiched between stop-and-go traffic of the cash lanes. This occurs due to the need of operating the three middle lanes as reversible depending on the greatest demand. See Table 4 Traffic Queue and Delay Summary

for details on the traffic analysis for this option. Since no upgrades would occur in this option, there is no associated construction cost involved. Future maintenance to improve the condition of the existing infrastructure, such as the leaking tunnel and the sinking approach slabs, will be required. The maintenance costs would be significantly higher than the maintenance costs for new or upgraded plazas. Also, since no improvements would be made to this facility, there would be no associated wetland impacts.

This option does not address the current physical and safety deficiencies which will grow worse with time. The York Toll Plaza will continue to have capacity and operational issues that too will worsen with time. A no-build option for the York Toll Plaza does not meet any of the Maine Turnpike Authority's goals nor is it aesthetically appropriate for the "gateway" to Maine.

# Option 2: Infrastructure Upgrade with No Additional Capacity

This option would upgrade the infrastructure within the immediate area of the toll plaza. The current lane configuration would remain with no increased capacity. The infrastructure to be replaced would include: toll booths and bumpers, canopy, tunnel, approach slabs and toll equipment. The upgrade would not include: altering vertical and horizontal alignment, addressing the entire plaza's geotechnical issues, or improving access to Exit 7 On/Off ramps. The layout of this option can be seen in Figure 12.

From an operational perspective, one of the major constraints of this option is the need to maintain toll collection capability and capacity during construction. It is estimated that an additional one to two years of construction would be necessary to consider plaza replacement inplace. Rehabilitation in-place is deemed infeasible when considering need for continuous toll operation and the current lack of capacity. This option assumes that the upgraded toll plaza would be located approximately 200 feet north of the existing facility. Moving the plaza 200 feet north allows for construction phasing and minimizes interruptions to toll plaza operations. Replacement of the tunnel and approach slabs would be done with consideration of poor soil conditions and projected settlement. However, the settlement of adjacent roadway would not be addressed here due to the poor soil limits extending up to 1,000 feet in each direction. (This would essentially be Option 3 without any additional capacity.)

Additionally, the existing significant queuing problems during the busiest periods would remain as they are today. During these peak periods, the dedicated ETC lanes have limited access due to the lengthy queues that extend back into the mainline three-lane section. Once able to maneuver into one of the two dedicated ETC lanes for each direction, patrons are limited to a 10 mph speed limit which slows processing time. Another concern with the ETC lanes is that this moving traffic is typically sandwiched between stop-and-go traffic of the cash lanes. This occurs due to the need of operating the three middle lanes as reversible depending on the greatest demand. See Table 4 Traffic Queue and Delay Summary for details on the traffic analysis for this option.

With respect to FHWA's Design Guidelines and Best Practices, this plaza would continue to be non-conforming to several standards. The plaza is not at a high point or located on a horizontal straight section. The Chase's Pond Road Interchange (Exit 7) is within 1,000 feet exacerbating a high crash location at the NB on ramp merge area. Sight distance design criteria is not met for either travel direction. The estimated construction cost to replace existing infrastructure is approximately \$10.4 million; see Table 6 Cost Comparison Table for details of this cost.

Since the improvements are being made within the existing footprint, no wetland impacts are expected. With this option, the majority of current infrastructure deficiencies will be addressed but many safety deficiencies will still exist and will grow worse with time. The York Toll Plaza will also continue to have capacity and operational issues that too will worsen with time. An "infrastructure upgrade" option for the York Toll Plaza does not meet all of the Maine Turnpike Authority's goals for safety, operation and maintenance, and will not address the outwardly visible aspects, operation and capacity, of essentially the "gateway" to Maine.

# Option 3: Upgrade Existing Site with Conventional Tolling and Increased Capacity

This option would upgrade the infrastructure, as noted in Option 2, along with additional conventional tolling capacity to meet peak traffic volumes. Several layouts were investigated during the design process altering the horizontal alignment to avoid the existing utility building and separating ramp traffic from mainline traffic. The chosen layout, seen in Figure 13, consists of 22 tolling lanes: eight (8) Northbound and ten (10) Southbound mainline toll lanes with two (2) dedicated ramp toll lanes for Exit 7 in each direction and either two or three dedicated ETC lanes per direction on mainline. This design minimizes the weaving conflicts of ramp and This layout assumes that the upgraded toll plaza would be located approximately 200 feet north of the existing facility. Moving the plaza 200 feet north allows for construction phasing and uninterrupted toll plaza operations. Rehabilitation in-place is infeasible when considering the need for continuous toll operation and the current lack of capacity. Replacement of the tunnel and approach slabs would be done with consideration of projected settlement. Lightweight fill will be considered to minimize differential settlement. For purposes of this report, conventional fill is utilized and included in the estimate. Advance signing for the Exit 7 Interchange and dedicated ramp lanes must be incorporated with the toll plaza signing. It will likely be complicated and potentially confusing to the public.

With this layout, vehicle processing time improves with the expanded plaza, but ETC users are still limited to slow vehicle speeds. This plaza would accommodate the heaviest traffic volumes with minimal queuing. See Table 4 Traffic Queue and Delay Summary for details on the traffic analysis for this option.

With respect to FHWA's Design Guidelines and Best Practices, this plaza would continue to be non-conforming to several standards. Although vertical adjustments are proposed, the toll plaza is not located on a high point. The plaza is also not located on a horizontal straight section. The Chase's Pond Road Interchange is within 1,000 ft of the toll plaza however, dedicated ramp booths minimize conflicts by physically separating mainline traffic from ramp traffic. Sight distance design criteria is not met for either travel direction.

The estimated construction cost to upgrade the existing infrastructure and additional conventional tolling is approximately \$27.3 million; see Table 6 Cost Comparison Table for details of this cost.

The existing site is surrounded by wetlands. Potentially, 16 acres of wetland will be impacted. Mitigation costs for these impacts are approximately \$6.6 million assuming a 4:1 replacement ratio.

Although traffic capacity will be improved, the \$27 plus million construction cost to update this facility - while not addressing the safety and geometric deficiencies - is not prudent.

# Option 4: Upgrade Existing Site with Highway Speed Tolling and Increased Capacity

This option would upgrade the existing facility with highway speed tolling. Layouts investigated during the design process included altering the horizontal alignment to avoid the existing Administration Building, reconfiguring the Exit 7 Interchange, and separating ramp traffic from mainline traffic. The final layout developed accepted impacts to the Administration Building in exchange for an improved horizontal alignment and minimized environmental impacts. The layout consists of seven NB and eight SB cash toll lanes, two highway speed toll lanes and two dedicated ramp toll lanes for each direction. This can be seen in Figure 14.

This design minimizes the weaving conflicts of ramp and mainline traffic. This layout assumes that the upgraded toll plaza would be located approximately 200 ft north of the existing facility. Moving the plaza 200 ft north allows for a more accommodating construction phasing and uninterrupted toll plaza operations. Rehabilitation in-place is infeasible when considering the need for continuous toll operation and the current lack of capacity. Replacement of the tunnel and approach slabs would be done with consideration of projected settlement. Lightweight fill will be considered to minimize differential settlement. For purposes of this report, conventional fill is utilized and included in the estimate. The advance signing for the Exit 7 Interchange and dedicated ramp lanes, in concert with signing for highway speed tolling that must be incorporated with the toll plaza signing, will likely be complicated and potentially confusing the public.

With this layout, vehicle processing time improves with the expanded plaza as ETC usage increases. This plaza would accommodate the heaviest traffic volumes with minimal queuing for both cash and ETC patrons. Toll plaza personnel will be interacting with the stopping traffic and not the free flowing ETC traffic which will result in improved safety at the toll plaza area. See Table 4 Traffic Queue and Delay Summary for details on the traffic analysis for this option.

This Option would continue to be non-conforming to several standards. Although vertical adjustments are proposed, the toll plaza is not located on a high point. The plaza is not located on a horizontal straight section. The Chase's Pond Road Interchange is within 1,000 ft of the toll plaza however, dedicated ramp booths physically separate mainline traffic from ramp traffic. Sight distance design criteria is not met.

The estimated construction cost to upgrade the existing facility with highway speed tolling is approximately \$37.3 million; see Table 6 Cost Comparison Table for details of this cost.

The existing site is surrounded by wetlands. Potentially, 26 acres of wetland will be impacted. Mitigation costs for these impacts are approximately \$10.6 million assuming a 4:1 replacement ratio.

Although traffic capacity and ETC processing time will be improved, the \$37.3 million construction cost and \$10.6 million wetland mitigation cost to update this facility, while not addressing the safety and geometric deficiencies, is not prudent.

# Option 5: Relocate Plaza to Alternate Location with Highway Speed Tolling

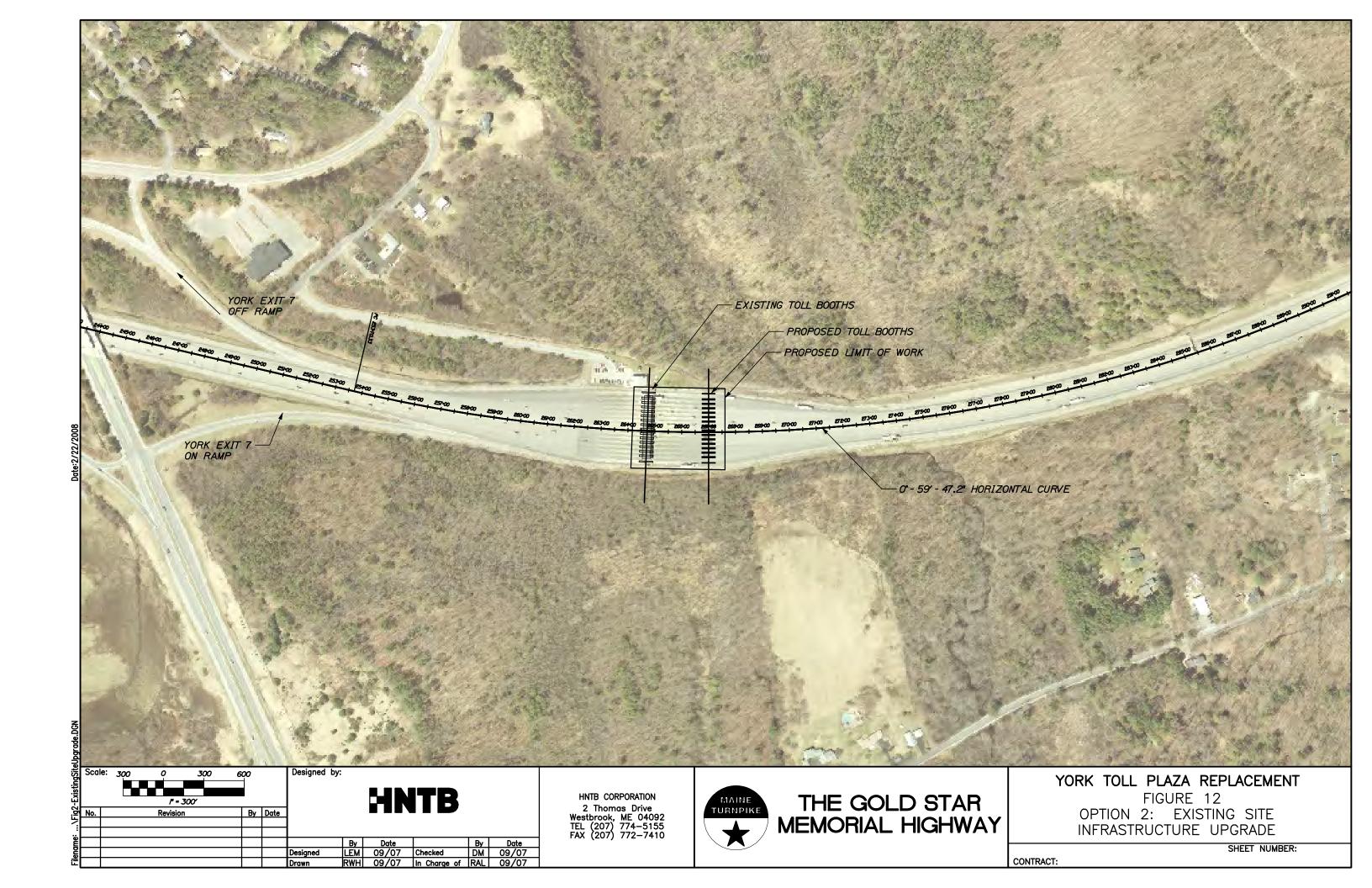
This option would locate the plaza to a new location with a combination of highway speed tolling and conventional cash tolls. This layout was developed with seven NB and eight SB cash toll lanes and two highway speed toll lanes in each direction. This can be seen in Figure 15. Locating a toll plaza the appropriate distance away from an interchange would eliminate the undesirable vehicle weaving maneuvers that are present for all options at the existing site. Construction phasing will be less complicated than the other options since nearly all of the work can occur without hindering the mainline traffic or toll collection at the existing plaza. Coordination of the new facility opening and demolition of the existing facility will also be less complicated.

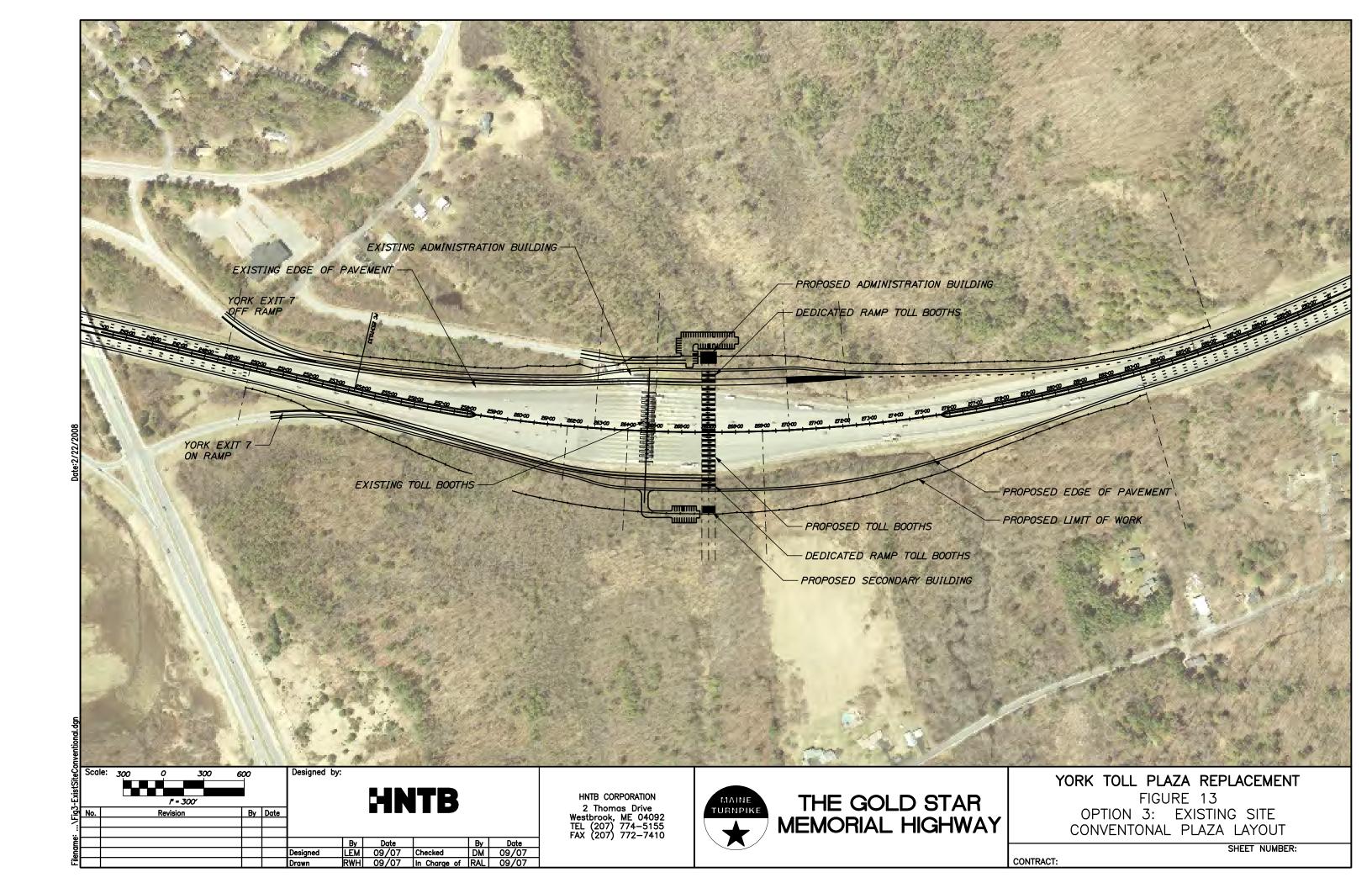
With this layout, processing time improves with the expanded plaza as ETC usage increases. This plaza would accommodate the heaviest traffic volumes with minimal queuing for both cash and ETC patrons. The potential vehicle and pedestrian conflicts still exist within the cash toll booth area however, it is minimized by not having any slow speed dedicated ETC lanes. See Table 4 Traffic Queue and Delay Summary for details on the traffic analysis for this option.

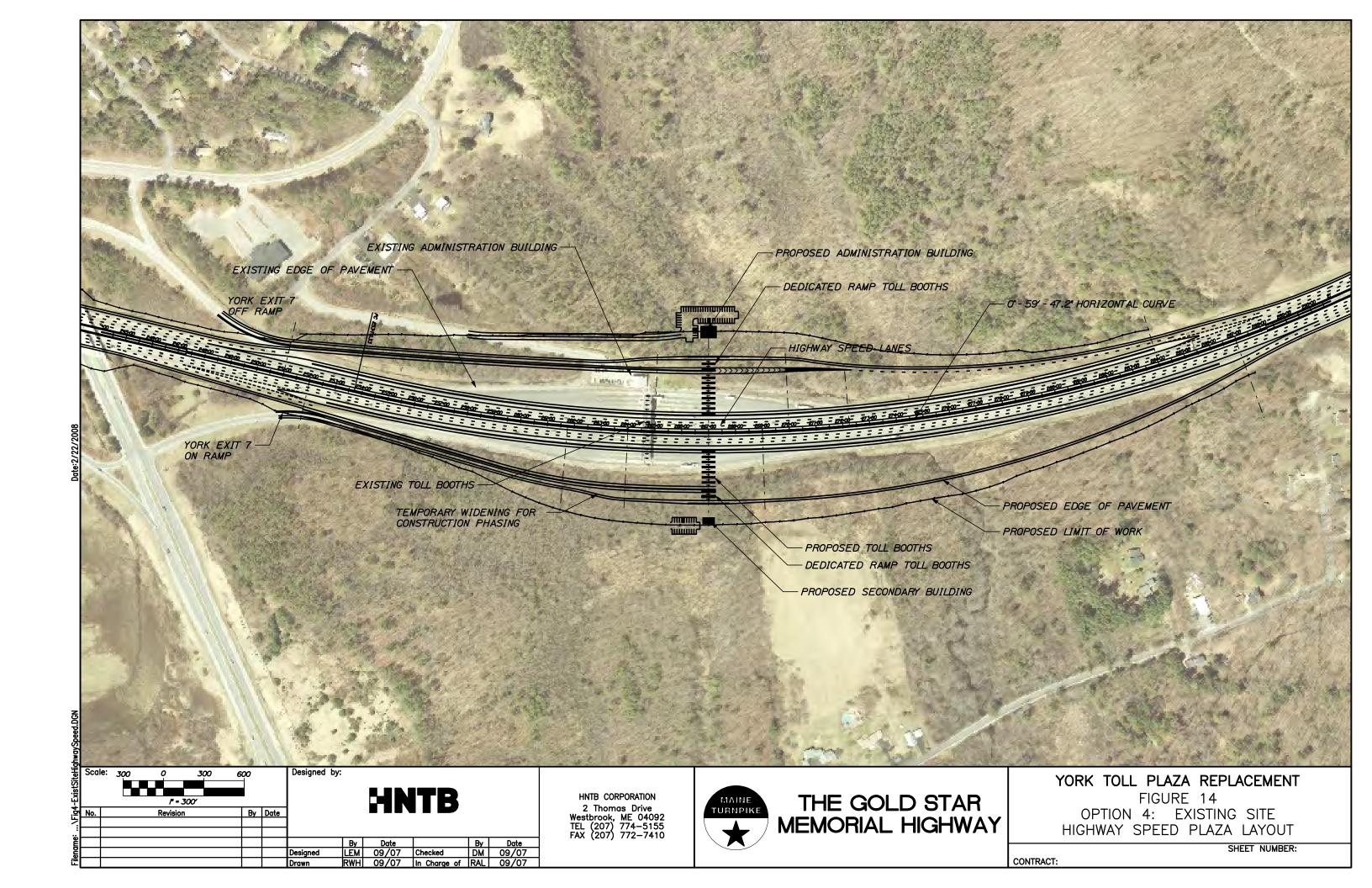
This option would adhere to the previously mentioned industry standards. The plaza would be located on a high point and on a horizontal straight section. Sight distance design criteria would be met. The construction cost to build a new tolling facility with highway speed tolling in a new location is approximately \$38.4 million; see Table 6 Cost Comparison Table for details of this cost.

Depending on the chosen alternate site, 1-11 acres of wetland will potentially be impacted. Mitigation costs for these impacts would range from approximately \$0.5 to \$4.2 million assuming a 4:1 replacement ratio.

This Option will result in a toll plaza that 1) operates safely for both Turnpike patrons and staff, 2) provides adequate capacity for current and future traffic demands, 3) meets today's industry standards for plaza location and infrastructure needs, and 4) implements modern technology to efficiently process Turnpike traffic with Highway Speed Tolling lanes. The construction and wetland mitigation costs are in upwards of \$38 million, which are very similar to other options that fail to provide these improvements. This Option is the most cost effective way to meet York Toll Plaza's safety and operational needs and will allow the York Toll Plaza to be a prominent "gateway" to the State of Maine.







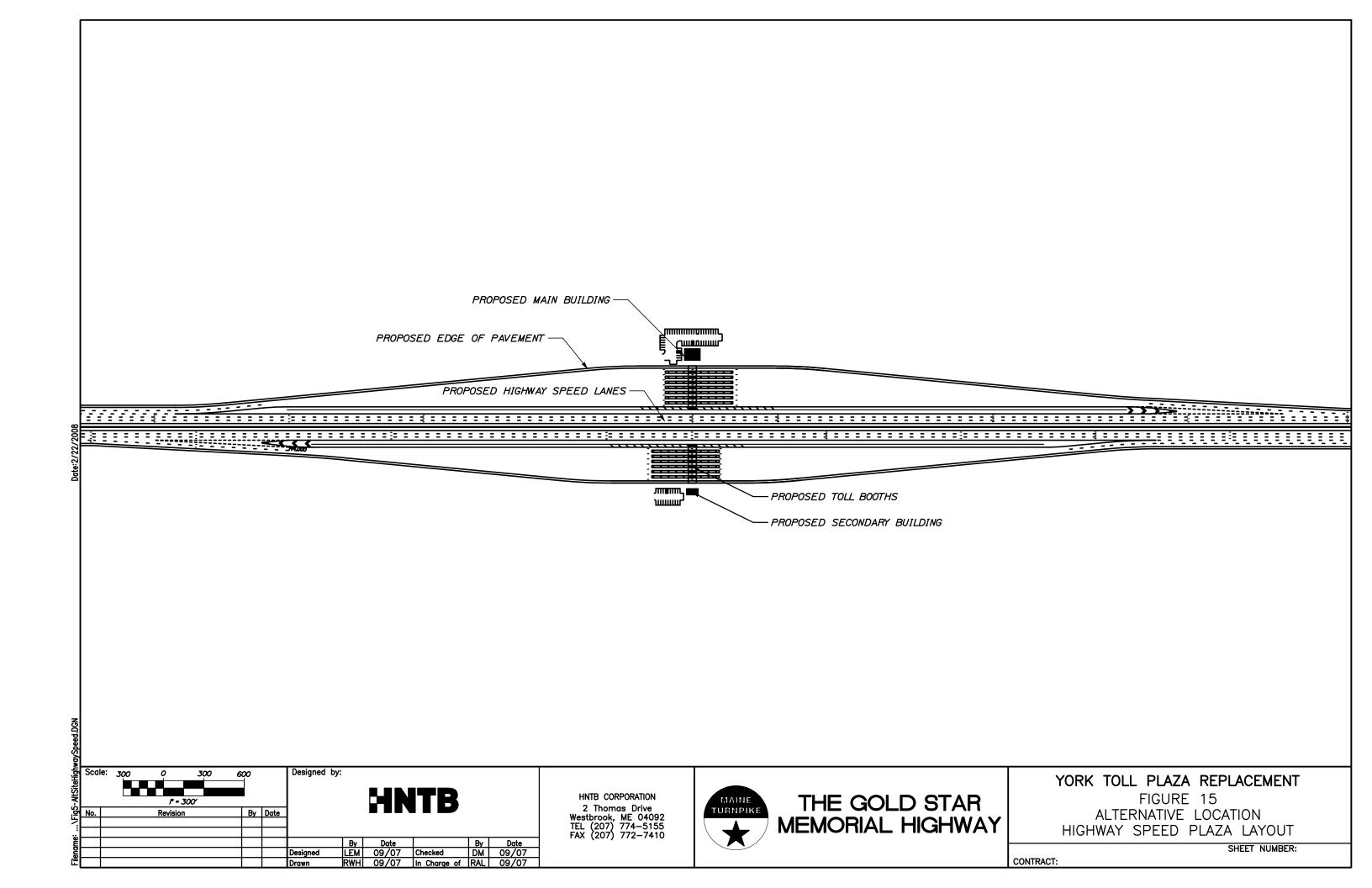


Table 6 Cost Comparison Table

	Option 2	Option 3:	Option 4:	Option 5:
Item Description	<b>Existing Site</b>	Existing Site Upgrade	Existing Site Upgrade	Alternate Location
	Infrastructure	with Conventional	with Highway	with Highway
	<b>Upgrade Only</b>	Tolling	Speed Tolling	Speed Tolling
Clearing	80	\$101,000	\$190,000	\$195,000
Common Excavation	\$150,000	\$468,000	\$976,800	\$3,084,600
Rock Excavation	0\$	\$62,500	\$127,500	\$967,500
Common Borrow	\$34,500	\$822,000	\$871,500	\$491,250
Hot Mix Asphalt	\$483,000	\$4,221,000	\$7,140,000	\$7,420,000
Gravel	\$173,000	\$1,782,000	\$2,880,000	\$3,166,000
Removing Pavement Surface	80	\$162,000	\$538,800	\$123,750
Drainage	80	\$852,000	\$1,282,000	\$1,403,250
Concrete Barrier And R/W Fence	0\$	\$1,000,500	\$2,274,000	\$2,023,500
Loam, Seed, Mulch, Erosion Control	0\$	\$314,500	\$632,700	\$649,350
Utilities	50,000	\$175,000	\$175,000	\$399,750
Lighting	100,000	\$441,000	\$441,000	\$441,000
Maintenance Of Traffic	\$1,200,000	\$1,200,000	\$1,200,000	\$1,200,000
Permanent Guide Signs	80	\$700,000	\$700,000	\$700,000
Administration Building	0\$	\$3,450,000	\$3,450,000	\$2,930,000
Plaza Infrastructure	\$3,100,000	\$4,700,000	\$4,900,000	\$4,300,000
Toll Equipment	\$1,600,000	\$1,900,000	\$3,300,000	\$2,900,000
Chase's Pond Road Bridge Modifications	0\$	0\$	000'00£\$	0\$
Removal Of Existing York Toll Plaza	\$2,500,000	\$2,500,000	\$2,500,000	\$2,500,000
Miscellaneous @ 10%	\$944,930	\$2,485,150	\$3,387,930	\$3,489,495
Total*	\$10,400,000	\$27,300,000	\$37,300,000	\$38,400,000

<sup>\*</sup> Estimate does not include ROW and wetland mitigation costs.

#### Table 7 Comparison Matrix

	Option 1: Existing Site No Build	Option 2: Existing Site Infrastructure Upgrade with No New Capacity	Option 3: Existing Site Upgrade with Conventional Tolling	Option 4: Existing Site Upgrade with Highway Speed Tolling	Option 5: Alternate Location with Highway Speed Tolling
Plaza Capacity	Current capacity issues would configuration of the plaza wou changed to optimize the avail	uld have to be continually	Plaza would accomodate all but the heaviest traffic volumes with acceptable queing.	Plaza would accomodate the l minimal queing for cash patro patrons.	
Vehicle Delays 2010 (seconds)	NB Peak Hour Cash: 61.0 ETC: 14.4	NB Peak Hour Cash: 61.0 ETC: 14.4	NB Peak Hour Cash: 35.7 ETC: 13.4	NB Peak Hour Cash: 31.1 ETC: 5.2	NB Peak Hour Cash: 33.8 ETC: 5.2
(seconds)	SB peak Hour Cash: 292 ETC: 153.7	SB peak Hour Cash: 292 ETC: 153.7	SB peak Hour Cash: 73.1 ETC: 25.7	SB peak Hour Cash: 62.4 ETC: 5.6	SB peak Hour Cash: 62.2 ETC: 4.5
Vehicle Delays 2020	NB Peak Hour Cash: 54.6 ETC: 21.0	NB Peak Hour Cash: 54.6 ETC: 21.0	NB Peak Hour Cash: 34.6 ETC: 20.5	NB Peak Hour Cash: 22.7 ETC: 6.6	NB Peak Hour Cash: 24.9 ETC: 4.1
(seconds)	SB peak Hour Cash: 200.2 ETC: 77.1	SB peak Hour Cash: 200.2 ETC: 77.1	SB peak Hour Cash: 72.4 ETC: 21.4	SB peak Hour Cash: 38.0 ETC: 7.2	SB peak Hour Cash: 39.1 ETC: 4.6
	Similar alignment to the toll p familiarity with this traffic pat		on decision making. There is Vehicles must decide to use high toll lanes. This will be a new trad		
	Electronic toll vehicles must s	low as they enter the toll plaza	a area.	Provides ETC customers with minimal queuing or speed red possible level of service for E <sup>*</sup> speeds leading to more efficie	luction. This provides the best TC customers with the higher
Operations	Processing of patrons remain	s the same.	Processing of cash patrons improved with expanded plaza but processing of ETC patrons limited to slow vehicle speed.	Increased efficiency of proces cash paying.	sing patrons - both ETC and
		icated toll lanes via the toll pla n area impacts access and effic		ETC patrons are not effected Cash lane queues minimized t from cash lanes.	
Construction Cost	\$0	\$10.4 Million	\$27.3 Million	\$37.3 Million	\$38.4 Million
Wetland Mitigation Costs	\$	0	could exceed \$6.6 million	could exceed \$10.6 million	\$0.5 to \$4.2 million or more
Potential wetland impacts (NRCS soils)	0 a	Potential 1 to 11 acres impacted			
General Layout	Existing plaza remains	Replace plaza approximately	ffic is separated to/from	Replace plaza at alternate location.	
	n/a	n/a	plaza.	The 15 Separated to/Hom	n/a
Horizontal Alignment	Plaza is not located on tanger	Plaza Area would be located on a tangent.			
Vertical Alignment	Existing Plaza is at a low poin point.	Plaza at high point, minor vertical grade adjustments possible.			
Sight Distance	Decision sight distance is not	completely satisfied.			Decision sight distance is satisfied.
	n/a	n/a	Modification to Chase's Pond Road Bridge is anticipated.	n/a	n/a
Proximity of plaza to interchanges / bridges	Recommended 1 mile separation from plaza and interchange is not met. Close proximity of Chase's Pond Rd Exit creates safety issues for vehicles. NB mainline lanes between entrance ramp and plaza is a high crash location.				Recommended 1 mile separation from plaza and interchange will be met.
Constructability	n/a	Construction phasing required. Impacts to mainline traffic to be minimized.			
Local Road Access	No additional local access nee	eded			Local access to be provided to main utility building
Tunnel & Plaza Work	Costs to repair tunnel are extensive.	Tunnel and Plaza Replacemer	nt is assumed		New Tunnel and Plaza will be constructed
Geotechnical conditions	Existing site has settlement is bumpers at toll booths are se points for vehicles with low gr issues for toll attendants.	ttling. This creates hangup	Geotechnical issues at toll pla weight fill.		Geotechnical issues are unknown.
Utility Building	Existing building functions pro	operly.	Replace Existing Utility Building utility building	ng and construct auxiliary	New Utility Buildings
Utilities	No modification to utilities.		Utilities exist but modification	ns are anticipated.	Utilities will be brought to alternate location
Potential displacements	No displacements.	No displacements.	No anticipated displacements	No anticipated displacements	Potential displacements

Level of Acceptability: Best

#### **SECTION 8 - PUBLIC INVOLVEMENT**

An integral part of the evaluation of York Toll Plaza's future is public input. As is common on MTA projects during the planning process, and as requested by the Maine Legislature, the MTA conducts a number of public informational meetings with local and interested citizens. As is shown below, a number of meetings and presentations were held to share and gather information surrounding the York Toll Plaza Replacement project. Due to the many commitments and previously scheduled meetings for these groups during October and November of 2007, the public input portion of this project ultimately delayed the delivery of this Final Report. The final presentation and information gathering session was held with the Joint Select Boards of Wells, Ogunquit and York on January 23, 2008.

The MTA continues to seek input from the public during the entire project. The purpose of these early input meetings is to better understand community requests, desires, and concerns. Meetings were held with a wide range of groups or audiences including Town Officials, Boards of Selectpersons from area Towns, State and Federal Environmental Resource Agencies, Local and Interested State Legislators, and the general Public. These meetings were designed to incorporate two-way communication, both project information sharing as well as listening, understanding, and answering questions and concerns. Following is a summary of the meetings that have been conducted:

- Town staff input and information sharing throughout
- Town Managers' meetings
  - 1<sup>st</sup> meeting Sept. 26, 2006

  - 2<sup>nd</sup> meeting Nov. 29, 2007
    3<sup>rd</sup> meeting January 22, 2008
  - 4<sup>th</sup> meeting February 15, 2008
- Joint Select Board meeting Oct. 25, 2006
- State/Federal Interagency meeting Oct. 10, 2006
- Legislative Tour & Briefing Aug 9, 2007
- Legislative Tour & Briefing Aug 10, 2007
- Legislative Tour & Briefing Sep 21, 2007
- Legislative Tour & Briefing Dec 10, 2007
- Joint Select Board presentation January 23,2008

Individual Meeting Notes are contained in Appendix A.

#### **SECTION 9 – RECOMMENDATION**

From the evaluation, and in conjunction with the plaza's accident history, the York Toll Plaza is operationally inefficient, structurally deficient, and is located such that these conditions compromise overall staff and patron safety. Replacement of the York Toll Plaza needs to occur to improve operations and meet current design guidelines. To determine the most effective course of action that addresses immediate and future needs, this report documents the comprehensive development and evaluation of five strategies or Options. Following is a summary of these five options along with recommendations.

Option 1: No Build (Leave Plaza in Existing Condition and Tolling Arrangement)

Option 1 does not satisfy any of York Toll Plaza's safety or operational needs, present or future. This option leaves the Plaza requiring extensive ongoing maintenance. **This Option is dismissed from further consideration.** 

Option 2: Infrastructure Upgrade with No Additional Capacity

Option 2 addresses only the structural deficiencies of the existing infrastructure. This option does not provide the needed additional capacity, does not address the location deficiencies, does not meet current industry design standards and will not address many safety or operational issues for Turnpike patrons and staff. The cost to provide this option would be lost without benefit as it would not remedy any of the truly needed improvements. **This Option is dismissed from further consideration.** 

Option 3: Upgrade Existing Site with Conventional Tolling and Increased Capacity

Option 3 increases capacity and upgrades the infrastructure but does not address the safety and operational concerns associated with the current plaza location. The cost of this option would be more than two-thirds the cost of the relocated option but would provide only marginal benefit. In addition, there is no opportunity for implementing modern Highway Speed Lanes with this option. **This Option is dismissed from further consideration.** 

Option 4: Upgrade Existing Site with Highway Speed Tolling and Increased Capacity

Option 4 marginally improves traffic capacity and ETC processing time but fails to address the safety concerns associated with the current plaza location. Full efficiency of Highway Speed Tolling will not be realized due to the location on a curve and near a hill. A costly interchange reconfiguration and reconstruction will be necessary resulting in confusing and complicated traffic patterns. The cost of this option is similar to that of the full build option but provides far less benefit. To effect additional safety benefits in traffic movements would require an interchange reconstruction that is far greater than considered here, likely more than doubling the cost of this option. **This Option is dismissed from further consideration.** 

Option 5: Relocate Plaza to Alternate Location with Highway Speed Tolling

Option 5 will result in a toll plaza that 1) operates safely for both Turnpike patrons and staff, 2) provides adequate capacity for current and future traffic demands, 3) meets today's industry standards for plaza location and infrastructure needs, and 4) implements modern technology to efficiently process Turnpike traffic with Highway Speed Tolling lanes. This Option is the most cost effective way to meet York Toll Plaza's safety and operational needs and will allow the York Toll Plaza to be a prominent "gateway" to the State of Maine. This Option is the only reasonable option and is the choice the MTA will pursue.

The results of the alternatives analysis support the MTA selecting and pursuing Option 5; constructing a new toll plaza, with Highway Speed Tolling, in a new location. Constructing a toll plaza in a new location will result in 1) safer operations for both Turnpike patrons and staff, 2) adequate capacity for current and future traffic demands, 3) a plaza that meets industry design standards for layout and operations, and 4) the ability to implement modern and more efficient Highway Speed Lanes. **None of the other four options are able to provide all of these features.** 

Option 5 is the most cost effective way to meet York Toll Plaza's needs and it will allow the York Toll Plaza to be a prominent "gateway" to the State of Maine. Constructing a new plaza, with Highway Speed Tolling, at a new location is the most prudent direction for addressing existing safety and operational issues and future needs of a Southern Toll Plaza and gives the Maine Turnpike Authority a sound investment in a facility that will provide the public with a safe, efficient, and modern toll plaza today and into the future.

The Maine Turnpike Authority will continue with the York Toll Plaza Replacement project by pursuing the identification of a new location for the plaza that meets national engineering standards and that will accommodate Highway Speed Tolling.

## **SECTION 10 NEXT STEPS**

The Maine Turnpike Authority will continue with the York Toll Plaza Replacement project by pursuing the site identification and screening process to find a new location for the plaza. The site identification and selection process to be followed is in accordance with the Alternatives Evaluation per the Army Corp of Engineers Highway Methodology, complies with Section 404 of the Clean Water Act and complies with the Department of Environmental Protection's National Resource Protection Act. A brief summary of the tasks or steps to accomplish this project is offered here for reference.

- Site Selection Studies completed
- Conceptual Designs and Estimates refinements underway
- Site Screening & Preferred Site Selection underway
- Public Participation
- Preliminary Design & Mitigation
- State and Federal Permit Applications
- Final Design
- Public Process per Permit Requirements
- Permit Development and Approval Process
- Construction

# APPENDIX A MEETING MINUTES



**HNTB** Corporation 2 Thomas Drive Westbrook, ME 04092 (207) 774-5155

☑ Joe Grilli, HNTB

ĭ Jonathon Labonte, MTA

Subject: Southern Toll Plaza Date: September 26, 2006

Place: Maine Turnpike, York Maintenance Facility

Attendees: 

Philip Clark, Town of Ogunquit

ĭ Jane Duncan, Town of Wells ☑ Jim Kanak, York County Coast Star ☑ Jon Speers, Town of Ogunquit

ĭ Steve Burns, Town of York

☑ Paul Godrey, HNTB ☑ Don Ettinger, HNTB

By: Don Ettinger

Copy: HNTB File No: 09009-xw-005-011

#### **Minutes**

#### Introduction

- 1. Conrad provided a history of the southern section of the Maine Turnpike and discuss traffic volumes on the turnpike and on Route 1. The revenue generated by the York Toll plaza was discussed. The perception of traffic diversion was discussed in length by the attendees.
- 2. It was explained that one way tolling was studied and determined not prudent.
- 3. Conrad explained that highway speed tolling is recommended for the replacement plaza.

## Study Purpose

Conrad & Joe explained that the study purpose is to find the most suitable location for replacing MTA's southern toll plaza.

#### Project Need

The condition of the existing plaza and need for replacement was explained. Geotechnical issues, horizontal and vertical geometry, safety, traffic congestion, as well as the age of the existing facility were noted as reasons for replacement.

# **Technical Scope**

- 1. The technical scope was explained. Effort to include establishment of design criteria, development of plaza footprint, considerations for reuse of the existing plaza, considerations for single vs split plazas, screening of possible plaza locations considering human resources, natural resources, and engineering constraints.
- 2. Detailed evaluation of short-listed plaza locations will be conducted and recommendations will be documented in a report.
- 3. Environmental agency coordination will be included in the process. Steve Burns mentioned that City environmental permits may be required as well.

- 4. The study area was defined from Chase's Pond Road to Wells interchange. Jane Duncan requested the study limits be extended south of Chase's Pond Road. HNTB to review federal incumbencies and traffic diversion associated with locating a plaza south of Chase's Pond Road.
- 5. HNTB to reach out to municipalities for latest tax map data (recent developments, subdivisions etc). Municipalities have recent aerial photos.

# **Community Input**

- 1. It was explained that another meeting with attendees would occur when the report was completed and just prior to a public meeting likely to occur in December.
- 2. Towns suggested no meetings in December. Public meeting to occur in January.
- 3. Towns suggested MTA reachout the Town planning boards early in this process, prior to any recommendations. It was agreed to meet with Town planning boards (Wells, Ogunquit, York) at a joint meeting tentively scheduled for Oct 25<sup>th</sup>, 6:30pm in Ogunquit. The meeting would be recorded and brought back to each community and retelevised in each community as a means of public outreach.

#### Schedule

The schedule was discussed. The study would be completed by end of the year. Public process in January, 07. Final design and permitting in 2007. Construction to begin spring of 2008 and extend for two construction seasons.

#### **MEETING NOTES**



Date: January 22.2008

HNTB Project No.: 09009-XW-005-011

Meeting Name: Project Update & Work Session With Town of York

Manager and Community Development Director

Location: York Town Office

Purpose: Project Update and Public Meeting Preparation

Attending: Rob Yandow, Steve Burns, Jonathan LaBonte, Dale Mitchell

• What review authority does/will the Town of York have?

- Noise has been a local concern especially from neighborhoods near MM8.1.
- Highway Speed Tolling is viewed by most as an improvement and a good idea.
- It would be viewed as a good gesture to make as much data as possible available online; possibly providing a link from the Town website to the MTA website.
- Town has Local Access television and can use it for advertising the meeting
- Town asked MTA to investigate what permitting is going to be required and report back.

This is our understanding of items discussed and decisions reached. Please contact us if there are changes or additions.

Submitted by,

HNTB CORPORATION - Dale A. Mitchell, P.E.

## **MEETING NOTES**



Date January 23, 2008

HNTB Project No.: 09009-XW-005-011

Meeting Name: Joint Select Board Presentation

Location: Town of Ogunquit – Dunaway Center

Purpose: Gain Public Input on Decision to Replace the York Toll Plaza in a New Location

Final Informational Session per LD 534

Attending: SelectBoards of Wells, York and Ogunquit; Public

Maine Turnpike: Conrad Welzel, Dan Paradee

HNTB Corporation: Dale Mitchell, Paul Godfrey, Roland Lavallee

- 1. Has the MTA looked at removing the York Toll Booth completely? Whatever revenue is lost should then be collected someplace north.
  - a. There is a Toll Rate Structure group studying many possibilities.
- 2. The O-D survey was carried out on the wrong day, it was raining and a Friday. The rain caused more folks to be on the road. Because of this the results are not valid.
  - a. The sun was out by 11:30am. Friday traffic, especially these summer volumes, is exactly what we were asked to base our research on.
- 3. Why is there a \$1.75 toll at York? Why not reduce the toll amount and add exit tolls back into the program?
  - a. This would essentially be going back to a 'ticket system' which had other backups and delays associated with it.
- 4. Good idea to upgrade or replace the plaza. Presentation makes a good case for the replacement as well as for the new Highway Speed Tolling.
- 5. Where are the potential sites that are being considered for a new plaza? How can we answer your replacement question if we don't know where the sites are?
  - a. Purpose of meeting is to discuss the need for plaza replacement and to validate that a new site is warranted; not where it might be located.
  - b. Study underway with results likely available for a late February or early March meeting. Currently, 16 identified sites have been narrowed to 4-6 sites.
- 6. Consider locating the new plaza in Ogunquit. Should also consider an interchange in Ogunquit.
- 7. What is the estimated cost of replacing the Plaza? Will this cause increased tolls?
  - a. Costs will be investigated when we arrive at a smaller number of sites.
  - b. Conceptual estimates of a new plaza are approximately \$35 million.
- 8. Can a new plaza be smaller; less of a structure?
  - a. Highway speed tolling will use typical mainline widths and remove need for as many cash toll booths.
- 9. Can the overhead structures be removed? Technology is surely available to either put sensors in the ground or on short shoulder mounted poles.
  - a. There are different types of sensors available and research is being done. At present, reading a toll tag requires some type of overhead viewer. Side mounted readers will not work for multiple lanes.
- 10. Biggest issue with Diversion is the truck traffic. Trucks leave the York Industrial Park and head north to wells, over local roads, to avoid the York Toll. The Wells toll plaza should be modified to collect these tolls and most importantly discourage these diverted trips.
- 11. Southern Maine residents should be given a discount on tolls; it should be based on home zip code. When you go through the York Toll Booth, this discount should be given.
  - a. Interstate Commerce Act prohibits this type of activity.
- 12. Biggest issue is toll inequity! Plaza replacement is secondary to fixing the toll rates.
  - a. A Toll Rate Structure group is currently meeting to investigate the overall system.
- 13. Consider adding more E-ZPass readers to at least make all E-ZPass trips equitable. More research needs to be done to make cash customer tolls more equitable.
  - a. A Toll Rate Structure group is currently meeting to investigate the overall system.

- 14. Relocation of the toll plaza south of the York plaza is not a good idea from a change in traffic pattern perspective. Replacement is a good idea based on current needs. Continue considering those locations north of the existing plaza.
- 15. Why are sites south of the York plaza not part of the short list of sites? There are plenty of open spaces.
  - a. The area south of York was evaluated with only two candidate locations identified. These two sites fell out following the secondary screening.
  - b. Site ID criteria included: straight stretch, no interchanges, no bridges and small hill. Site screening criteria included environmental and human resource impacts.
- 16. Aren't there restrictions for building south of York?
  - a. There are still Federal and State restrictions and implications for this but there are also technical reasons to not build in this area.
- 17. Consider locating a plaza south of Littlefield Road. At the same time an interchange should be built at some location south but as close to this as possible.
- 18. What exactly should we learn from the diversion numbers? Are these values good or bad?
  - a. Diversion rates are within the range estimated. At this time, these values are considered typical and are similar or lower than other toll way diversion rates.
- 19. Were Diversion numbers collected for commercial vehicles? These vehicles are creating safety concerns when diverting because they are using small local roads.
  - a. No, commercial vehicles were not surveyed.
- 20. Are Maine based accounts the same as out-of-State?
  - a. Maine based accounts benefit from the discount plans; others do not.
- 21. Did this (LD534) process slow things down? We were hoping to learn alternative sites tonight?
  - a. The LD report contains data and information normally investigated and reported. However, because the request came when it did, time was spent to go backwards and rejustify the conclusion we had already come to, i.e. replace the plaza in a new location with highway speed tolling.

This is our understanding of items discussed and decisions reached. Please contact us if there are changes or additions.

Submitted by,

HNTB CORPORATION – Dale A. Mitchell, P.E.

#### **MEETING NOTES**



Date: February 15, 2008

HNTB Project No.: 09009-XW-005-011

Meeting Name: Project Update & Work Session With Town of York

Manager and Community Development Director

Location: York Town Office

Purpose: Project Update and Public Meeting Preparation

Attending: Rob Yandow, Steve Burns, Jonathan LaBonte, Dale Mitchell

- Reviewed draft agenda for upcoming Public Information Meeting. Public needs to understand the selection process.
- Comparison Matrix is helpful but for now it should not have colors. Allow the Public to provide input then factor the colors in later.
- Development of a Fact Sheet to be left at Town Offices is a good idea. This can be left with display graphics. Plan and Profile along with Corridor Limits and 16 alternatives will work.
- For Public Meeting it would be useful to have a comparison matrix, without data, as one of the displays; basically to give people a sense that there is a methodology to the process.
- Be sure to answer all questions. There has been some public input that questions were not really answered
  instead there was some evasion.
- The presentation must be convincing and credible!
- If Public input is going to be used then share with Public how it is to be used. Do not give false hopes, be clear and honest on how much influence the Public has on the site selection.
- Following review of the Noise Video: Video has some good data and it would be useful at a later point in time. They do not believe it would add much to the purpose of the 2/27 meeting Site ID and initial screening. Consider showing this as a tool when a preferred site is selected.
- Let the Public request additional information before giving it to them.
- Video should include some type of point-of-reference for the dBA values.
- Send Public Notice to the Town for inclusion on their Local Access Channel.
- Asked if MTA could install some temporary markers at the 4 Alternative Sites. They wondered if folks might find a visual helpful.
- Eventually, graphics and reports should be made available on a website. Likely the best way would be to have a link from the Town's site to the MTA website.

This is our understanding of items discussed and decisions reached. Please contact us if there are changes or additions.

Submitted by,

HNTB CORPORATION - Dale A. Mitchell, P.E.

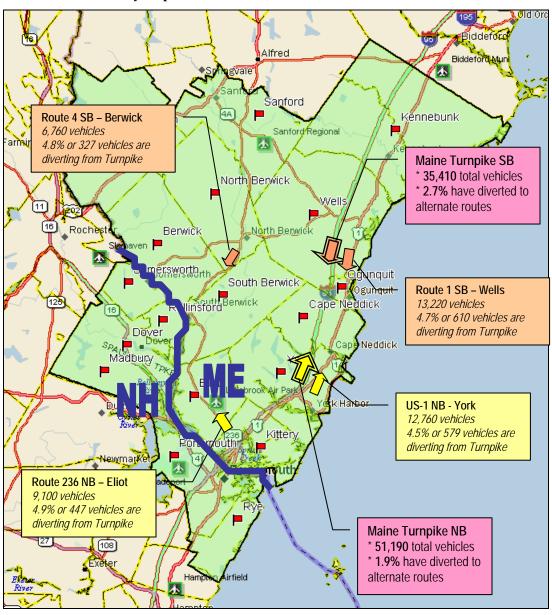
# APPENDIX B YORK TOLL DIVERSION STUDY 2007

# **Executive Summary**

HNTB Corporation conducted a diversion study for the Maine Turnpike Authority along the southern portions of the Maine Turnpike between Wells and York. The study included a broad interview survey and a smaller-scale license plate trace survey. The purpose of this study is to understand the level of traffic diverting from I-95 to major local routes in order to avoid the York Toll Plaza

Figure 16 summarizes the key results of the interview survey.

Figure 16: Diversion Summary Map



As Figure 16 illustrates, the interview survey indicated that about 4-5% of the traffic on the two alternate routes were comprised of vehicles diverting around the York toll plaza. This equates to less than 2000 diverting vehicles out of almost 130,000 vehicles on the Maine Turnpike and parallel corridors during the survey period of a typical Friday in the summer months of July and August. Overall, the interview survey suggested that approximately **2-3%** of the traffic on I-95 diverts to avoid the toll plaza.

The license plate trace survey focused on the Route 1 diversion route. This survey indicated that about 0.7%-1.6% of vehicles on I-95 divert around the toll plaza on Route 1. This range is consistent with the result of the interview survey.

More details are available from the Maine Turnpike Authority in the full version of the York Toll Diversion Study 2007.